



Notice of Intent

Foxboro Learning Early Childhood Education Center

January 2026

“0” Cambridge Street Burlington, Massachusetts

Prepared For:

Foxboro Learning, LLC
Cambridge Street
Burlington, Massachusetts

Prepared By:

TRC Environmental Corporation
650 Suffolk Street
Lowell, Massachusetts 01854





650 Suffolk St., Suite 200
Lowell, MA 01854

T. 978.970.5600
TRCcompanies.com

January 5, 2026

Burlington Conservation Commission
29 Center Street
Burlington, MA 01803

RE: Notice of Intent (NOI)
Foxboro Learning Early Childhood Education Center
"0" Cambridge Street, Burlington, MA

Dear Commissioners:

On behalf of Foxboro Learning, LLC, (FLL), TRC Environmental Corporation (TRC) is pleased to submit the enclosed Notice of Intent (NOI) to develop the site located at "0" Cambridge Street in Burlington, Massachusetts (the Site; Parcel 35-115-0) into an early childhood education center, including an associated building and site appurtenances (the Project).

This NOI is being filed with the Burlington Conservation Commission (BCC) because a portion of Project is within wetland resource areas including the 100-foot Buffer Zone under the Wetlands Protection Act (WPA; 310 CMR 10.00 *et seq.*), as well as streams, bank, land under water bodies (LUW) and the associated 100-foot Buffer Zone and 20-foot No Disturb/Erosion Control Boundary under the Bylaw (Article XIV and its implemented regulations). The Project, as proposed, will involve the rerouting of three streams jurisdictional under the Bylaw only.

We trust that the enclosed information meets the requirements of the BCC to issue an Order of Conditions (OOC) for the proposed Project and appreciate your review of this information. If you should have any questions about this NOI, please do not hesitate to contact Jeremy Foote at 401-578-1708 or via email at JFoote@trccompanies.com or Ryan Clapp at 781-701-1353 or via email at RClapp@trccompanies.com.

Sincerely,

TRC Environmental Corporation


Jeremy Foote
Senior Wetland Scientist/Project Manager


Ryan Clapp
Wetland Scientist

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ACRONYM AND ABBREVIATIONS LIST

ACEC	Areas of Critical Environmental Concern
BCC	Burlington Conservation Commission
BLSF	Bordering Land Subject to Flooding
BMP	Best Management Practice
BVW	Bordering Vegetated Wetland
CMR	Code of Massachusetts Regulations
EPA	Environmental Protection Agency
ESS	ESS Group, LLC
FEMA	Federal Emergency Management Agency
FIRM	Flood Insurance Rate Maps
FLL	Foxboro Learning, LLC
LF	Linear Feet
LUW	Land Under Wetlands and Waterbodies
MA	Massachusetts
MAHW	Mean Annual High Water
MassDEP	Massachusetts Department of Environmental Protection
MCP	Massachusetts Contingency Plan
MassGIS	Massachusetts Geographic Information System
MCP	Massachusetts Contingency Plan
M.G.L.	Massachusetts General Laws
NHESP	Natural Heritage and Endangered Species Program
NOI	Notice of Intent
NOR	Notice of Responsibility
NPDES	National Pollutant Discharge Elimination System
NRCS	Natural Resources Conservation Service
OMM	Operation, Maintenance, and Monitoring
OOC	Order of Conditions
ORW	Outstanding Resource Water
O&M	Operation and Maintenance
Project	Development of "0" Cambridge Street
RA	Riverfront Area
SF	Square Feet
Site	"0" Cambridge Street, Burlington, MA
SWPPP	Stormwater Pollution Prevention Plan
TRC	TRC Environmental Corporation
US	United States
WPA	Wetlands Protection Act

WPA FORM 3 – NOTICE OF INTENT



Massachusetts Department of Environmental Protection
Bureau of Resource Protection - Wetlands

WPA Form 3 – Notice of Intent

Massachusetts Wetlands Protection Act M.G.L. c. 131, §40

Provided by MassDEP:

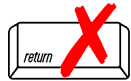
MassDEP File Number

Document Transaction Number

City/Town

Important:

When filling out forms on the computer, use only the tab key to move your cursor - do not use the return key.



Note:
Before completing this form consult your local Conservation Commission regarding any municipal bylaw or ordinance.

A. General Information

1. Project Location (**Note:** electronic filers will click on button to locate project site):

a. Street Address

b. City/Town

c. Zip Code

Latitude and Longitude:

d. Latitude

e. Longitude

f. Assessors Map/Plat Number

g. Parcel /Lot Number

2. Applicant:

a. First Name

b. Last Name

c. Organization

d. Street Address

e. City/Town

f. State

g. Zip Code

h. Phone Number

i. Fax Number

j. Email Address

3. Property owner (required if different from applicant): ☐ Check if more than one owner

a. First Name

b. Last Name

c. Organization

d. Street Address

e. City/Town

f. State

g. Zip Code

h. Phone Number

i. Fax Number

j. Email address

4. Representative (if any):

a. First Name

b. Last Name

c. Company

d. Street Address

e. City/Town

f. State

g. Zip Code

h. Phone Number

i. Fax Number

j. Email address

5. Total WPA Fee Paid (from NOI Wetland Fee Transmittal Form):

a. Total Fee Paid

b. State Fee Paid

c. City/Town Fee Paid



Massachusetts Department of Environmental Protection
Bureau of Resource Protection - Wetlands

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A. General Information (continued)

6. General Project Description:

**Construction of an early childhood education center, including utilities and associated facilities.
Several streams will need to be rerouted as a part of this project.**

7a. Project Type Checklist: (Limited Project Types see Section A. 7b.)

- | | |
|---|---|
| 1. <input type="checkbox"/> Single Family Home | 2. <input type="checkbox"/> Residential Subdivision |
| 3. <input type="checkbox"/> Commercial/Industrial | 4. <input type="checkbox"/> Dock/Pier |
| 5. <input type="checkbox"/> Utilities | 6. <input type="checkbox"/> Coastal engineering Structure |
| 7. <input type="checkbox"/> Agriculture (e.g., cranberries, forestry) | 8. <input type="checkbox"/> Transportation |
| 9. <input type="checkbox"/> Other | |

7b. Is any portion of the proposed activity eligible to be treated as a limited project (including Ecological Restoration Limited Project) subject to 310 CMR 10.24 (coastal) or 310 CMR 10.53 (inland)?

1. ☐ Yes ☐ No If yes, describe which limited project applies to this project. (See 310 CMR 10.24 and 10.53 for a complete list and description of limited project types)

2. Limited Project Type

If the proposed activity is eligible to be treated as an Ecological Restoration Limited Project (310 CMR 10.24(8), 310 CMR 10.53(4)), complete and attach Appendix A: Ecological Restoration Limited Project Checklist and Signed Certification.

8. Property recorded at the Registry of Deeds for:

a. County

b. Certificate # (if registered land)

c. Book

d. Page Number

B. Buffer Zone & Resource Area Impacts (temporary & permanent)

- ☐ Buffer Zone Only – Check if the project is located only in the Buffer Zone of a Bordering Vegetated Wetland, Inland Bank, or Coastal Resource Area.
- ☐ Inland Resource Areas (see 310 CMR 10.54-10.58; if not applicable, go to Section B.3, Coastal Resource Areas).

Check all that apply below. Attach narrative and any supporting documentation describing how the project will meet all performance standards for each of the resource areas altered, including standards requiring consideration of alternative project design or location.



Massachusetts Department of Environmental Protection
Bureau of Resource Protection - Wetlands

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B. Buffer Zone & Resource Area Impacts (temporary & permanent) (cont'd)

For all projects affecting other Resource Areas, please attach a narrative explaining how the resource area was delineated.

Resource Area	Size of Proposed Alteration	Proposed Replacement (if any)
a. <input type="checkbox"/> Bank	1. linear feet	2. linear feet
b. <input type="checkbox"/> Bordering Vegetated Wetland	1. square feet	2. square feet
c. <input type="checkbox"/> Land Under Waterbodies and Waterways	1. square feet 3. cubic yards dredged	2. square feet

Resource Area	Size of Proposed Alteration	Proposed Replacement (if any)
d. <input type="checkbox"/> Bordering Land Subject to Flooding	1. square feet 3. cubic feet of flood storage lost	2. square feet 4. cubic feet replaced
e. <input type="checkbox"/> Isolated Land Subject to Flooding	1. square feet 2. cubic feet of flood storage lost	3. cubic feet replaced
f. <input type="checkbox"/> Riverfront Area	1. Name of Waterway (if available) - specify coastal or inland	

2. Width of Riverfront Area (check one):

- ☐ 25 ft. - Designated Densely Developed Areas only
- ☐ 100 ft. - New agricultural projects only
- ☐ 200 ft. - All other projects

3. Total area of Riverfront Area on the site of the proposed project: _____ square feet

4. Proposed alteration of the Riverfront Area:

a. total square feet _____ b. square feet within 100 ft. _____ c. square feet between 100 ft. and 200 ft. _____

5. Has an alternatives analysis been done and is it attached to this NOI? ☐ Yes ☐ No

6. Was the lot where the activity is proposed created prior to August 1, 1996? ☐ Yes ☐ No

3. ☐ Coastal Resource Areas: (See 310 CMR 10.25-10.35)

Note: for coastal riverfront areas, please complete **Section B.2.f.** above.



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B. Buffer Zone & Resource Area Impacts (temporary & permanent) (cont'd)

Check all that apply below. Attach narrative and supporting documentation describing how the project will meet all performance standards for each of the resource areas altered, including standards requiring consideration of alternative project design or location.

Online Users:
Include your document transaction number (provided on your receipt page) with all supplementary information you submit to the Department.

<u>Resource Area</u>	<u>Size of Proposed Alteration</u>	<u>Proposed Replacement (if any)</u>
a. <input type="checkbox"/> Designated Port Areas	Indicate size under Land Under the Ocean, below	
b. <input type="checkbox"/> Land Under the Ocean	1. square feet	
	2. cubic yards dredged	
c. <input type="checkbox"/> Barrier Beach	Indicate size under Coastal Beaches and/or Coastal Dunes below	
d. <input type="checkbox"/> Coastal Beaches	1. square feet	2. cubic yards beach nourishment
e. <input type="checkbox"/> Coastal Dunes	1. square feet	2. cubic yards dune nourishment
	<u>Size of Proposed Alteration</u>	<u>Proposed Replacement (if any)</u>
f. <input type="checkbox"/> Coastal Banks	1. linear feet	
g. <input type="checkbox"/> Rocky Intertidal Shores	1. square feet	
h. <input type="checkbox"/> Salt Marshes	1. square feet	2. sq ft restoration, rehab., creation
i. <input type="checkbox"/> Land Under Salt Ponds	1. square feet	
	2. cubic yards dredged	
j. <input type="checkbox"/> Land Containing Shellfish	1. square feet	
k. <input type="checkbox"/> Fish Runs	Indicate size under Coastal Banks, inland Bank, Land Under the Ocean, and/or inland Land Under Waterbodies and Waterways, above	
	1. cubic yards dredged	
l. <input type="checkbox"/> Land Subject to Coastal Storm Flowage	1. square feet	

4. ☐ Restoration/Enhancement

If the project is for the purpose of restoring or enhancing a wetland resource area in addition to the square footage that has been entered in Section B.2.b or B.3.h above, please enter the additional amount here.

a. square feet of BVW

b. square feet of Salt Marsh

5. ☐ Project Involves Stream Crossings

a. number of new stream crossings

b. number of replacement stream crossings



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C. Other Applicable Standards and Requirements

- ☐ This is a proposal for an Ecological Restoration Limited Project. Skip Section C and complete Appendix A: Ecological Restoration Limited Project Checklists – Required Actions (310 CMR 10.11).

Streamlined Massachusetts Endangered Species Act/Wetlands Protection Act Review

1. Is any portion of the proposed project located in **Estimated Habitat of Rare Wildlife** as indicated on the most recent Estimated Habitat Map of State-Listed Rare Wetland Wildlife published by the Natural Heritage and Endangered Species Program (NHESP)? To view habitat maps, see the *Massachusetts Natural Heritage Atlas* or go to http://maps.massgis.state.ma.us/PRI_EST_HAB/viewer.htm.

a. ☐ Yes ☐ No

If yes, include proof of mailing or hand delivery of NOI to:

**Natural Heritage and Endangered Species Program
Division of Fisheries and Wildlife
1 Rabbit Hill Road
Westborough, MA 01581**

b. Date of map

If yes, the project is also subject to Massachusetts Endangered Species Act (MESA) review (321 CMR 10.18). To qualify for a streamlined, 30-day, MESA/Wetlands Protection Act review, please complete Section C.1.c, and include requested materials with this Notice of Intent (NOI); *OR* complete Section C.2.f, if applicable. *If MESA supplemental information is not included with the NOI, by completing Section 1 of this form, the NHESP will require a separate MESA filing which may take up to 90 days to review (unless noted exceptions in Section 2 apply, see below).*

- c. Submit Supplemental Information for Endangered Species Review*

1. ☐ Percentage/acreage of property to be altered:

(a) within wetland Resource Area

percentage/acreage

(b) outside Resource Area

percentage/acreage

2. ☐ Assessor's Map or right-of-way plan of site

2. ☐ Project plans for entire project site, including wetland resource areas and areas outside of wetlands jurisdiction, showing existing and proposed conditions, existing and proposed tree/vegetation clearing line, and clearly demarcated limits of work **

(a) ☐ Project description (including description of impacts outside of wetland resource area & buffer zone)

(b) ☐ Photographs representative of the site

* Some projects **not** in Estimated Habitat may be located in Priority Habitat, and require NHESP review (see <https://www.mass.gov/mas-endangered-species-act-mesa-regulatory-review>).

Priority Habitat includes habitat for state-listed plants and strictly upland species not protected by the Wetlands Protection Act.

** MESA projects may not be segmented (321 CMR 10.16). The applicant must disclose full development plans even if such plans are not required as part of the Notice of Intent process.



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C. Other Applicable Standards and Requirements (cont'd)

- (c) ☐ MESA filing fee (fee information available at <https://www.mass.gov/how-to/how-to-file-for-a-mesa-project-review>).

Make check payable to "Commonwealth of Massachusetts - NHESP" and **mail to NHESP** at above address

Projects altering 10 or more acres of land, also submit:

- (d) ☐ Vegetation cover type map of site
- (e) ☐ Project plans showing Priority & Estimated Habitat boundaries
- (f) OR Check One of the Following

1. ☐ Project is exempt from MESA review.
Attach applicant letter indicating which MESA exemption applies. (See 321 CMR 10.14, <https://www.mass.gov/service-details/exemptions-from-review-for-projectsactivities-in-priority-habitat>; the NOI must still be sent to NHESP if the project is within estimated habitat pursuant to 310 CMR 10.37 and 10.59.)

2. ☐ Separate MESA review ongoing. a. NHESP Tracking # _____ b. Date submitted to NHESP _____

3. ☐ Separate MESA review completed.
Include copy of NHESP "no Take" determination or valid Conservation & Management Permit with approved plan.

3. For coastal projects only, is any portion of the proposed project located below the mean high water line or in a fish run?

- a. ☐ Not applicable – project is in inland resource area only b. ☐ Yes ☐ No

If yes, include proof of mailing, hand delivery, or electronic delivery of NOI to either:

South Shore - Bourne to Rhode Island border, and the Cape & Islands:

North Shore - Plymouth to New Hampshire border:

Division of Marine Fisheries -
Southeast Marine Fisheries Station
Attn: Environmental Reviewer
836 South Rodney French Blvd.
New Bedford, MA 02744
Email: dmf.envreview-south@mass.gov

Division of Marine Fisheries -
North Shore Office
Attn: Environmental Reviewer
30 Emerson Avenue
Gloucester, MA 01930
Email: dmf.envreview-north@mass.gov

Also if yes, the project may require a Chapter 91 license. For coastal towns in the Northeast Region, please contact MassDEP's Boston Office. For coastal towns in the Southeast Region, please contact MassDEP's Southeast Regional Office.

- c. ☐ Is this an aquaculture project? d. ☐ Yes ☐ No

If yes, include a copy of the Division of Marine Fisheries Certification Letter (M.G.L. c. 130, § 57).



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WPA Form 3 – Notice of Intent

Massachusetts Wetlands Protection Act M.G.L. c. 131, §40

Provided by MassDEP:

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City/Town

C. Other Applicable Standards and Requirements (cont'd)

Online Users:

Include your document transaction number (provided on your receipt page) with all supplementary information you submit to the Department.

4. Is any portion of the proposed project within an Area of Critical Environmental Concern (ACEC)?
a. ☐ Yes ☐ No If yes, provide name of ACEC (see instructions to WPA Form 3 or MassDEP Website for ACEC locations). **Note:** electronic filers click on Website.
- b. ACEC
5. Is any portion of the proposed project within an area designated as an Outstanding Resource Water (ORW) as designated in the Massachusetts Surface Water Quality Standards, 314 CMR 4.00?
a. ☐ Yes ☐ No
6. Is any portion of the site subject to a Wetlands Restriction Order under the Inland Wetlands Restriction Act (M.G.L. c. 131, § 40A) or the Coastal Wetlands Restriction Act (M.G.L. c. 130, § 105)?
a. ☐ Yes ☐ No
7. Is this project subject to provisions of the MassDEP Stormwater Management Standards?
a. ☐ Yes. Attach a copy of the Stormwater Report as required by the Stormwater Management Standards per 310 CMR 10.05(6)(k)-(q) and check if:
1. ☐ Applying for Low Impact Development (LID) site design credits (as described in Stormwater Management Handbook Vol. 2, Chapter 3)
2. ☐ A portion of the site constitutes redevelopment
3. ☐ Proprietary BMPs are included in the Stormwater Management System.
b. ☐ No. Check why the project is exempt:
1. ☐ Single-family house
2. ☐ Emergency road repair
3. ☐ Small Residential Subdivision (less than or equal to 4 single-family houses or less than or equal to 4 units in multi-family housing project) with no discharge to Critical Areas.

D. Additional Information

- ☐ This is a proposal for an Ecological Restoration Limited Project. Skip Section D and complete Appendix A: Ecological Restoration Notice of Intent – Minimum Required Documents (310 CMR 10.12).

Applicants must include the following with this Notice of Intent (NOI). See instructions for details.

Online Users: Attach the document transaction number (provided on your receipt page) for any of the following information you submit to the Department.

1. ☐ USGS or other map of the area (along with a narrative description, if necessary) containing sufficient information for the Conservation Commission and the Department to locate the site. (Electronic filers may omit this item.)
2. ☐ Plans identifying the location of proposed activities (including activities proposed to serve as a Bordering Vegetated Wetland [BVW] replication area or other mitigating measure) relative to the boundaries of each affected resource area.



Massachusetts Department of Environmental Protection
Bureau of Resource Protection - Wetlands

WPA Form 3 – Notice of Intent

Massachusetts Wetlands Protection Act M.G.L. c. 131, §40

Provided by MassDEP:

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D. Additional Information (cont'd)

3. ☐ Identify the method for BVW and other resource area boundary delineations (MassDEP BVW Field Data Form(s), Determination of Applicability, Order of Resource Area Delineation, etc.), and attach documentation of the methodology.
4. ☐ List the titles and dates for all plans and other materials submitted with this NOI.

a. Plan Title

b. Prepared By

c. Signed and Stamped by

d. Final Revision Date

e. Scale

f. Additional Plan or Document Title

g. Date

5. ☐ If there is more than one property owner, please attach a list of these property owners not listed on this form.
6. ☐ Attach proof of mailing for Natural Heritage and Endangered Species Program, if needed.
7. ☐ Attach proof of mailing for Massachusetts Division of Marine Fisheries, if needed.
8. ☐ Attach NOI Wetland Fee Transmittal Form
9. ☐ Attach Stormwater Report, if needed.

E. Fees

1. ☐ Fee Exempt: No filing fee shall be assessed for projects of any city, town, county, or district of the Commonwealth, federally recognized Indian tribe housing authority, municipal housing authority, or the Massachusetts Bay Transportation Authority.

Applicants must submit the following information (in addition to pages 1 and 2 of the NOI Wetland Fee Transmittal Form) to confirm fee payment:

2. Municipal Check Number
1304423

3. Check date
1/28/2026

4. State Check Number
1304422

5. Check date
1/28/2026

6. Payor name on check: First Name

7. Payor name on check: Last Name



Massachusetts Department of Environmental Protection
Bureau of Resource Protection - Wetlands

WPA Form 3 – Notice of Intent

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Provided by MassDEP:

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F. Signatures and Submittal Requirements

I hereby certify under the penalties of perjury that the foregoing Notice of Intent and accompanying plans, documents, and supporting data are true and complete to the best of my knowledge. I understand that the Conservation Commission will place notification of this Notice in a local newspaper at the expense of the applicant in accordance with the wetlands regulations, 310 CMR 10.05(5)(a).

I further certify under penalties of perjury that all abutters were notified of this application, pursuant to the requirements of M.G.L. c. 131, § 40. Notice must be made by Certificate of Mailing or in writing by hand delivery or certified mail (return receipt requested) to all abutters within 100 feet of the property line of the project location.

Manoj Gandhi

1. Signature of Applicant

2. Date

3. Signature of Property Owner (if different)

4. Date

5. Signature of Representative (if any)

6. Date

For Conservation Commission:

Two copies of the completed Notice of Intent (Form 3), including supporting plans and documents, two copies of the NOI Wetland Fee Transmittal Form, and the city/town fee payment, to the Conservation Commission by certified mail or hand delivery.

For MassDEP:

One copy of the completed Notice of Intent (Form 3), including supporting plans and documents, one copy of the NOI Wetland Fee Transmittal Form, and a **copy** of the state fee payment to the MassDEP Regional Office (see Instructions) by certified mail or hand delivery.

Other:

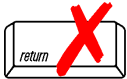
If the applicant has checked the "yes" box in any part of Section C, Item 3, above, refer to that section and the Instructions for additional submittal requirements.

The original and copies must be sent simultaneously. Failure by the applicant to send copies in a timely manner may result in dismissal of the Notice of Intent.



Massachusetts Department of Environmental Protection
Bureau of Resource Protection - Wetlands
NOI Wetland Fee Transmittal Form
Massachusetts Wetlands Protection Act M.G.L. c. 131, §40

Important: When filling out forms on the computer, use only the tab key to move your cursor - do not use the return key.



A. Applicant Information

1. Location of Project:

a. Street Address

b. City/Town

c. Check number

d. Fee amount

2. Applicant Mailing Address:

a. First Name

b. Last Name

c. Organization

d. Mailing Address

e. City/Town

f. State

g. Zip Code

h. Phone Number

i. Fax Number

j. Email Address

3. Property Owner (if different):

a. First Name

b. Last Name

c. Organization

d. Mailing Address

e. City/Town

f. State

g. Zip Code

h. Phone Number

i. Fax Number

j. Email Address

To calculate filing fees, refer to the category fee list and examples in the instructions for filling out WPA Form 3 (Notice of Intent).

B. Fees

Fee should be calculated using the following process & worksheet. **Please see Instructions before filling out worksheet.**

Step 1/Type of Activity: Describe each type of activity that will occur in wetland resource area and buffer zone.

Step 2/Number of Activities: Identify the number of each type of activity.

Step 3/Individual Activity Fee: Identify each activity fee from the six project categories listed in the instructions.

Step 4/Subtotal Activity Fee: Multiply the number of activities (identified in Step 2) times the fee per category (identified in Step 3) to reach a subtotal fee amount. Note: If any of these activities are in a Riverfront Area in addition to another Resource Area or the Buffer Zone, the fee per activity should be multiplied by 1.5 and then added to the subtotal amount.

Step 5/Total Project Fee: Determine the total project fee by adding the subtotal amounts from Step 4.

Step 6/Fee Payments: To calculate the state share of the fee, divide the total fee in half and subtract \$12.50. To calculate the city/town share of the fee, divide the total fee in half and add \$12.50.



Massachusetts Department of Environmental Protection
Bureau of Resource Protection - Wetlands
NOI Wetland Fee Transmittal Form
Massachusetts Wetlands Protection Act M.G.L. c. 131, §40

B. Fees (continued)

Step 1/Type of Activity	Step 2/Number of Activities	Step 3/Individual Activity Fee	Step 4/Subtotal Activity Fee

Step 5/Total Project Fee: _____

Step 6/Fee Payments:

Total Project Fee: _____ a. Total Fee from Step 5

State share of filing Fee: _____ b. 1/2 Total Fee **less** \$12.50

City/Town share of filing Fee: _____ c. 1/2 Total Fee **plus** \$12.50

C. Submittal Requirements

- a.) Complete pages 1 and 2 and send with a check or money order for the state share of the fee, payable to the Commonwealth of Massachusetts.

Department of Environmental Protection
Box 4062
Boston, MA 02211

- b.) **To the Conservation Commission:** Send the Notice of Intent or Abbreviated Notice of Intent; a **copy** of this form; and the city/town fee payment.

To MassDEP Regional Office (see Instructions): Send a copy of the Notice of Intent or Abbreviated Notice of Intent; a **copy** of this form; and a **copy** of the state fee payment. (E-filers of Notices of Intent may submit these electronically.)

ABUTTER INFORMATION

Current Owner
108 CAMBRIDGE ST
BURLINGTON, MA 1803

Current Owner
200 WEST CUMMINGS PK
WOBURN, MA 1801

Current Owner
109 CAMBRIDGE ST
BURLINGTON, MA 1803

Current Owner
1 ANNA RD
BURLINGTON, MA 1803

Current Owner
3 ANNA RD
BURLINGTON, MA 1803

Current Owner
7 ANNA RD
BURLINGTON, MA 1803

Current Owner
30 ARLINGTON RD
BURLINGTON, MA 1803

Current Owner
28 ARLINGTON RD
BURLINGTON, MA 1803

Current Owner
29 CENTER ST
BURLINGTON, MA 1803

Current Owner
539 SOUTH MAIN ST
FINDLAY, OH 45840

Notification to Abutters

By Hand Delivery, Certified Mail (return receipt requested), or Certificates of Mailing

This is a notification required by law. You are receiving this notification because you have been identified as the owner of land abutting another parcel of land for which certain activities are proposed. Those activities require a permit under the Massachusetts Wetlands Protection Act (M.G.L. c. 131, § 40).

In accordance with the second paragraph of the Massachusetts Wetlands Protection Act, and 310 CMR 10.05(4)(a) of the Wetlands Regulations, you are hereby notified that:

- A. A Notice of Intent was filed with the Burlington Conservation Commission on _____ seeking permission to remove, fill, dredge, or alter an area subject to protection under M.G.L. c. 131 §40. The following is a description of the proposed activity/activities:

Construction of an early childhood education center, including associated utilities.

- B. The name of the applicant is: Foxboro Learning, LLC.
- C. The address of the land where the activity is proposed is: “0” Cambridge Street (Parcel ID: 35-115-0), Burlington, MA.
- D. Copies of the Notice of Intent may be examined or obtained at the office of the Burlington Conservation Commission, located at 29 Center Street, Burlington, MA. The regular business hours of the Commission are Monday, Tuesday, Thursday: 8:30 AM – 4:30 PM; Wednesday: 8:30 AM – 7:00 PM; Friday: 8:30 AM – 1:00 PM, and the Commission may be reached at 781-270-1655.
- E. Copies of the Notice of Intent may be obtained from the applicant or their representative by calling Ryan Clapp, TRC Environmental Corporation at 781-701-1353. An administrative fee may be applied for providing copies of the NOI and plans.
- F. Information regarding the date, time, and location of the public hearing regarding the Notice of Intent may be obtained from the Burlington Conservation Commission. Notice of the public hearing will be published at least five business days in advance, in a locally-circulated newspaper.

Notification provided pursuant to the above requirement does not automatically confer standing to the recipient to request Departmental Action for the underlying matter. See 310 CMR 10.05(7)(a)4.

PROJECT NARRATIVE

1.0 INTRODUCTION

On behalf of Foxboro Learning, LLC, (FLL) TRC Environmental Corporation (TRC) is submitting a Notice of Intent (NOI) to the Burlington Conservation Commission (BCC) for site development in support of an early childhood education center, including an associated building and site appurtenances. The Project is expected to occur within the property boundary of "0" Cambridge Street in Burlington, Massachusetts (Project Site; Burlington Parcel ID No. 35-115-0).

1.1 Project History

1.1.1 Site Location and Description

The Project Site consists of one parcel with a land area of approximately 3.7 acres, located in Burlington, Massachusetts. The Project Site is bordered by single-family residences along Anna Road to the north, Cambridge Street to the east, a business park to the south, and a business park to the west. (**Attachment A, Figure 1**). According to a Town of Burlington zoning map, the Project Site is zoned as "RO" (one-family dwelling).

The Project Site is currently undeveloped woodland, with no buildings or site improvements built upon it.

1.1.2 Resource Area Degradation and Restoration

In early 2025, unauthorized heavy machinery was brought onto the Site, heavily disturbing and degrading several resource areas jurisdictional under the Act and Bylaw, as well as the 100-foot Buffer Zone to wetlands. Accordingly, the BCC issued an Enforcement Order, requiring that the resource areas (not Buffer Zone), be restored by hand without the use of additional machinery.

On August 5, 2025, TRC staff under the supervision of a TRC wetland scientist accessed the site to support FLL in resolving the resource area degradation. Using hand tools, the team excavated portions of the stream bed, as well as filled in and restored the banks and channels of the various disturbed streams. Following this work, the area was seeded with New England Erosion Control/Restoration Mix for Detention Basins and Moist Sites, provided by New England Wetland Plants, Inc., and straw was placed down for stabilization. As of the date of this NOI, the Enforcement Order is still in effect, pending final inspection by the BCC.

1.2 Conservation Commission Jurisdiction

FLL is filing this NOI with the BCC pursuant to the Massachusetts Wetland Protection Act (WPA; M.G.L. c. 131, § 40) and its Regulations (310 CMR 10.00) and the Town of Burlington Wetland Bylaw (Article XIV and its implemented regulations). While the Project has been designed to limit impacts to wetland resource areas to the maximum extent practicable, the proposed Work is located within Inland Bank, Streams and 20-foot No Disturb/Erosion Control Boundary (Article XIV) as well as the associated 100-foot Buffer Zone (Article XIV; WPA).

2.0 PROJECT SITE DESCRIPTION AND EXISTING CONDITIONS

The Project Site spans a portion of one parcel, where the focus of Work is towards the east of the parcel. The Project Site is undeveloped with no buildings or site improvements upon it. To the west and south of the parcel is a Bordering Vegetated Wetland (BVW) identified as W-RPC-01. This wetland is a Palustrine Forested Wetland (PFO) as defined by the ACOE. Flowing into this wetland, resulting from culverting and sheet flow from Cambridge Street to the east, are four intermittent streams identified as S-RPC-01, S-RPC-02, S-RPC-03 and S-RPC-04. These streams largely lack structure and alternate between a semi-defined stream channel and sheet flow throughout their course.

The surrounding land has been highly developed for commercial and residential use, with various residences and businesses to the north, south, east and west. The Project Site is mapped as a mix of Montauk Fine Sandy Loam, 3 to 8 percent slopes (300B) and Udorthents – Urban land complex (656) by the Natural Resources Conservation Service (NRCS) Web Soil Survey (NRCS 2023¹). The Project Site is located within the Shawsheen River watershed, which drains to the north towards the Merrimack River (MassMapper 2023²).

According to Federal Emergency Management Area (FEMA) Flood Insurance Rate Map (FIRM) Panel 25017C0402F (effective date July 8, 2025), the Project Site is not located within any flood hazard layers. Therefore, Bordering Land Subject to Flooding (BLSF) is not located within the Project Site (**Attachment A, Figure 3**).

2.1 Jurisdictional Resource Areas

Multiple resource areas, as defined in the WPA and described in the following sections, exist at the Project Site. In October of 2018, Oxbow Associates (OA) delineated the BVW present onsite. An Order of Resource Area Delineation (ORAD) was issued by the BCC in October 2018, which has since been extended and still is valid. At the time of issuance of the ORAD, marginal references were made to the streams present onsite, but not formally delineated. On May 20, 2025, these streams were delineated by TRC following the unauthorized site disturbance. Notably, these streams were significantly different from the references made in 2018. Whether that is the result of natural change over time or the site disturbance is unknown.

Wetland W-RPC-01 receives a 100-Foot Buffer Zone under the WPA and a 20-Foot No Disturbance Zone under the Bylaw. Streams S-RPC-(01-04) receive a 100-Foot Buffer Zone and 20-Foot No Disturbance Zone.

See **Attachment A, Figure 3** for a Resources Map. Refer to Section 4.0 herein for further details about impacts to each resource area associated with the proposed Project.

2.1.1 Bank and Stream

Per 310 CMR 10.54(2), Bank is “*the portion of the land surface which normally abuts and confines a water body. It occurs between a water body and a vegetated bordering wetland and adjacent*

¹ NRCS (Natural Resources Conservation Service). 2023. Soil Survey Staff, United States Department of Agriculture, Web Soil Survey.

² Commonwealth of Massachusetts, MassGIS Bureau of Geographic Information. 2023. MassMapper. <https://maps.massgis.digital.mass.gov/MassMapper/MassMapper.html>

flood plain, or, in the absence of these, it occurs between a water body and an upland.”

Per 310 CMR 10.04, a stream is defined as: “A body of running water, including brooks and creeks, which moves in a definite channel in the ground due to a hydraulic gradient, and which flows within, into or out of an Area Subject to Protection under MGL c. 131 s. 40...”

Bank at the Project Site is associated with delineated intermittent streams S-RPC-01, 02, 03, and 04. These streams have limited flow and are generally considered historic drainage channels through the site. At several locations, these streams lose definition and become sheet flow before coalescing back into a channel.

2.1.2 Buffer Zones

Per 310 CMR 10.02, a buffer zone extends 100 feet outward from Bank and BVW, whichever abuts the upland portion of the Project Site.

Buffer zone exists along the Inland Bank and BVW throughout the Project Site as shown in **Attachment A, Figure 3**.

The proposed Project includes impacts to buffer zone associated with the stabilization measure installation, temporary access and other ground disturbances for stormwater management, as described in Section 4.2.

2.1.3 20-foot No Disturbance/Erosion Control Distance (Town of Burlington Wetland Bylaw)

Per Section 21.9 of the Town of Burlington Wetland Protection Regulations, the inner 20' of the 100-foot Buffer Zone is regulated as a No-Disturbance Distance. The 20-foot No-Disturbance Distance exists along the Bank and BVW throughout the project as shown in **Attachment A**. The proposed Project includes impacts to the 20-foot no disturbance distance.

Per Section 21.9(c)(2), “These presumptions are rebuttable and may be overcome upon a clear showing that the nature of the proposed work, special design measures, construction controls, or site conditions will prevent alteration of the resource area. Furthermore, per Section 21.9(c)(4), “These presumptions are rebuttable and may be overcome when the nature of the work or site conditions will result in alteration of the resource area unless special preventative measures are taken.” While alteration of the resource areas will be permanently altered by the Project as designed, special preventative measures including the 1:1 restoration of altered resource area will be taken, as described in Section 3.2.3 below.

2.1.4 Additional Regulatory Information

The Project Site is not located within an Outstanding Resource Water (ORW), an Area of Critical Environmental Concern (ACEC), Natural Heritage Endangered Species Program (NHESP) Estimated or Priority Habitats of Rare Species, or proximal to any Certified or Potential Vernal Pools according to MassGIS (**Attachment A, Figure 3**).

3.0 PROPOSED PROJECT DESCRIPTION

Details of the proposed Project and Project Work are provided below. Project Work will be performed by a contractor (the Contractor) retained by FLL. TRC will oversee the Work and provide direction, as necessary.

3.1 Site Access, Construction Entrances, & Staging Areas

Access to the Project Site will be from Cambridge Street to the east. The proposed Project will need to remove trees and alter the grade to allow for machinery to access the Site (**Attachment B**).

Erosion and sediment controls will be established prior to any earthwork and will involve siltation fencing, straw wattles and river dewatering. See Section 5.1 for further details on the erosion and sediment controls proposed.

During this phase of the proposed Project, all erosion and sediment controls, soil stabilization materials, drainage, and geotextile fabrics will be installed by the selected Contractor.

3.2 Proposed Work Description

3.2.1 Land Clearing, Grubbing & Demolition

Significant grading, land clearing and demolition are necessary to provide access to the Site, as well as to prepare the Site for suitable use as an early childhood education center. The proposed limits of soil disturbance are outlined in the Project Plans (**Attachment B**).

All efforts will be made to locate soil stockpiles outside of the 100-Foot Buffer Zone. In the event that stockpiling must be performed in the Buffer Zone, the stockpiles will be surrounded by erosion control barriers for stabilization and protection of resource areas when not in use.

3.2.2 Learning Center (Building, Ancillary Structures, Landscaping)

Once Site preparation is completed, work will begin to construct the learning center. This center will be composed of a 2-story building with an approximately 5,500 SF footprint. Additionally, there will be an asphalt parking area with approximately 33 spaces, a trash enclosure, and retaining wall and guardrails. Also proposed is an approximately 3,480 SF playground and associated concrete walkway, gates and fencing.

The Site will also have electrical, gas, sewer and other utilities installed to serve the center.

A stormwater management system consisting of a plunge pool, the rerouted stream, and rip-rap aprons designed to filter and slow stormwater entering and leaving the site will also be constructed within the Buffer Zone.

3.2.3 Stream Rerouting

To make use of the Site, FLL will need to reroute the four streams onsite to create suitable upland for the proposed building, play area, and ancillary structures. In order to do so, a new stream bed

will be excavated to meet an equivalent square footage of stream to be filled. The soils from the stream will be screened and tested for contaminants before being used to fill the existing stream beds.

The newly excavated stream bed will be seeded with an appropriate seed mix and stabilized. All work to reroute the streams onsite will be performed under the supervision of a wetland scientist or other such professional under the guidance and in accordance with the requirements of the BCC.

3.3 Construction Timeline

1. Pre-Construction and Environmental Protection Measures

Prior to the commencement of any land disturbing activities, the General Contractor will conduct a pre-construction coordination meeting with the project team to review approved plans, erosion and sedimentation control requirements, and construction sequencing.

All required erosion and sedimentation controls will be installed and inspected in accordance with the approved Order of Conditions prior to earth disturbance.

2. Construction Access and Site Logistics

A designated construction access point will be established and maintained throughout the project, including a stabilized construction entrance with track pad.

3. Earthwork, Excavation and Utilities

Earthwork and utility installation will proceed in a phased manner to limit exposed soils.

4. Building Construction Activities

Vertical construction will commence following completion of foundations and underground work.

5. Site Improvements and Final Stabilization

Final grading, paving, landscaping and permanent stabilization will be completed per approved plans.

6. Inspection, Maintenance and Compliance

Erosion controls will be inspected weekly and after significant rainfall events.

7. Site Demobilization

Temporary controls will be removed following permanent stabilization and Conservation approval.

4.0 PROJECT IMPACTS AND PERFORMANCE STANDARDS

The proposed Project will result in temporary and permanent impacts to jurisdictional resource areas as described in Table 1:

Table 1. Project Impacts

RESOURCE AREA	IMPACTS	IMPACT DESCRIPTION
Inland Bank	936 LF	<ul style="list-style-type: none"> Permanent impacts from the grading and site preparation, as well as construction of Early Childhood Education Center and associated structures and utilities. Rerouting of intermittent streams.
Stream	468 LF	<ul style="list-style-type: none"> Permanent impacts from the grading and site preparation, as well as construction of Early Childhood Education Center and associated structures and utilities. Rerouting of intermittent streams.
20-foot Undisturbed Buffer	46,380 SF	<ul style="list-style-type: none"> Permanent impacts from the grading and site preparation, as well as construction of Early Childhood Education Center and associated structures and utilities. Rerouting of intermittent streams.
50-foot Buffer Zone	46,712 SF	<ul style="list-style-type: none"> Permanent impacts from the grading and site preparation, as well as construction of Early Childhood Education Center and associated structures and utilities. Rerouting of intermittent streams.
100-foot Buffer Zone	54,075	<ul style="list-style-type: none"> Permanent impacts from the grading and site preparation, as well as construction of Early Childhood Education Center and associated structures and utilities. Rerouting of intermittent streams.

4.1 Inland Bank

While the Project has been designed to limit impacts to the Inland Bank to the maximum extent possible, unavoidable permanent impacts to the Bank of the four intermittent streams are required due to site constraints. TRC has evaluated the performance standards assuming the rerouting of the four intermittent streams into one large channel offsets the impacts from the loss of the four streams by meeting the performance standards in the newly established stream. The proposed Work complies to the maximum extent practicable with the WPA performance standards for Bank as described in Table 2 below:

Table 2. WPA Performance Standards for Inland Bank*

PERFORMANCE STANDARDS 310 CMR 10.54(4)	PROJECT'S COMPLIANCE WITH PERFORMANCE STANDARD
<p>(a) Where the presumption set forth in 310 CMR 10.54(3) is not overcome, any proposed Work on a Bank shall not impair the following:</p> <ol style="list-style-type: none"> 1. the physical stability of the Bank; 2. the water carrying capacity of the existing channel within the Bank; 3. ground water and surface water quality; 4. the capacity of the Bank to provide breeding habitat, escape cover and food for fisheries; 5. the capacity of the Bank to provide important wildlife habitat functions. A project or projects on a single lot, for which Notice(s) of Intent is filed on or after November 1, 1987, that (cumulatively) alter(s) up to 10% or 50 feet (whichever is less) of the length of the bank found to be significant to the protection of wildlife habitat, shall not be deemed to impair its capacity to provide important wildlife habitat functions. In the case of a bank of a river or an intermittent stream, the impact shall be measured on each side of the stream or river. Additional alterations beyond the above threshold may be permitted if they will have no adverse effects on wildlife habitat, as determined by procedures contained in 310 CMR 10.60. 6. Work on a stream crossing shall be presumed to meet the performance standard set forth in 310 CMR 10.54(4)(a) provided the Work is performed in compliance with the Massachusetts Stream Crossing Standards by consisting of a span or embedded culvert in which, at a minimum, the bottom of a span structure or the upper surface of an embedded culvert is above the elevation of the top of the bank, and the structure spans the channel width by a minimum of 1.2 times the bankfull width. This presumption is rebuttable and may be overcome by the submittal of credible evidence from a competent source. Notwithstanding the requirement of 310 CMR 10.54(4)(a)5., the impact on bank caused by the installation of a stream crossing is exempt from the requirement to perform a 	<p>(a) The proposed Work on Bank will abide by the performance standards to the greatest extent practicable:</p> <ol style="list-style-type: none"> 1. The proposed Work will not impair the physical stability of the Bank and will improve the stability of the Bank. 2. The proposed Work will not impair the water carrying capacity of the existing channel within the Bank. 3. The proposed Work will not impair the ground water and surface water quality of the Bank. 4. The proposed Work will not impair the capacity of the Bank to provide important wildlife functions. See Attachment D for wildlife habitat evaluation. 5. See previous. The proposed Work will not impair the capacity of the Bank to provide important wildlife functions. 6. There are no proposed permanent stream crossings and our construction methodology addresses temporary stream access.

Table 2. WPA Performance Standards for Inland Bank*

PERFORMANCE STANDARDS 310 CMR 10.54(4)	PROJECT'S COMPLIANCE WITH PERFORMANCE STANDARD
habitat evaluation in accordance with the procedures contained in 310 CMR 10.6.	
(b) Notwithstanding the provisions of 310 CMR 10.54(4)(a), structures may be permitted in or on a Bank when required to prevent flood damage to facilities, buildings and roads constructed prior to the effective date of 310 CMR 10.51 through 10.60 or constructed pursuant to a Notice of Intent filed prior to the effective date of 310 CMR 10.51 through 10.60 (April 1, 1983), including the renovation or reconstruction (but not substantial enlargement) of such facilities, buildings and roads, provided that the following requirements are met: 1. The proposed protective structure, renovation or reconstruction is designed and constructed using best practical measures so as to minimize adverse effects on the characteristics and functions of the resource area; 2. The applicant demonstrates that there is no reasonable method of protecting, renovating or rebuilding the facility in question other than the one proposed.	(b) Structures proposed as a part of this Project are not required to prevent flood damage. Therefore, this standard is not applicable.
(c) Notwithstanding the provisions of 310 CMR 10.54(4)(a) or (b), no project may be permitted which will have any adverse effect on specified habitat sites of Rare Species, as identified by procedures established under 310 CMR 10.59.	(c) There are no NHESP Priority or Estimated habitat areas mapped at the Project Site. Therefore, the proposed Project does not anticipate adverse effects on rare species habitat.
*Note that there are no performance standards for streams, as they do not qualify as Land Under Water. It is assumed that by meeting Bank standards, any standards for streams are also met. The definition of an intermittent stream is associated with the mean annual low water level, which for an intermittent stream is 0.	

4.2 Buffer Zone

While the WPA recognizes the importance of the buffer zone, it does not include specific performance standards. The Bylaw, however, provides several performance standards, as outlined in Table 3.

Table 3. Bylaw Performance Standards for Buffer Zone

PERFORMANCE STANDARDS 310 CMR 10.54(4)	PROJECT'S COMPLIANCE WITH PERFORMANCE STANDARD
1. Work within the Buffer Zone shall result in either no alteration of a resource area, or in	The work proposed by this Project will result in an alteration of Bank and Streams. However, by

Table 3. Bylaw Performance Standards for Buffer Zone

PERFORMANCE STANDARDS 310 CMR 10.54(4)	PROJECT'S COMPLIANCE WITH PERFORMANCE STANDARD
alteration permitted by the Commission that complies with the applicable performance standards for the resource area and any other conditions the Commission may require to enforce those performance standards	rerouting these streams, the performance standards will be met by the newly established stream.
2. All new construction projects shall meet the resource area no-disturb and building setbacks listed in the presumptions above unless the presumption is overcome.	FLL is seeking a waiver of the no-disturbance zone due to site constraints.
3. Vegetation, particularly mature trees, shall be preserved to the maximum extent possible. Where trees within the buffer zone are cut, the Commission may require plantings of new trees as mitigation	FLL will comply with revegetation of trees per the requirements of the Commission. This site is proposed as an Early Childhood Education Center, and will need a yard area for use, however, potentially limiting the amount of tree revegetation.
4. Cutting of trees in the buffer zone, other than removal of dead limbs or vista pruning, shall require the prior approval of the Conservation Department.	In filing the Notice of Intent, FLL is seeking approval for the cutting of trees.
5. Lots that were developed prior to the adoption of the 2013 Wetland Bylaw may not meet the no-disturb or building setbacks required by these regulations. The Commission may require any applicant for projects on pre-existing lots that do not meet the setbacks to increase the naturally vegetated buffer to a resource area as part of the permitting process for new construction on the lot	This lot was established in 1947, predating the 2013 Wetland Bylaw. As such, FLL is seeking a waiver of the no-disturbance zone and building setbacks.
6. The Commission may require that an applicant mitigates any tree cutting in the buffer zone by planting native tree species in at least a 1:1 ratio.	FLL is willing to offset the tree cutting proposed in this Project. However, as noted above, with the need for a yard area this may limit the amount of tree revegetation.
7. For small projects such as single-family lots, point discharge of surface runoff within or through a Buffer Zone shall be controlled to minimize increase in peak flow in the watercourse downstream of the discharge point for the runoff, as determined for the 2-year, 10-year, and 100-year storms, and to cause no increase in flood elevations outside the project site. Massachusetts DEP stormwater management standards shall apply to non-residential projects and residential projects over four lots	This standard is not applicable.
8. Runoff from any new impervious surface within the buffer zone shall be infiltrated on site to the maximum extent possible.	Runoff is being managed accordingly. For more details, refer to the Stormwater Report in Attachment C .

Proposed Work within the buffer zone includes up to approximately 54,075 SF, encompassing the entire Site. Buffer zone will be stabilized and seeded per Section 6.0 herein following Project activities.

4.3 20-Foot Undisturbed Buffer

While the Wetlands Protection Act recognizes the importance of the buffer zone, it does not include specific performance standards. Specific standards for the 20-Foot Undisturbed Buffer are not provided in the Bylaw either. Impacts to the 20-foot Undisturbed Buffer are permanent, including up to 25,140 SF of disturbance.

Due to the site constraints and special mitigation efforts involved in rerouting the streams, TRC is requesting that the BCC grant a variance to allow Work closer than 20-feet from the Inland Bank and Bordering Vegetated Wetlands.

4.4 Stormwater Management

Erosion and sediment controls are proposed to limit disturbance to wetland resource areas. During construction, overland flow from disturbed areas will be filtered by perimeter erosion and sediment controls, and any pumped water from upgradient areas will be discharged through filter bags. A Stormwater Report is attached to this NOI as **Attachment C**, detailing compliance with the Massachusetts Stormwater Management Standards.

5.0 CONSTRUCTION BEST MANAGEMENT PRACTICES

Work at the Project Site has been designed to minimize impacts to resource areas. These BMPs are described in the following sections.

5.1 Erosion and Sediment Controls

The Contractor will be required to install erosion and sediment controls throughout the Project Site, as specified on the Project Plans in **Attachment B**. These include, but are not limited to, the following:

- Prior to the start of Work, perimeter silt fencing or similar will be installed around the perimeter of the Work area per the Project Plans.
- All soil stockpiles will be surrounded by erosion control barriers and will be temporarily seeded and mulched if stockpiled for a period of more than seven days.
- Fueling and maintenance of equipment will occur outside the resource area and away from any drainage basins.

Locations of proposed erosion and sediment control features are displayed in the Project Plans in **Attachment B**. Erosion and sediment controls will be inspected regularly and after significant precipitation events.

Field changes based on site-specific conditions and availability may be required and will be reviewed by TRC and/or the BCC prior to implementation.

5.2 Site Access

Site access will be from Cambridge Street to the east of the Project Site. This access will be outfitted with a construction entrance of crushed stone to prevent sediment trackout into the street. Vehicles and machinery entering and exiting the Site will be inspected and washed to prevent the spread of invasive species.

6.0 PROJECT SITE STABILIZATION AND RESTORATION

Once construction associated with the Project is completed, the Site will be reestablished with an erosion control seed mix suitable for the Site. Erosion control barriers will remain in place until final stabilization is completed and will be removed only with the prior authorization of the BCC.

The Site will be monitored to avoid the establishment of invasive species, with targeted removal (mechanical and/or chemical) to be performed on an as-needed basis.

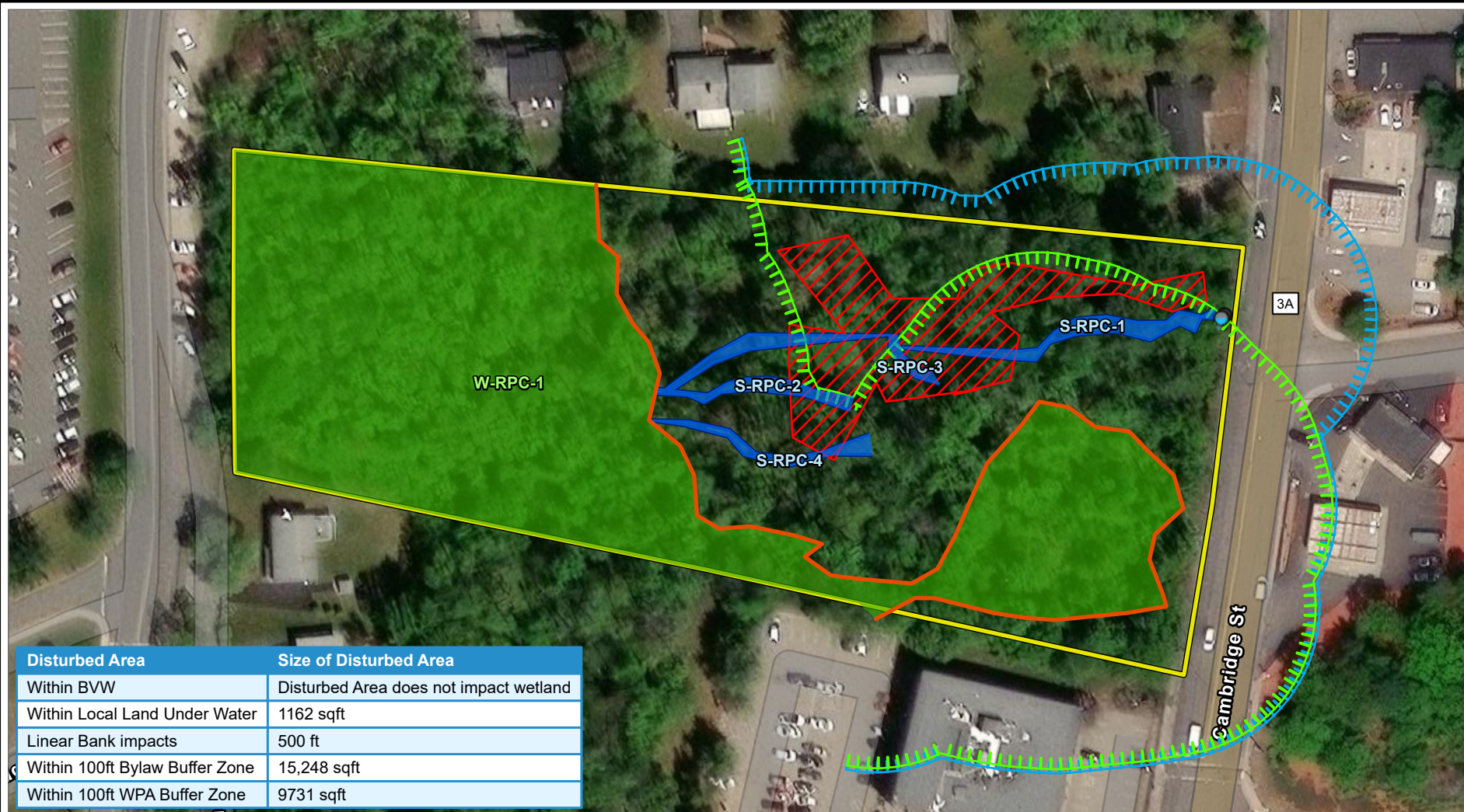
The newly established stream will be revegetated with an appropriate seed mix, such as that mix used for the site restoration in August 2025.

7.0 CONCLUSION

Foxboro Learning, LLC, proposes to develop the Site at "0" Cambridge Street in Burlington, MA, into an early childhood education center, including a building, play area, and other ancillary structures. While some impacts are permanent, stabilization serves as mitigation to future erosion and degradation of the streams and BVW. The streams will be rerouted and re-established and stabilized as a part of this Work.

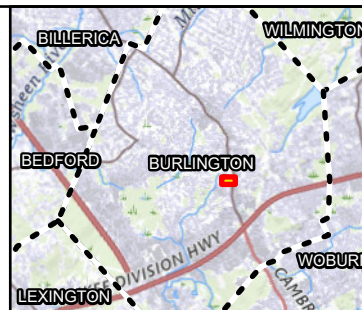
ATTACHMENT A – FIGURES

-- SAVED BY: JCORSO ON 5/21/2025, 17:14:31 PM: FILE PATH: T:\1-PROJECTS\FOXBORO LEARNING LLC\662119_CAMBRIDGEST\2-APR\662119_CAMBRIDGEST\APPX: LAYOUT NAME: FIG 2 DELINEATED RESOURCES (M

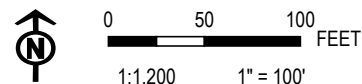



Disturbed Area	Size of Disturbed Area
Within BVW	Disturbed Area does not impact wetland
Within Local Land Under Water	1162 sqft
Linear Bank impacts	500 ft
Within 100ft Bylaw Buffer Zone	15,248 sqft
Within 100ft WPA Buffer Zone	9731 sqft

- | | | | |
|--|-------------------|--|------------------------|
| | PROJECT BOUNDARY | | WETLAND BOUNDARY |
| | CULVERT | | DELINEATED WETLAND |
| | STREAM BANK/EDGE | | AREA OF DISTURBANCE |
| | DELINEATED STREAM | | 100' WPA BUFFER ZONE |
| | | | 100' BYLAW BUFFER ZONE |



BASE MAP: USGS COLOR ORTHO IMAGERY
DATA SOURCES: TRC



PROJECT: FOXBORO LEARNING LLC	
CAMBRIDGE STREET BURLINGTON, MA	
TITLE: DELINEATED RESOURCES	
DRAWN BY: J. CORSO	PROJ. NO.: 662119.0000.0000
CHECKED BY: J. FREDENBURG	FIGURE 2
APPROVED BY: J. SHUSTER	
DATE: MAY 2025	
 <div>650 SUFFOLK STREET SUITE 200 LOWELL, MA 01854 PHONE: 978.970.5600</div>	
FILE: 662119_CAMBRIDGEST	

ATTACHMENT B – Project Plans

FOR

BORO LEARNING, LLC

PROPOSED

CHILD CARE FACILITY

PROPOSED

LOCATION OF SITE

CAMBRIDGE STREET, TOWN OF BURLINGTON

MIDDLESEX COUNTY, MASSACHUSETTS

PARCEL ID: 35-115-0

REFERENCES AND CONTACTS

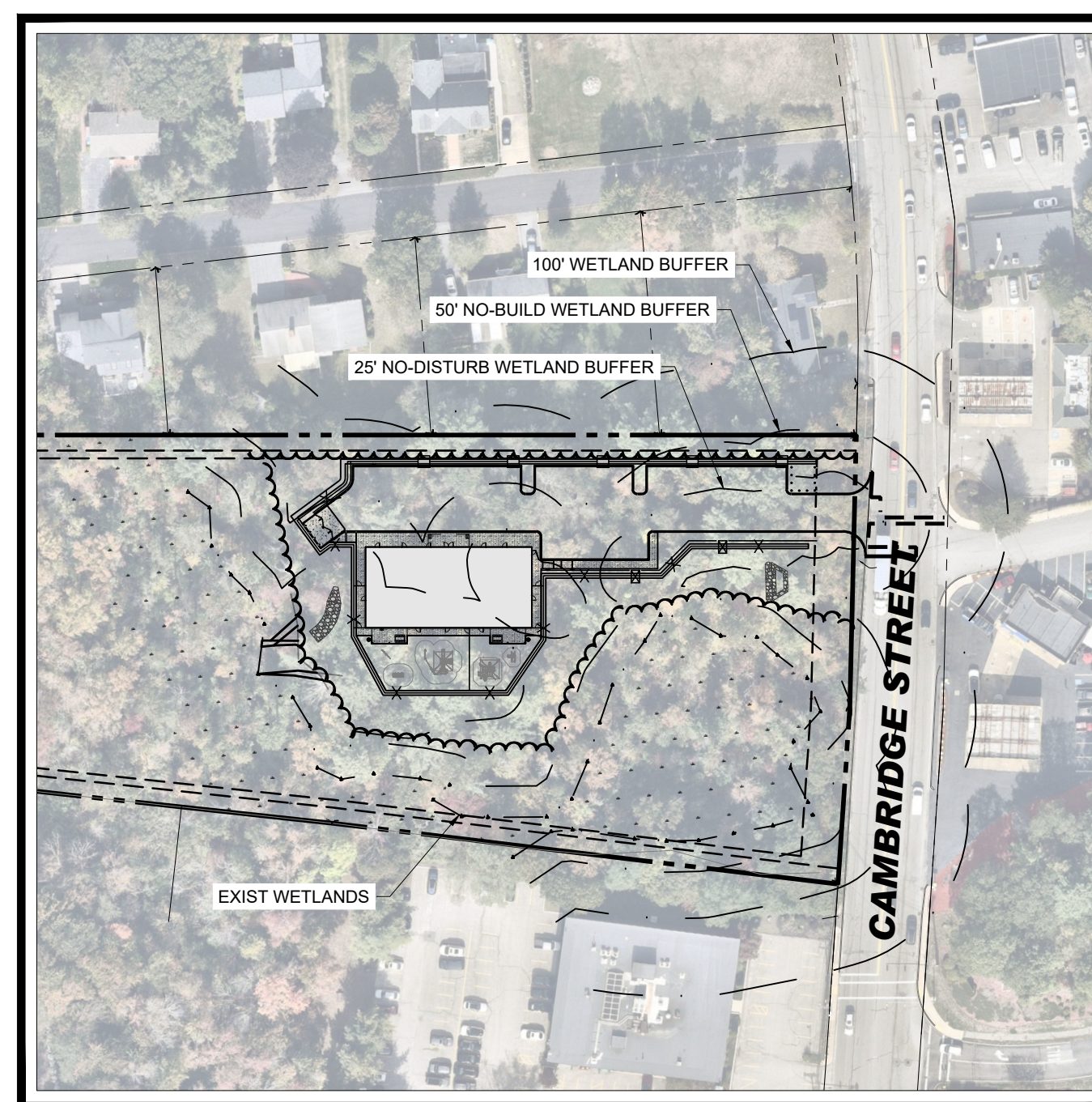
REFERENCES

- ◆ **BOUNDARY & TOPOGRAPHIC SURVEY:**
FELDMAN GEOSPATIAL
152 HAMPDEN STREET
BOSTON, MA 02119
DATED: 03/07/2025
SURVEY JOB #2500110
ELEVATIONS: NAVD 1988
- ◆ **GEOTECHNICAL INVESTIGATION REPORT:**
WHITESTONE ASSOCIATES, INC.
352 TURNPIKE ROAD, SUITE 105
SOUTH BURLINGTON, MA 01772
DATED: 04/01/2025
- ◆ **ARCHITECTURAL PLAN:**
JARMEL KIZEL ARCHITECTS AND ENGINEERS, INC.
42 CHICKER PARKWAY
LIVINGSTON, NH 07039
DATED: MM/DD/YY

THE ABOVE REFERENCED DOCUMENTS ARE INCORPORATED BY REFERENCE AS PART OF THESE PLANS. HOWEVER, BOHLER ENGINEERING DOES NOT CERTIFY THE ACCURACY OF THE WORK REFERENCED OR DERIVED FROM THESE DOCUMENTS, BY OTHERS.



TAX MAP
SCALE: 1" = 1,000'
SOURCE: USGS

**AERIAL MAP**

SCALE: 1" = 1,000'
SOURCE: NEARMAP AERIAL IMAGERY

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REVISIONS

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PROJECT No.:	MAA250027-00-
DRAWN BY:	JWT/S
CHECKED BY:	NPD/NE
DATE:	12/16/20
CAD I.D.:	P-CIVL-CNI

PROJECT:

**PROP.
SITE PLAN
DOCUMENTS**

_____ FOR

**FOXBORO
LEARNING,
LLC**

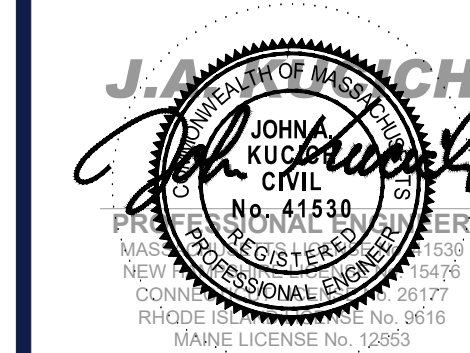
**PROPOSED
CHILD CARE CENTER**

**CAMBRIDGE STREET
TOWN OF BURLINGTON
MIDDLESEX COUNTY
MASSACHUSETTS
PARCEL ID: 35-115-0**

BOHLER //

**50 WASHINGTON ST., SUITE 2000
WESTBOROUGH, MA 01581**
Phone: (508) 480-9900

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SHEET TITLE:

COVER SHEET

SHEET NUMBER:

C-101

ORG. DATE - 12/16/2025

GENERAL NOTE

GENERAL NOTE:

IT IS THE RESPONSIBILITY OF THE CONTRACTOR TO REVIEW ALL OF THE DRAWINGS AND SPECIFICATIONS ASSOCIATED WITH THIS PROJECT WORK SCOPE PRIOR TO THE INITIATION OF CONSTRUCTION. SHOULD THE CONTRACTOR FIND A CONFLICT WITH THE DOCUMENTS RELATIVE TO THE SPECIFICATIONS OR APPLICABLE CODES, IT IS THE CONTRACTOR'S RESPONSIBILITY TO NOTIFY THE PROJECT ENGINEER OF RECORD IN WRITING PRIOR TO THE START OF CONSTRUCTION. FAILURE BY THE CONTRACTOR TO NOTIFY THE PROJECT ENGINEER SHALL CONSTITUTE ACCEPTANCE OF FULL RESPONSIBILITY BY THE CONTRACTOR. WORK OF THE CONTRACTOR SHALL BE AS DEFINED BY THE DRAWINGS AND SPECIFICATIONS. **THE CONTRACTOR SHALL COMPLY WITH ALL CITY, STATE AND FEDERAL REQUIREMENTS.**

PREPARED BY

The logo for BOHLER, featuring the word "BOHLER" in a bold, dark blue, sans-serif font, followed by two parallel orange diagonal lines.

CONTACT: NICK DEWHURST

(Rev. 10/2025)

- [illegible]

(Rev. 10/2025)

1. WHEN APPLICABLE, OWNER OPERATOR MUST FILE THE NOI FOR NPDES PERMITS AT APPROPRIATE AND/OR REQUIRED TIME FRAMES BASED UPON THE DESIRED STATE OF CONSTRUCTION, LAND DISTURBING ACTIVITIES AND/OR COMBINED SEWER OVERFLOW PREVENTION PLAN (CSP) OR STORMWATER POLLUTION PREVENTION PLAN (SWPPP). THE CONTRACTOR MUST STRICTLY ADHERE TO THE APPROVED SWPPP PLAN DURING CONSTRUCTION OPERATIONS (IF PROVIDED).
 2. THE CONTRACTOR SHALL MAINTAIN AND MONITOR ALL EXISTING AND PROPOSED EROSION CONTROL MEASURES OF JURISDICTIONAL STANDARD PSI AT 28 DAYS (OR 4,000 PSI) UNLESS OTHERWISE NOTED ON THE PLANS, DETAILS AND/OR GEOTECHNICAL REPORT. IF THERE ARE ANY DISCREPANCIES BETWEEN THE PLANS, DETAILS AND GEOTECHNICAL REPORT THEN THE GEOTECHNICAL REPORT MUST GOVERN.
 3. THE CONTRACTOR SHALL BE RESPONSIBLE FOR OBTAINING NECESSARY PERMITS UNDER SEPARATE APPLICATION UNLESS DONE SO AS PART OF JURISDICTIONAL PERMITTING PROCEDURES.
 4. THE CONTRACTOR MUST REPAIR OR REPLACE IN KIND, AT THE CONTRACTORS' SOLE COST AND EXPENSE, ALL DAMAGE TO EXISTING DRIVEWAYS, SIDEWALKS AND PAVEMENT DAMAGED BY CONSTRUCTION ACTIVITIES WHETHER SPECIFIED ON THIS PLAN OR NOT.
 5. WORK WITHIN THE RIGHT-OF-WAY MUST BE PERFORMED IN ACCORDANCE WITH ALL APPLICABLE REQUIREMENTS AND STANDARDS OF THE DEPARTMENT OF PUBLIC WORKS, ENGINEERING DEPARTMENT, HIGHWAY DESIGN DIVISION AND THE RIGHT-OF-WAY ACT.
 6. WHERE RETAINING WALLS ARE IDENTIFIED ON THE PLANS, TOP AND BOTTOM OF WALL WIDTHS DO NOT REPRESENT THE ACTUAL WIDTH OF THE PROPOSED WALL, RATHER THEY ARE AN ASSUMPTION BASED ON WALL LOCATION AND WALL TYPE. THE CONTRACTOR SHALL VERIFY THE EXACT WALL WIDTHS AND LOCATIONS ARE TO BE SET/DETERMINED BY THE CONTRACTOR OR WALL DESIGNER, AND MUST BE SET BASED UPON FINAL STRUCTURAL DESIGN SHOP DRAWINGS PREPARED BY THE APPROPRIATE PROFESSIONAL LICENSED IN THE STATE WHERE THE CONSTRUCTION OCCURS. THE CONTRACTOR MUST ENSURE THAT AN APPROPRIATELY SIGNED AND SEALED PROFESSIONAL ENGINEER REVIEW HEREON PRIOR TO CONSTRUCTION, REFER TO GRADING NOTES REGARDING RETAINING WALL DESIGN.
- CONTRACTOR IS CAUTIONED OF EXISTING UTILITY SERVICES TO REMAIN IN PROXIMITY TO PROPOSED CONSTRUCTION. CONTRACTOR SHALL PROVIDE ADEQUATE WARNING SIGNS, BARRICADES AND BOLLARDS WITH SIGNAGE AS NEEDED TO AVOID CONFLICTS WITH EXISTING UTILITY SERVICES TO REMAIN.

(Rev. 10/2025)

2. PROPOSED TOP OF CURB ELEVATIONS ARE GENERALLY SIX (6) INCHES ABOVE PAVEMENT GRADE UNLESS OTHERWISE NOTED.
3. CONTRACTOR MUST CONFIRM AND ENSURE THAT AS CONSTRUCTED IMPROVEMENTS CREATE THE FOLLOWING MINIMUM SLOPES (EXCEPT WHERE ADA REQUIREMENTS LIMIT THEM): 1.0% ON ALL CONCRETE SURFACES, 1.5% ON ASPHALT SURFACES, 1.5% IN LANDSCAPED AREAS AND 0.75% SLOPE AGAINST ALL ISLANDS, GUTTERS AND CURBSIDE DRAINAGE DEVICES.
4. WHERE RETAINING WALLS ARE IDENTIFIED IN THE PLANS, TOP AND BOTTOM OF WALL ELEVATIONS (GH & GL) REPRESENT THE PROPOSED FINISHED GRADE AT THE FACE OF THE TOP AND THE BOTTOM OF THE WALL AND DO NOT REPRESENT THE OVERALL HEIGHT OF THE WALL TO ITS EXTENTS (I.E. ANY CAP UNITS OR COURSES ABOVE THE FINISHED GRADE). THE TOTAL HEIGHT OF THE RETAINING WALL SHALL BE THE FINISHED GRADE IDENTIFIED HEREIN AND ARE TO BE SET/DETERMINED BY THE CONTRACTOR OR WALL DESIGNER, AND MUST BE SET BASED UPON FINAL STRUCTURAL DESIGN SHOP DRAWINGS PREPARED BY THE APPROPRIATE PROFESSIONAL LICENSED ENGINEER. THE STATE WHERE THE CONSTRUCTION OCCURS MAY REQUIRE CONTRACTOR ENSURE THAT THE AREA OF NO EXCAVATION OR PASSAGE OF THE RETAINING WALL, NO EXCAVATION MAY BE PERFORMED ON THE PASSIVE SIDE OF THE RETAINING WALL WITHOUT APPROPRIATELY AND SAFELY SUPPORTING THE WALL IN ACCORDANCE WITH THE STANDARD OF CARE AND ALL APPLICABLE RULES.
5. MSE OR GRAVITY BLOCK WALLS MUST BE CONSTRUCTED SUCH THAT UPON COMPLETION OF CONSTRUCTION THERE IS NO UNFINISHED SURFACE OR LIFTING RISGS VISIBLE (E.G. USE OF FINISHED TOP BLOCK OR CAPS).
6. STORMWATER RUNOFF WITHIN PROPERTY MUST BE COLLECTED ON-SITE WITH NO OVERLAND RUNOFF ONTO THE RIGHT-OF-WAY OR ADJACENT PROPERTIES TO THE MAXIMUM EXTENT POSSIBLE OR IN THE MANNER SHOWN ON THE CONSTRUCTION DRAWINGS. STORMWATER RUNOFF ONTO ADJACENT PROPERTIES MUST BE PREVENTED AS FAR AS POSSIBLE.
7. BEFORE COMMENCING GRADING WORK, CONTRACTOR MUST SUBMIT SAMPLES OF ALL NATIVE AND IMPORTED MATERIALS WITHIN THEIR INTENDED FOR STRUCTURAL USES TO THE GEOTECHNICAL ENGINEER OF RECORD. REFER TO GENERAL NOTES SHEET FOR ADDITIONAL ADA GUIDELINES AND REQUIREMENTS.
8. FOR ALL RAILING, THE RAILING HEIGHT SHALL BE GREATER IN HEIGHT (HEIGHT AS DEFINED IN INTERNATIONAL BUILDING CODE AND STATE BUILDING CODE):
 - A. THE OWNER OR THE OWNER'S CONTRACTOR IS TO PROVIDE A SITE-SPECIFIC RETAINING WALL DESIGN PREPARED BY THE OWNER'S GEOTECHNICAL ENGINEER TO BE USED FOR ALL RETAINING WALLS IN THE AREA WHERE THE CONSTRUCTION OCCURS. SOIL TYPES, WATER TABLE ELEVATION, EXISTING & PROPOSED SURROUNDING IMPROVEMENTS/CONDITIONS (INCLUDING BUT NOT LIMITED TO SLOPES, DRIVE ALLEYS, ROADS, FENCING, GUIDERAILS, UTILITIES, DRAINAGE FACILITIES, STRUCTURES, FOUNDATIONS, LIVE LOADS) AND ALL OTHER SITE-SPECIFIC INFORMATION MUST BE PROVIDED TO THE ENGINEER.
 - B. THE RETAINING WALL DESIGN SHALL BE CONSTRUCTED TO MEET THE DESIGN CRITERIA FOR THE STRUCTURAL INTEGRITY, CONSTRUCTABILITY AND/OR LONGEVITY MUST BE CONSIDERED AND INCORPORATED INTO THE RETAINING WALL DESIGN AS WELL AS THE GLOBAL STABILITY ANALYSIS.
9. BEFORE NEW OR EXISTING RETAINING WALL DESIGN MUST BE COMPLETED BY THE OWNER'S GEOTECHNICAL ENGINEER TO CERTIFY THE DESIGN MEETS INDUSTRY STANDARDS FOR FACTOR OF SAFETY. SOIL TYPES, WATER TABLE ELEVATION AND DESIGN PROPERTIES AS NOTED ABOVE MUST BE FIELD CONFIRMED AND APPROVED BY THE GEOTECHNICAL ENGINEER PRIOR TO WALL CONSTRUCTION.
10. CONTRACTOR MUST INSTALL CURB ALONG FACE OF BUILDING / WALL AS SHOWN TO PROVIDE CONSISTENT WALL ALONG LENGTH OF PROPOSED ACCESSIBLE RAMP AND RAMP LANDING TO MEET ADA/AB REQUIREMENTS.
11. CONTRACTOR MUST REVIEW RETAINING WALL LOCATIONS VERSUS APPLICABLE STATE AND LOCAL CODES AND PROVIDE FALL PROTECTION (E.G. FENCING OR RAILING) IN ACCORDANCE WITH SAID CODE.
12. CONTRACTOR MUST COORDINATE WITH OWNER/OPERATOR AT REVIEW EXISTING DEPRESSIONS WITHIN EXISTING PAVEMENT TO REMAIN AND MUST BE AVOIDED TO THE SCOPE OF WORK MUST PROVIDE POSITIVE DRAINAGE BY FIXING ANY EXISTING AREAS OF PONDING.

(Rev. 10/2025)

1. IN ADDITION TO THE ACCESSIBILITY DESIGN GUIDELINES ON SHEET C-102, THE CONTRACTOR MUST ALSO ENSURE THAT ALL ACCESSIBLE COMPONENTS AND ACCESSIBLE ROUTES ARE CONSTRUCTED IN STRICT ACCORDANCE WITH THE MASSACHUSETTS ARCHITECTURAL ACCESS BOARD (AAB) REGULATIONS 521 CMR. THE CONTRACTOR MUST IMMEDIATELY NOTIFY THE ENGINEER OF RECORD, IN WRITING, OF ANY DISCREPANCIES BETWEEN THE "AMERICANS WITH DISABILITIES ACT" (ADA) CODE, AAB, AND STATE BUILDING CODE AS IT RELATES TO ANY ACCESSIBLE IMPROVEMENTS BEING CONSTRUCTED PRIOR TO COMMENCING THE WORK.

(Rev. 10/2025)

- WHEN THESE PLANS INVOLVE MULTIPLE BUILDINGS, SOME OF WHICH MAY BE BUILT AT A LATER DATE, THE CONTRACTOR MUST EXTEND ALL UTILITY LOCATIONS TO THE LATEST DATE. THE CONTRACTOR MUST NOT EXTEND TO UTILITIES LOCATED AT AN EARLIER DATE AT LEAST FIVE (5) FEET. THE CONTRACTOR MUST PROVIDE THE PLANS FOR WHICH THE CONTRACTOR IS RESPONSIBLE. THE CONTRACTOR MUST CAP ENDS OF INSTALLED UTILITIES AS SHOWN ON THE PLANS. THE CONTRACTOR MUST PROVIDE A UTILITY LOCATIONS AND DEPTHS RECORD FOR EACH UTILITY. THE CONTRACTOR MUST PROVIDE A UTILITY LOCATIONS AND DEPTHS RECORD FOR EACH 10-FT STAKE, AND MUST NOTE THE LOCATION OF ALL UTILITY STUBS ON A CLEAN COPY OF THE PLAN. THIS RECORD DOCUMENT MUST BE PREPARED IN A NEAT AND WORKMAN-LIKE MANNER AND TURNED OVER TO THE PROJECT ENGINEER FOR REVIEW AND APPROVAL. THE CONTRACTOR MUST PROVIDE A UTILITY LOCATIONS AND DEPTHS RECORD AND SANITARY PIPE LENGTHS INDICATED ARE NOMINAL AND ARE MEASURED FROM CENTER OF STRUCTURE TO CENTER OF STRUCTURE UNLESS INDICATED ON THE PLANS OTHERWISE.
2. ALL SANITARY PIPES SHALL BE 12" (305MM) DIAMETER (AS PER THE PLANS) CONCRETE PIPE (CP) OR CLASS III WITH SILT/SILT-TIGHT JOINTS. WHEN HIGH-DENSITY POLYETHYLENE PIPE (HDPE) IS CALLED FOR ON THE PLANS, IT MUST CONFORM TO AASHTO M252 FOR PIPES 4" TO 10" AND TO AASHTO M254 FOR PIPES 12" TO 36" (305MM TO 914MM) DIAMETER. THE CONTRACTOR SHALL PROVIDE A UTILITY LOCATIONS AND DEPTHS RECORD FOR ROOF DRAIN CONNECTION MUST BE SDR 26 PVC OR SCHEDULE 40 UNLESS INDICATED OTHERWISE. HDPE PIPE JOINT GASKETS MUST BE PROVIDED AND CONFORM TO ASTM F447. DRAIN PIPE INSTALLED WITHIN THE ROOF SHALL BE FULLY INSULATED TO PREVENT CONDENSATION. GROUNDWATER CONDITIONS SHALL BE SAMPLED FOR POLYPROPYLENE PIPE (PP), OR APPROVED EQUIVALENT.

(Rev. 10/2025)

- THE CONTRACTOR'S PRICE FOR WATER AND SEWER SERVICE INSTALLATIONS MUST INCLUDE ALL FEES, COSTS, AND APPURTENANCES REQUIRED BY THE UTILITY PROVIDER (AND OTHER AGENCIES HAVING JURISDICTION OVER SUCH WORK). THE CONTRACTOR SHALL BE RESPONSIBLE FOR OBTAINING ALL NECESSARY FEES, TESTING, DISINFECTING, INSPECTIONS, ROAD OPENING & BACKFILL REQUIREMENTS, TRAFFIC CONTROL AND SURETY BONDS AS DEFINED BY THE PROVIDER (AND OTHER AGENCIES HAVING JURISDICTION OVER THE WORK).
2. ALL UTILITIES POLES, OVERHEAD WIRES AND ANY/all APPURTENANCES MUST BE COORDINATED BY THE CONTRACTOR WITH THE LOCAL UTILITY COMPANIES PRIOR TO THE ORDERING OF ANY MATERIALS. THIS MAY INCLUDE BUT IS NOT LIMITED TO THE REMOVAL, INSTALLATION, RELOCATION OR PROTECTION OF ANY BRACING, GUY WIRES, OVERHEAD WIRES, ETC. AS MAY BE REQUIRED TO ACCOMMODATE THE PROJECT.
3. SEWERS CONVEYING SANITARY FLOW, OR INDUSTRIAL FLOW MUST BE SEPARATED FROM WATER MAINS BY A DISTANCE OF AT LEAST TEN (10) FEET HORIZONTALLY. IF SUCH LATERAL SEPARATION IS NOT POSSIBLE, THE SEWER LINE SHALL BE INSTALLED IN AN EXCAVATION DEEP ENOUGH TO MAINTAIN A MINIMUM VERTICAL SEPARATION FROM THE BOTTOM OF THE WATER MAIN TO THE TOP OF THE SEWER LINE. WHERE APPROPRIATE SEPARATION FROM A WATER MAIN IS NOT POSSIBLE, THE SEWER MUST BE ENCASED IN CONCRETE, OR CONSTRUCTED OF DUCTILE IRON PIPE USING MECHANICAL OR SLIP-ON JOINTS FOR A DISTANCE OF AT LEAST TWO (2) FEET ON EACH END OF THE CROSSING. WHENEVER A SEWER LINE CROSSES UNDER A WATER MAIN LOCATED SO BOTH JUNCTIONS WILL BE AS FAR FROM THE WATER LINE AS POSSIBLE. WHERE A WATER MAIN CROSSES UNDER A SANITARY SEWER, ADEQUATE STRUCTURAL SUPPORT FOR THE SANITARY SEWER MUST BE PROVIDED. ALL CROSSINGS MUST BE IN ACCORDANCE WITH JURISDICTIONAL PERMITTING/UTILITY AUTHORITIES REGULATIONS.
4. WHEN THESE PLANS INVOLVE MULTIPLE BUILDINGS, SOME OF WHICH MAY BE BUILT AT A LATER DATE, THE CONTRACTOR MUST EXTEND ALL UTILITY SERVICES, INCLUDING BUT NOT LIMITED TO STORM, SANITARY, GAS, ELECTRIC, RENTED TELEPHONE, CABLE TV, ETC., THROUGH THE ENTIRE LOT. THE CONTRACTOR IS RESPONSIBLE. THE CONTRACTOR MUST CAP ENDS OF INSTALLED UTILITIES AS APPROPRIATE. MARK UTILITY ENDS WITH MAGNETIC TRACER TAPE. MARK TERMINUS LOCATIONS WITH A 2 IN X 4 FRAME SIGN. AND MUST NOTE THE LOCATION OF ALL UTILITY STUBS ON A CLEAN COPY OF THE PLAN. THIS INFORMATION MUST BE SUBMITTED TO THE CITY ENGINEER AND THE UTILITY PROVIDER. IT IS THE OWNER/DEVELOPER UPON COMPLETION OF THE WORK, ALL OF WHICH IS AT THE CONTRACTOR'S SOLE COST. UNLESS INDICATED OTHERWISE, ALL NEW UTILITIES/SERVICES, INCLUDING ELECTRIC, TELEPHONE, CABLE TV, ETC., MUST BE INSTALLED UNDERGROUND. ALL NEW UTILITY SERVICES MUST BE INSTALLED IN ACCORDANCE WITH THE CITY ENGINEER'S SPECIFICATIONS.
5. SANITARY PIPE MUST BE POLYVINYL CHLORIDE (PVC) SDR 35 EXCEPT WHERE CLEARLY INDICATED OTHERWISE. SANITARY LATERAL(S) MUST BE PVC SDR 26 UNLESS CLEARLY INDICATED OTHERWISE.
6. UNLESS CLEARLY INDICATED OTHERWISE, ALL SANITARY PIPING SHALL BE:
- A. FOR PIPES LESS THAN TWELVE (12) FEET DEEP: POLYVINYL CHLORIDE (PVC) SDR 35 PER ASTM D3034.
 - B. FOR PIPES GREATER THAN TWELVE (12) FEET DEEP: POLYVINYL CHLORIDE (PVC) SDR 26 PER ASTM D3034.
 - C. UNLESS LOCAL OR STATE BUILDING / PLUMBING CODE CLEARLY SPECIFIES DIFFERENTLY, SANITARY PIPING SHALL BE INSTALLED WITH A MINIMUM COVER OF 48 INCHES.
7. FOR ALL UTILITY PIPING (INCLUDING DRAIN) WITHIN TEN (10) FEET OF A BUILDING, PIPE MATERIAL MUST COMPLY WITH APPLICABLE LOCAL OR STATE BUILDING AND PLUMBING CODES. CONTRACTOR MUST REFER TO PLUMBING ENGINEERING PLANS AND VERIFY PIPE MATERIAL WITH LOCAL OFFICIAL PRIOR TO ORDERING OF MATERIALS.
8. CONTRACTOR MUST VERIFY THE CONNECTION OF EXTERIOR PIPING TO ANY FIXTURES (SUCH AS AN EXTERIOR GREASE INTERCEPTOR) OR OTHER DRAINAGE SYSTEMS WITH LOCAL OFFICIALS FOR COMPLIANCE WITH APPLICABLE LOCAL OR STATE BUILDING AND PLUMBING CODES PRIOR TO ORDERING OF MATERIALS.
9. WATER MAIN PIPING MUST BE INSTALLED IN ACCORDANCE WITH THE REQUIREMENTS AND SPECIFICATIONS OF THE LOCAL WATER COMPANY. IN THE ABSENCE OF SUCH REQUIREMENTS, WATER MAIN PIPING MUST BE INSTALLED IN ACCORDANCE WITH THE REQUIREMENTS AND SPECIFICATIONS OF THE UTILITY PROVIDER. ALL WATER MAIN PIPING SHALL COMPLY WITH THE APPLICABLE AWWA STANDARDS IN EFFECT AT THE TIME OF APPLICATION.
10. GAS METERS MUST BE PROTECTED AS REQUIRED BY THE JURISDICTIONAL GAS PROVIDER.
11. TRANSFORMERS AND SWITCH GEARS: GC MUST PROVIDE ALL APPURTENANCES IN ACCORDANCE WITH UTILITY PROVIDER'S SPECIFICATIONS. GC SHALL BE RESPONSIBLE FOR THE PROTECTION OF ALL EQUIPMENT, BUT NOT BE LIMITED TO, THE CONCRETE PAD, SECONDARY SPILL CONTAINMENT AREAS (IF REQUIRED), BOLLARD VEHICULAR PROTECTION, AND OTHER ITEMS AS MAY BE REQUIRED BY THE UTILITY PROVIDER FOR INSTALLATION OF THE TRANSFORMER AND SWITCH GEAR (IF REQUIRED). GC MUST COORDINATE ALL ASPECTS OF THE INSTALLATION WITH THE UTILITY PROVIDER PRIOR TO ORDERING OF MATERIALS AND NOTIFY DESIGN ENGINEER OF ANY CONFLICTS IN WRITING.

Dec 16, 2025
BOHLEREN[illegible]

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CAD I.D.: P-CIVL-CNDS

PROJECT:

FOR

**FOXBORO
LEARNING,
LLC**

**PROPOSED
CHILD CARE CENTER**

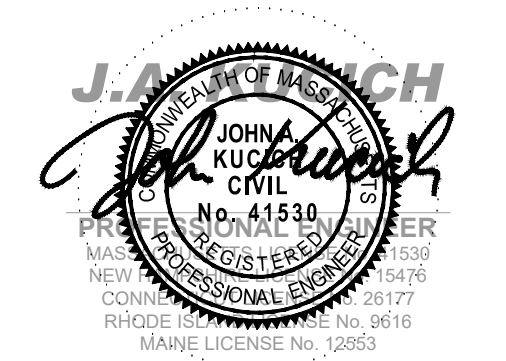
**CAMBRIDGE STREET
TOWN OF BURLINGTON
MIDDLESEX COUNTY
MASSACHUSETTS
PARCEL ID: 35-115-0**



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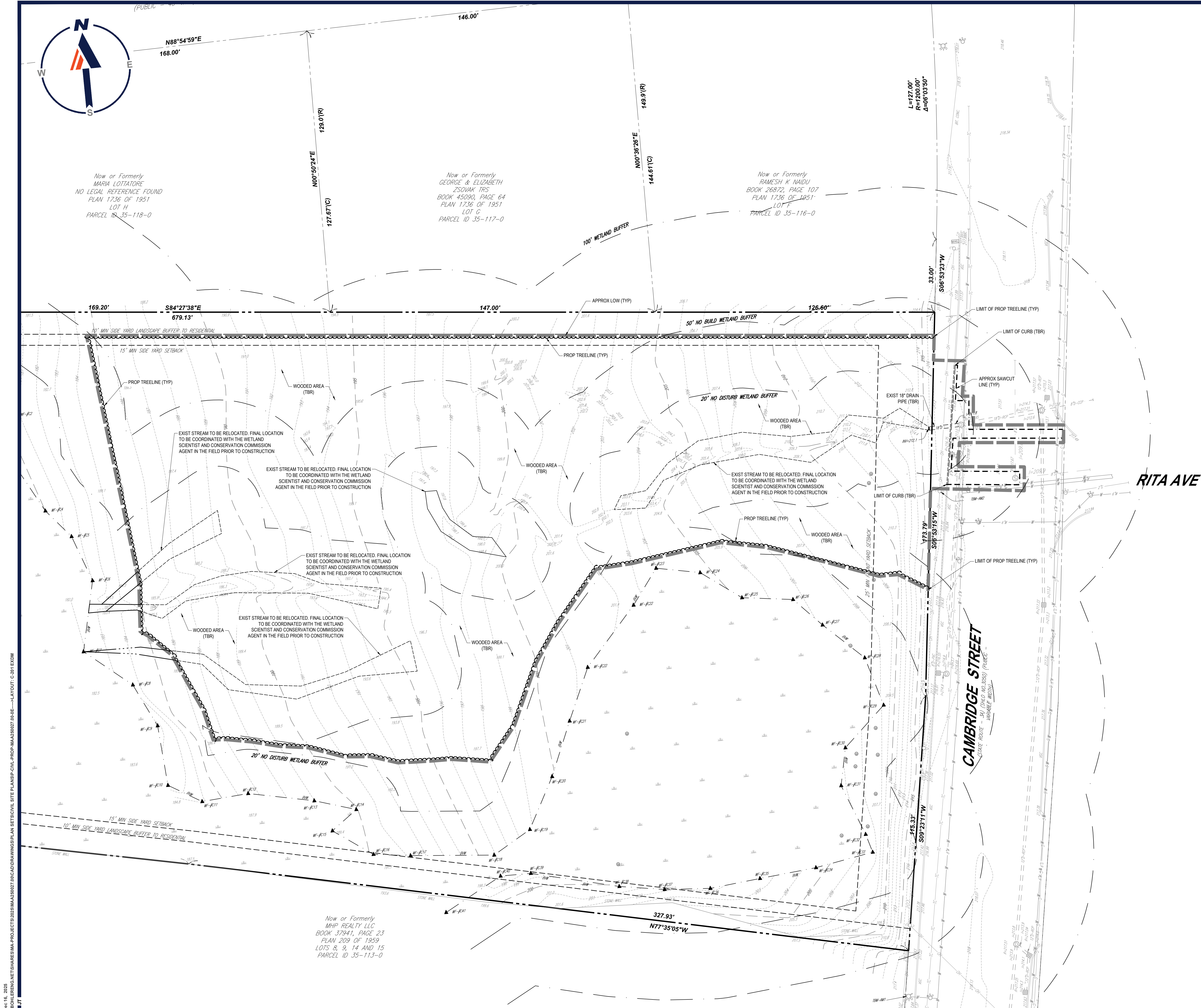
SHEET TITLE

SHEET NUMBER

C

C-103

ORG. DATE - 12/16/2025



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SITE CIVIL AND CONSULTING ENGINEERING

PROGRAM MANAGEMENT

LANDSCAPE ARCHITECTURE

SUSTAINABLE DESIGN

PERMITTING SERVICES

TRANSPORTATION SERVICES

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PROJECT:

PROP. SITE PLAN DOCUMENTS

FOR

FOXBORO LEARNING, LLC

PROPOSED CHILD CARE CENTER

CAMBRIDGE STREET
TOWN OF BURLINGTON
MIDDLESEX COUNTY
MASSACHUSETTS
PARCEL ID: 35-115-0

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J. A. BOHLER

JOHN A. BOHLER
No. 41530
PROFESSIONAL ENGINEER
MASSACHUSETTS
NEW PROFESSIONAL DESIGNER
CON. NO. 15476
RHODE ISLAND REG. NO. 0616
MAINE LICENSE NO. 12853

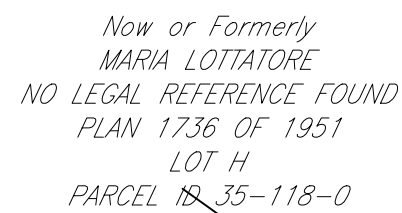
SHEET TITLE:

EXISTING CONDITIONS/DEMOLITION PLAN

SHEET NUMBER:

C-201

ORG. DATE - 12/16/2025



Now or Formerly
GEORGE & ELIZABETH
ZSOVAK TRS
BOOK 45090, PAGE 64
PLAN 1736 OF 1951
LOT G
PARCEL ID 35-117-0

Now or Formerly
RAMESH K NAIDU
BOOK 26872, PAGE 107
PLAN 1736 OF 1951
LOT F
PARCEL ID 35-116-0

Now or Formerly
MHP REALTY LLC
BOOK 37941, PAGE 23
PLAN 209 OF 1959
LOTS 8, 9, 14 AND 15
PARCEL ID 35-113-0

PARKING REQUIREMENTS				
ITEM	CODE	PERMITTED	EXISTING	PROPOSED
MIN STALL SIZE	ARTICLE VII 7.2.4	9' X 18'	N/A	9' X 18'
MIN ADA STALL SIZE	ARTICLE VII 7.2.4	13' X 18'	N/A	8' X 18'
MIN AISLE WIDTH	ARTICLE VII 7.2.4	24 FT	N/A	24 FT
MIN NUMBER OF SPACES	ARTICLE VII 7.2.5	33 SPACES	N/A	33 SPACES
MAX NUMBER OF SPACES	ARTICLE VII 7.2.5	33 SPACES	N/A	33 SPACES
MIN NUMBER OF ADA SPACES	521 CMR 23.2.1	2 SPACES	N/A	2 SPACES
CHILD CARE CENTER: REQUIRED= (3) SPACES PER 1,000 SQUARE FEET CALCULATION= (3) SPACES X 11,000 SF/1,000 = 33 SPACES				

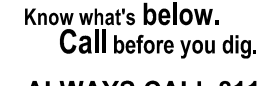
(1) WHERE GROUND SLOPE IS 10% OR LESS IN RO DISTRICTS, THE MINIMUM LOT AREA SHALL BE 20,000 SF. WHERE THE GROUND SLOPE IS MORE THAN 10%, THE MINIMUM LOT SIZE SHALL BE INCREASED BY 1,000 ADDITIONAL SQUARE FEET FOR EACH ADDITIONAL ONE PERCENTAGE OF SLOPE, TO A MAXIMUM OF 45,000 SF.

(1)1) THE CALCULATED GROUND SLOPE PURSUANT TO SECTION 5.12.5 (A) IS 8.5%

CAMBRIDGE STREET
(STATE ROUTE - 34) (SHILO NO. 3050) (PUBLIC)
(UNIMPAVED MATERIAL)

SCALE: 1" = 20'

REVISIONS

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PROJECT:

**PROP.
SITE PLAN
DOCUMENTS**

_____ FOR _____

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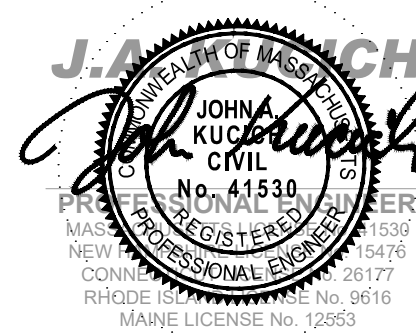
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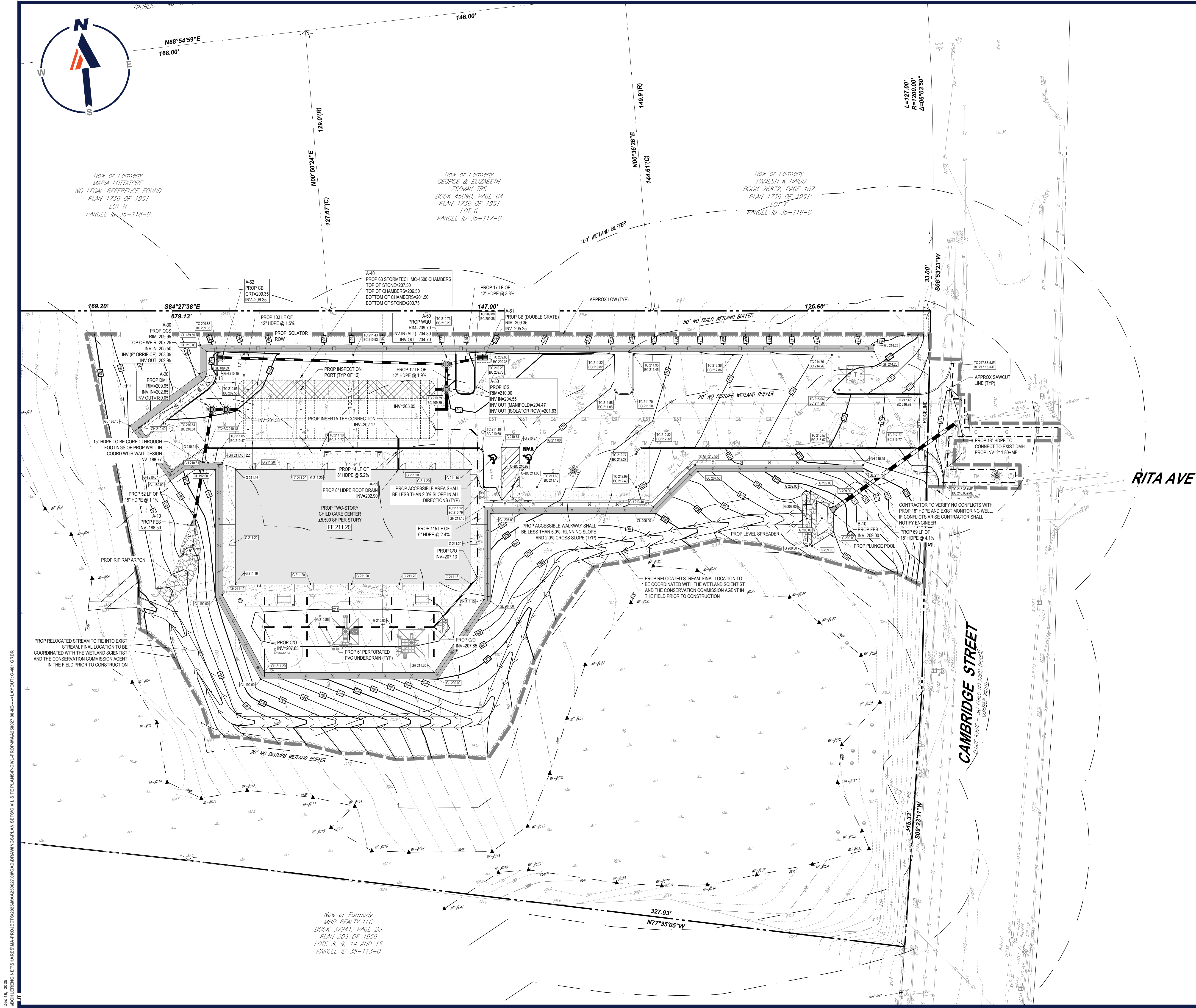
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SITE PLAN

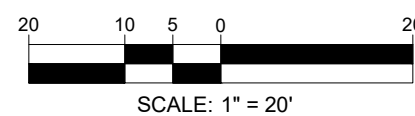
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C-301

ORG. DATE - 12/16/2025



THIS PLAN TO BE UTILIZED
FOR GRADING, DRAINAGE AND
UTILITIES PURPOSES ONLY



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PROGRAM MANAGEMENT
LANDSCAPE ARCHITECTURE
SUSTAINABLE DESIGN
PERMITTING SERVICES
TRANSPORTATION SERVICES

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DATE: 12/16/2025
CAD ID: P-CIVL-PROP

PROJECT:
**PROP.
SITE PLAN
DOCUMENTS**
FOR
**FOXBORO
LEARNING,
LLC**

PROPOSED
CHILD CARE CENTER

CAMBRIDGE STREET
TOWN OF BURLINGTON
MIDDLESEX COUNTY
MASSACHUSETTS
PARCEL ID: 35-115-0

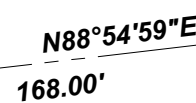
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WESTBOROUGH, MA 01581
Phone: (508) 480-9900
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J. A. BOHLER
JOHN A. BOHLER
No. 41530
REGISTERED PROFESSIONAL ENGINEER
MASSACHUSETTS
NEWTON, MA 02459
COUNCIL NO. 15476
RHODE ISLAND No. 0616
MAINE LICENSE No. 12853

SHEET TITLE:
**GRADING AND
DRAINAGE PLAN**

SHEET NUMBER:
C-401

ORG. DATE - 12/16/2025



Now or Formerly
MARIA LOTTATORE
NO LEGAL REFERENCE FOUND
PLAN 1736 OF 1951
LOT H
PARCEL ID 35-118-0

Now or Formerly
GEORGE & ELIZABETH
ZSOVAK TRS
BOOK 45090, PAGE 64
PLAN 1736 OF 1951
LOT G
PARCEL ID 35-117-0

Now or Formerly
RAMESH K NAIDU
BOOK 26872, PAGE 107
PLAN 1736 OF 1951
LOT F
PARCEL ID 35-116-0

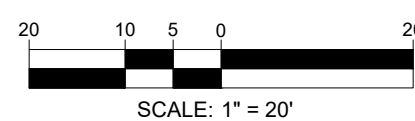
$L=127.00'$
 $R=1200.00'$
 $\Delta=06^{\circ}03'50''$

RITA AVE

CAMBRIDGE STREET
(STATE ROUTE - 3A) (SHILOH ROAD) (SOUTH)

ALL WET UTILITIES WITHIN 10-FT OF THE BUILDING, INCLUDING OIL/WATER SEPARATOR (IF APPLICABLE), ARE GOVERNED BY THE MASSACHUSETTS UNIFORM STATE PLUMBING CODE (248 CMR 10.00). SEE PLUMBING PLANS FOR PIPE MATERIAL WITHIN 10-FT OF THE BUILDING. WET UTILITIES ARE DRAWN TO THE BUILDING FACE FOR COORDINATION PURPOSES ONLY.

**THIS PLAN TO BE UTILIZED
FOR UTILITIES PURPOSES
ONLY**



REVISIONS

[illegible]

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**FOR CONCEPT
PURPOSES ONLY**

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UNLESS INDICATED OTHERWISE.

PROJECT No.:	MAA250027.00-0E
DRAWN BY:	JWT/SJR
CHECKED BY:	NPD/NEM
DATE:	12/16/2025
CAD I.D.:	P-CIVL-PROP

PROJECT:

**PROP.
SITE PLAN
DOCUMENTS**

**FOXBORO
LEARNING,
LLC**

**PROPOSED
CHILD CARE CENTER**

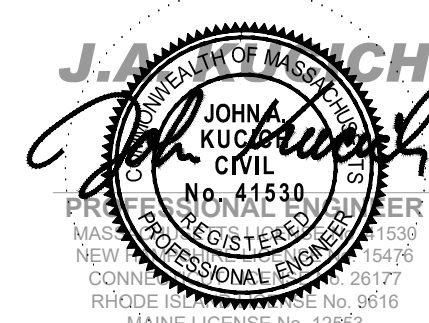
**CAMBRIDGE STREET
TOWN OF BURLINGTON
MIDDLESEX COUNTY
MASSACHUSETTS
PARCEL ID: 35-115-0**



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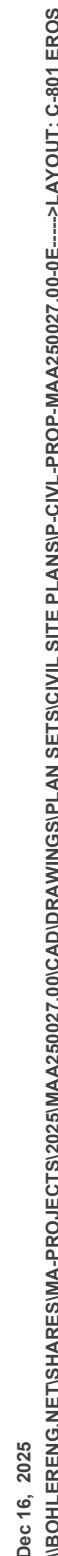
SHEET TITLE:

UTILITY PLAN

SHEET NUMBER:

C-501

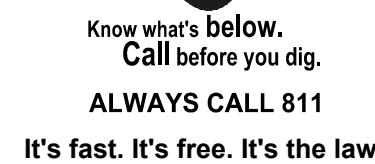
ORG. DATE - 12/16/2025



Now or Formerly
MHP REALTY LLC
BOOK 37941, PAGE 23
PLAN 209 OF 1959
LOTS 8, 9, 14 AND 15
PARCEL ID 35-113-0



CAMBRIDGE STREET
(STATE ROUTE - 3M) (SALO NO. 3050) (PENDING)

[illegible]

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UNLESS INDICATED OTHERWISE.

PROJECT No.: MAA250027.00-08
DRAWN BY: JWT/SJF
CHECKED BY: NPD/NEM
DATE: 12/16/2021
CAD I.D.: P-CIVL-PROF

PROJECT:

— FOR

**FOXBORO
LEARNING,
LLC**

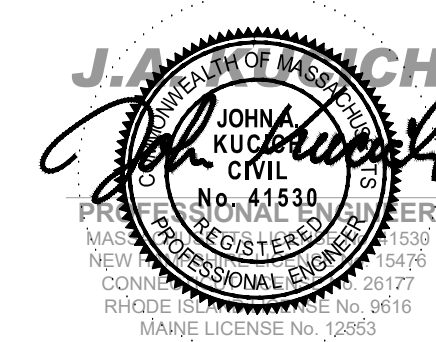
**PROPOSED
CHILD CARE CENTER**

**CAMBRIDGE STREET
TOWN OF BURLINGTON
MIDDLESEX COUNTY
MASSACHUSETTS
PARCEL ID: 35-115-0**

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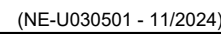
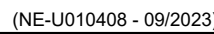
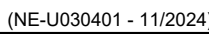
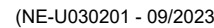
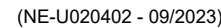
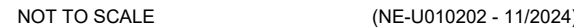


SHEET TITLE:

SHEET NUMBER:

C-801

ORG. DATE - 12/16/2025



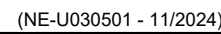
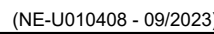
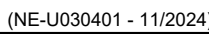
NOTES:

1. DO NOT COVER BELLS OR FLANGES WITH CONCRETE
2. WRAP ALL FITTINGS WITH VISQUEEN.
3. BACK ALL TEES ACCORDING TO SIZE OF BRANCH.
4. BACK ALL FUTURE LINE EXTENSIONS SHALL BE SUCH THAT LATER REMOVAL IS POSSIBLE.
5. ALL BENDS WHERE FITTINGS ARE USED, BOTH HORIZONTAL OR VERTICAL SHALL BE BACKED.
6. REACTION BACKING TABLE IS BASED ON 100 PSI AND SOIL BEARING PRESSURE OF 2,000 LBS/SF. ADDITIONAL BACKING MAY BE REQUIRED IN SOME AREAS AS DIRECTED BY ENGINEERS.
7. ALL CONCRETE SHALL BE 2500 PSI.
8. 16" AND LARGER REQUIRES SPECIFIC ANTI-THRUST DESIGN.

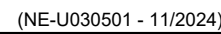
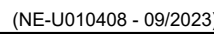
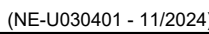


UPWARD THRUST BLOCKING															
(REQUIRED REINFORCING BARS & CUBIC YARDS OF P.C. CONCRETE)															
PIPE SIZE	CONC Y	90° BEND			45° BEND			22 1/2° BEND			11 1/4° BEND				
		REIN QTY	REIN SIZE		CONC Y	REIN QTY	REIN SIZE		CONC Y	REIN QTY	REIN SIZE		CONC Y	REIN QTY	REIN SIZE
6"	1.5	3	5		1.5	3	5	1.25	2	5		1.25	2	5	
8"	2	3	5		2	3	5	1.5	2	5		1.5	2	5	
10"	2.5	3	5		2.5	3	5	2	3	5		2	3	5	
12"	3	3	5		3	3	5	2.5	3	5		2.5	3	5	

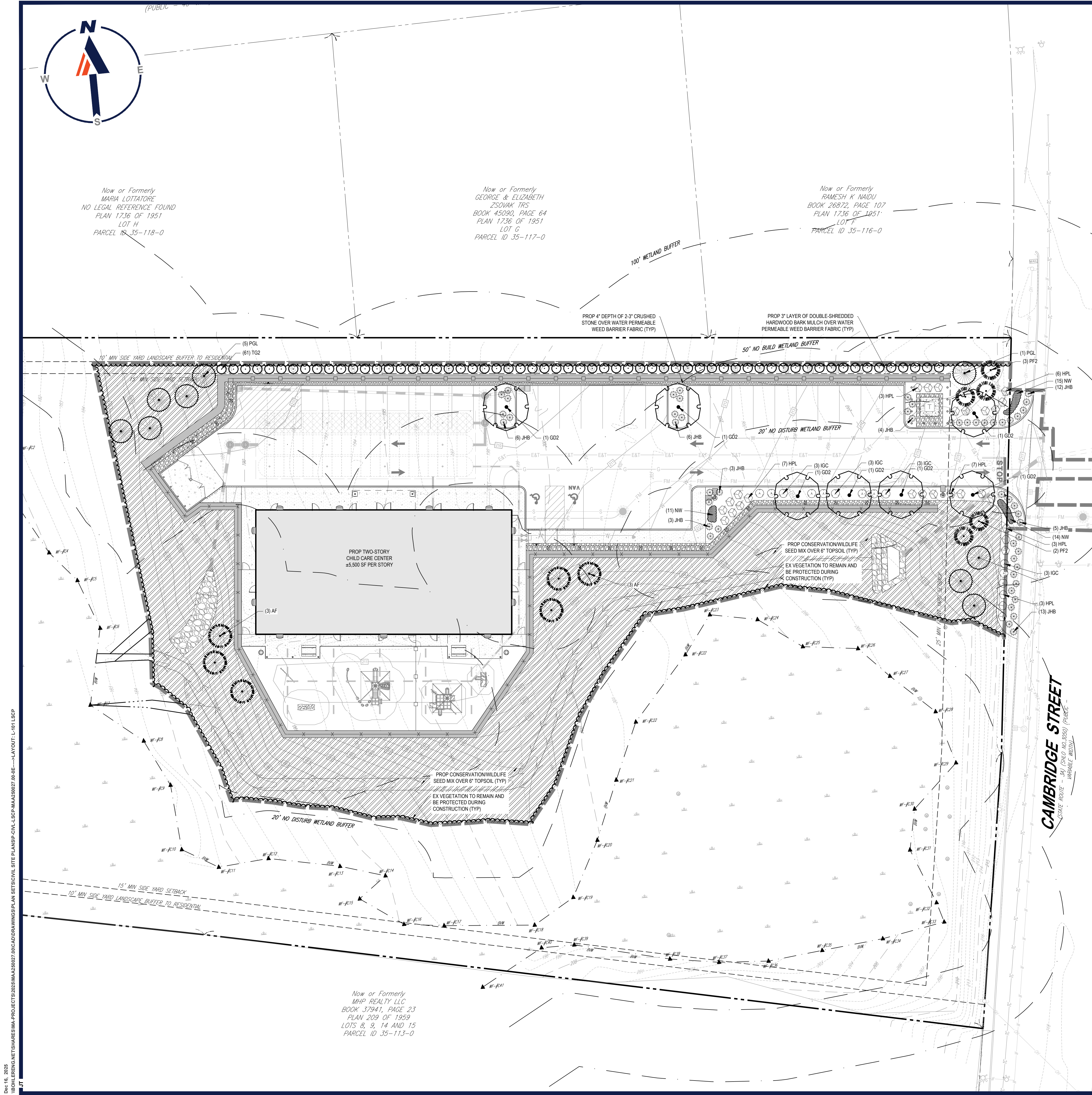
1. DO NOT COVER BELLS OR FLANGES WITH CONCRETE
2. WRAP ALL FITTINGS WITH VISQUEEN.
3. BACK ALL TEES ACCORDING TO SIZE OF BRANCH.
4. BACKING FUTURE LINE EXTENSIONS SHALL BE SUCH THAT LATER REMOVAL IS POSSIBLE.
5. ALL BENDS WHERE FITTINGS ARE USED, BOTH HORIZONTAL OR VERTICAL SHALL BE BACKED.
6. REACTION BACKING TABLE IS BASED ON 100 PSI AND SOIL BEARING PRESSURE OF 2,000 LBS/SF. ADDITIONAL BACKING MAY BE REQUIRED IN SOME AREAS AS DIRECTED BY ENGINEERS.
7. ALL CONCRETE SHALL BE 2500 PSI
8. 18" AND LARGER REQUIRES SPECIFIC ANTI-THRUST DESIGN.



REMOVAL IS POSSIBLE.
ALL BENDS WHERE FITTINGS ARE USED, BOTH HORIZONTAL OR VERTICAL
SHALL BE BACKED.
REACTION BACKING TABLE IS BASED ON 100 PSI AND SOIL BEARING
PRESSURE OF 2,000 LBS/SF ADDITIONAL BACKING MAY BE REQUIRED IN
SOME AREAS AS DIRECTED BY ENGINEERS.



REMOVAL IS POSSIBLE.
ALL BENDS WHERE FITTINGS ARE USED, BOTH HORIZONTAL OR VERTICAL
SHALL BE BACKED.
REACTION BACKING TABLE IS BASED ON 100 PSI AND SOIL BEARING
PRESSURE OF 2,000 LBS/SF ADDITIONAL BACKING MAY BE REQUIRED IN
SOME AREAS AS DIRECTED BY ENGINEERS.



TOWN OF BURLINGTON, MASSACHUSETTS LANDSCAPE REQUIREMENTS				
SECTION	REQUIREMENT	PROPOSED	CALCULATION	
ARTICLE VII - GENERAL REGULATIONS	§7.4.0 GENERAL LANDSCAPING REQUIREMENTS §7.4.4 LANDSCAPING REQUIREMENTS 1. SCREENING. IN ACCORDANCE WITH AN APPROVED SITE PLAN, SCREENING SHALL BE PROVIDED, ERECTED AND MAINTAINED TO SHIELD RO AND RG DISTRICTS AND MUNICIPAL PROPERTIES FROM ADJOINING BUSINESS. SCREENING SHALL BE ERECTED OR PLANTED BEFORE THE PREMISES ARE FIRST OCCUPIED. ALTERNATIVELY, THE PLANNING BOARD MAY ACCEPT A FINANCIAL GUARANTEE IN THE AMOUNT OF THE COST OF INSTALLING THE SCREENING AND A WRITTEN AGREEMENT TO COMPLETE THE SCREENING WITHIN A SPECIFIED TIME AND PERMIT OCCUPANCY BEFORE THE SCREENING IS INSTALLED. 3. RESIDENTIAL BUFFERS. PROPERTY LINE(S) WHICH ALSO BOUND RESIDENTIAL DISTRICTS SHALL BE SCREENED FROM NONRESIDENTIAL USES BY MEANS OF PLANTINGS OR MAINTENANCE OF TREES OF A SPECIES COMMON TO THE AREA AND APPROPRIATE FOR SCREENING, SPACED TO MINIMIZE VISUAL INTRUSION, AND PROVIDING AN OPAQUE YEAR-ROUND VISUAL BUFFER BETWEEN USES. 4. ACCESSORY RECEPTACLES. DUMPSTERS AND SIMILAR ACCESSORY RECEPTACLES OVER ONE CUBIC YARD CAPACITY SHALL BE ENCLOSED AND SCREENED FROM ALL ADJACENT PREMISES AND STREETS FROM WHICH SUCH FEATURES WOULD OTHERWISE BE VISIBLE IN ACCORDANCE WITH THIS SECTION. 5. UTILITIES. ANY LOADING AREA OR HVAC EQUIPMENT OR OTHER ELECTRICAL EQUIPMENT PLACED ON THE GROUND LEVEL SHALL BE SCREENED FROM ALL ADJACENT PREMISES AND STREETS FROM WHICH IT WOULD OTHERWISE BE VISIBLE IN ACCORDANCE WITH THIS SECTION. 6. PLANT MATERIALS. PLANTED AREAS SHALL CONTAIN AN APPROPRIATE MIX OF NATIVE PLANT SPECIES AS IDENTIFIED BY THE NATIVE PLANT TRUST THAT ARE APPROPRIATE TO THE PROPOSED USE, SITE LAYOUT, SOILS, AND OTHER ENVIRONMENTAL CONDITIONS. VEGETATION IS PREFERABLE TO MULCH WHERE PRACTICAL. 7. EXISTING TREES. EXISTING TREES WITH A DIAMETER AT BREAST HEIGHT (DBH) OF TWELVE INCHES (12") OR MORE SHALL NOT BE REMOVED EXCEPT BY PRIOR APPROVAL OF THE PLANNING BOARD, AND IF REMOVED, SHALL BE REPLACED WITH A MINIMUM 3" CALIPER TREE, UNLESS WAIVED BY THE PLANNING BOARD.	N/A - SITE IS LOCATED WITHIN RO DISTRICT. HOWEVER, SCREENING IS PROVIDED		
		PROVIDED		
		PROVIDED		
		PROVIDED		
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		PROVIDED		
	§7.4.7 PARKING LOT LANDSCAPING 2. PERIMETER SCREENING. ALL SURFACE PARKING LOTS WITH FRONTAGE ON ANY PORTION OF A STREET RIGHT-OF-WAY SHALL BE SCREENED WITH THE FOLLOWING: A) A MINIMUM 10-FOOT WIDE, LANDSCAPED AREA WITH A CONTINUOUS ROW OF SHRUBS, GRASSES, AND/OR SHADE TREES MUST BE PROVIDED BETWEEN THE STREET AND PARKING LOT. B) SHRUBS AND GRASSES MUST BE A MINIMUM OF 18 INCHES IN HEIGHT WHEN PLANTED AND MUST REACH A MINIMUM SIZE OF 36 INCHES IN HEIGHT WITHIN 3 YEARS OF PLANTING. C) AN INTERIOR ISLAND ABUTTING A SINGLE ROW OF PARKING SPACES SHALL BE A MINIMUM OF 8.5 FEET IN WIDTH AND 150 SQUARE FEET IN AREA. 3. INTERIOR ISLANDS. A) A LANDSCAPED INTERIOR ISLAND SHALL BE PROVIDED FOR EVERY 10 PARKING SPACES. INTERIOR ISLANDS SHALL BE DISTRIBUTED EVENLY THROUGHOUT THE PARKING AREA. INTERIOR ISLANDS MAY BE CONSOLIDATED, OR INTERVALS MAY BE EXPANDED IN ORDER TO PRESERVE EXISTING TREES. C) AN INTERIOR ISLAND ABUTTING A SINGLE ROW OF PARKING SPACES SHALL BE A MINIMUM OF 8.5 FEET IN WIDTH AND 150 SQUARE FEET IN AREA. 5. TREE COVERAGE. B) IN NO CASE CAN THERE BE LESS THAN ONE SHADE TREE FOR EVERY 2,000 SQUARE FEET OF PARKING AREA INCLUDING DRIVING AISLES.	EXISTING TREES TO REMAIN WHERE FEASIBLE	BUFFER PROVIDED	
		PROVIDED WHERE SIGHT LINES ARE NOT AFFECTED		
		PROVIDED		
		PROVIDED		
		PROVIDED		
		REQUIRED: 7 TREES		13,953 SF / 2,000 = 6.98 TREES

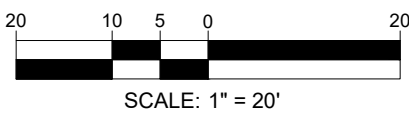
PLANT SCHEDULE					
CODE	QTY	BOTANICAL NAME	COMMON NAME	SIZE	CONTAINER
CANOPY TREES					
GD2	7	GLEDTISIA TRIACANTHOS INERMIS 'DRAVES'	STREET KEEPER® HONEY LOCUST	2.5"-3" CAL.	B&B
EVERGREEN TREES					
AF	6	ABIES FRASERI	FRASER FIR	6'-8"	B&B
PF2	5	PINUS FLEXILIS 'VANDERWOLF'S PYRAMID'	VANDERWOLF'S PYRAMID LIMBER PINE	5'-6" HT.	B&B
PGL	9	PICEA GLAUCOA	WHITE SPRUCE	6'-8"	B&B
TG2	61	THUJA STANDISHII X PLICATA 'GREEN GIANT'	GREEN GIANT ARBORVITAE	6'-8"	B&B
SHRUBS					
HPL	29	HYDRANGEA PANICULATA 'SMHPLQF'	LITTLE QUICK FIRE PANICLE HYDRANGEA	24-30"	CONTAINER
IGC	12	ILEX GLABRA 'COMPACTA'	COMPACT INKBERRY	24-30"	CONTAINER
GROUND COVER					
JHB	52	JUNIPERUS HORIZONTALIS 'BAR HARBOR'	BAR HARBOR CREEPING JUNIPER	15-18" SPRD	CONTAINER
PERENNIALS					
NW	40	NEPETA X 'WALKER'S LOW'	WALKER'S LOW CATMINT	1 GAL.	CONTAINER

SEED MIX KEY	
HATCH	DESCRIPTION
	CONSERVATION / WILDLIFE SEED MIX

GENERAL LANDSCAPE NOTES:

- THESE LANDSCAPE PLANS ARE TO BE READ IN CONJUNCTION WITH THE LANDSCAPE SPECIFICATIONS, AND ASSOCIATED DETAILS FOUND ON THE LANDSCAPE DETAILS SHEET. THE GENERAL NOTES, FOUND ON THE NOTES PAGE OF THIS PLAN SET, ARE CONSIDERED PART OF LANDSCAPE PLANS. THE CONTRACTOR MUST REFER TO, AND FULLY COMPLY WITH, ALL NOTES, SPECIFICATIONS AND DETAILS DESCRIBED HEREIN, ON THE LANDSCAPE PLANS AND IN THE LANDSCAPE DETAILS SHEET.
- ALL DISTURBED UNPAVED AREAS, EXCLUDING PLANTING BEDS, ARE TO BE INSTALLED AS LAWN IN ACCORDANCE WITH "MATERIALS" SECTION OF THE LANDSCAPE SPECIFICATIONS, UNLESS OTHERWISE SPECIFICALLY STATED ON THIS PLAN.
- SHRUBS PLANTED ALONG HEAD-IN PARKING STALLS SHALL BE INSTALLED TO ALLOW A CLEARANCE OF TWO FEET FROM FACE OF CURB TO ALLOW FOR BUMPER OVERHANG.
- PLANT MATERIAL SUBSTITUTIONS MUST BE FORMALLY SUBMITTED TO BOHLER AND THE MUNICIPALITY'S ENGINEERING AND LANDSCAPE CONSULTANTS FOR REVIEW AND APPROVAL PRIOR TO INSTALLATION.
- WITHOUT EXCEPTION, WEED BARRIER FABRIC SHALL NOT BE INSTALLED WITHIN ANY STORMWATER FACILITY, INCLUDING RAINGARDENS, INFILTRATION TRENCHES, VEGETATIVE SWALES AND STORMWATER BASINS.
- IF IRRIGATION IS REQUIRED BY THE OWNER OR APPROVING MUNICIPALITY, THE CONTRACTOR SHALL PROVIDE AN IRRIGATION SYSTEM MEETING THE SPECIFICATIONS OF THE CHOSEN PRODUCT'S MANUFACTURER. THE IRRIGATION DESIGN SHALL ACCOMMODATE LAWN AND BED AREAS EACH UNDER SEPARATE ZONES TO MAXIMIZE WATER EFFICIENCY. THE CONTRACTOR SHALL BE RESPONSIBLE FOR SECURING ANY PERMITS REQUIRED FOR THE INSTALLATION OF AN IRRIGATION SYSTEM.
- CONTRACTOR SHALL BE RESPONSIBLE FOR THE MAINTENANCE OF ALL LANDSCAPING UNTIL PROJECT MAINTENANCE IS TURNED OVER TO THE PROPERTY OWNER OR OTHER RESPONSIBLE PARTY. SUCH RESPONSIBILITIES INCLUDE, BUT ARE NOT LIMITED TO, THE CARE, WATERING, AND MAINTENANCE OF ALL PLANT MATERIAL; LAWN MOWING; AND SEASONAL MAINTENANCE.

THIS PLAN TO BE UTILIZED
FOR LANDSCAPE
PURPOSES ONLY



BOHLER
SITE CIVIL AND CONSULTING ENGINEERING
PROGRAM MANAGEMENT
LANDSCAPE ARCHITECTURE
SUSTAINABLE DESIGN
PERMITTING SERVICES
TRANSPORTATION SERVICES

REVISIONS			
REV	DATE	COMMENT	DRAWN BY / CHECKED BY

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PROJECT No.: MAA250027.00-0E
DRAWN BY: JJW/SJR
CHECKED BY: NPD/DEM
DATE: 12/16/2025
CAD ID: P-CIVL-LSCP

PROJECT:
**PROP.
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DOCUMENTS**
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LEARNING,
LLC**
PROPOSED
CHILD CARE CENTER
CAMBRIDGE STREET
TOWN OF BURLINGTON
MIDDLESEX COUNTY
MASSACHUSETTS
PARCEL ID: 35-115-0

BOHLER
50 WASHINGTON ST., SUITE 2000
WESTBOROUGH, MA 01581
Phone: (508) 480-9900
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SHEET TITLE:
**LANDSCAPE
PLAN**
SHEET NUMBER:
L-101
ORG. DATE - 12/16/2025

SCOPE OF WORK:

THE LANDSCAPE CONTRACTOR SHALL BE REQUIRED TO PERFORM ALL CLEARING, FINISHED GRADING, SOIL PREPARATION, FERTILIZER SEEDING, SOILS, PLANTING AND MULCHING INCLUDING ALL LABOR, MATERIALS, TOOLS AND EQUIPMENT NECESSARY FOR THE COMPLETION OF THIS PROJECT, UNLESS OTHERWISE CONTRACTED BY THE GENERAL CONTRACTOR.

2. MATERIALS

- GENERAL - ALL HARDSCAPE MATERIALS SHALL MEET OR EXCEED SPECIFICATIONS AS OUTLINED IN THE STATE DEPARTMENT OF TRANSPORTATION'S SPECIFICATIONS.
- TOPSOIL - NATURAL, FRIABLE, LOAMY SILT SOIL HAVING AN ORGANIC CONTENT NOT LESS THAN 5%, A PH RANGE BETWEEN 4.5-7.0 IT SHALL BE FREE OF DEBRIS, ROCKS LARGER THAN ONE INCH (1"), WOOD, ROOTS, VEGETABLE MATTER AND CLAY CLUMPS.
- LAWN -
 - ALL DISTURBED AREAS ARE TO BE TREATED WITH A MINIMUM 6" THICK LAYER OF TOPSOIL, OR AS DIRECTED BY THE LOCAL ORDINANCE, WITH SEEDS OR SODDED IN ACCORDANCE WITH THE PERMANENT STABILIZATION METHODS INDICATED ON THE LANDSCAPE PLAN.
 - LAWN SEED MIXTURE SHALL BE FRESH, CLEAN NEW CROP SEED.
 - SOD SHALL BE STRONGLY ROOTED, WEED AND DISEASE-FREE PLANTS WITH A UNIFORM THICKNESS, SOD INSTALLED ON TOP OF GREATLY EXPOSED ROOTS OR OTHER OBSTRUCTIONS.
 - MULCH - ALL PLANTING BEDS SHALL BE MULCHED WITH A 3" THICK LAYER OF DOUBLE SHREDDED HARDWOOD BARK MULCH, UNLESS OTHERWISE STATED ON THE LANDSCAPE PLAN AND/OR LANDSCAPE PLAN NOTES / DETAILS.
- FERTILIZER -
 - FERTILIZER SHALL BE DELIVERED TO THE SITE MIXED AS SPECIFIED IN THE ORIGINAL UNOPENED STANDARD BAGS SHOWING WEIGHT, ANALYSIS AND NAME OF MANUFACTURER. FERTILIZER SHALL BE STORED IN A WEATHERPROOF PLACE SO THAT IT CAN BE KEPT DRY PRIOR TO USE.
 - FOR THE PURPOSES OF THIS CONTRACT, FERTILIZER SHALL BE 10% NITROGEN, 6% PHOSPHORUS AND 4% POTASSIUM BY WEIGHT. A FERTILIZER SHOULD NOT BE SELECTED WITHOUT A SOIL TEST PERFORMED BY A CERTIFIED SOIL LABORATORY. CONTRACTOR TO ADHERE TO STATE REGULATIONS REGARDING APPLICATION OF FERTILIZERS.
- PLANT MATERIAL -
 - ALL PLANTS SHALL IN ALL CASES CONFORM TO THE REQUIREMENTS OF THE "AMERICAN STANDARD FOR NURSERY STOCK" (ANSI Z60.1), LATEST EDITION, AS PUBLISHED BY THE AMERICAN NURSERY & LANDSCAPE ASSOCIATION.
 - EXCEPT WHERE THE ABOVE REQUIREMENTS ARE SPECIFICALLY NOTED, ALL PLANTS SHALL BE OF THE BEST QUALITY IN ALL CASES. BOTANICAL NAMES SHALL TAKE PRECEDENCE OVER COMMON NAMES FOR ANY AND ALL PLANT MATERIAL.
 - ALL PLANTS SHALL BE LEGIBLY TAGGED WITH THE PROPER NAME AND SIZE. TAGS ARE TO REMAIN ON AT LEAST ONE PLANT OF EACH SPECIES FOR VERIFICATION PURPOSES DURING THE FINAL INSPECTION.
 - TREES WITH ABRASION OF THE BARK, SUN SCALDS, DISFIGURATION OR FRESH CUTS OF LIMBS OVER 1/2", WHICH HAVE NOT BEEN COMPLETELY CALLOUSED, SHALL BE REJECTED. PLANTS SHALL NOT BE BOUND WITH WIRE OR ROPE AT ANY POINTS.
 - ALL PLANTS SHALL BE TYPICAL OF THEIR SPECIES OR VARIETY AND SHALL HAVE A NORMAL HABIT OF GROWTH: WELL DEVELOPED BRANCHES, DENSELY FOLIATED, VIGOROUS ROOT SYSTEMS AND BE FREE OF DISEASE, INSECTS, PESTS, OR LARVAE.
 - CALIPER MEASUREMENTS OF NURSERY GROWN TREES SHALL BE TAKEN AT A POINT ON THE TRUNK SIX INCHES (6") ABOVE THE NATURAL GRADE FOR TREES UP TO AND INCLUDING A FOUR INCH (4") CALIPER SIZE. IF THE CALIPER AT SIX INCHES (6") ABOVE THE GROUND EXCEEDS FOUR INCHES (4") IN CALIPER, THE CALIPER SHOULD BE MEASURED AT 12" TO 12" ABOVE THE GROUND.
 - SHRUBS SHALL BE MEASURED TO THE AVERAGE HEIGHT OR SPREAD OF THE SHRUB, AND NOT TO THE LONGEST BRANCH.
 - ROOTS AND SHRUBS SHALL BE HANDLED WITH CARE BY THE ROOT BALL.

3. GENERAL WORK PROCEDURES

- CONTRACTOR TO UTILIZE WORKMANLIKE INDUSTRY STANDARDS IN PERFORMING ALL LANDSCAPE CONSTRUCTION. THE SITE IS TO BE LEFT IN A CLEAN STATE AT THE END OF EACH WORKDAY. ALL DEBRIS, MATERIALS AND TOOLS SHALL BE REMOVED FROM THE SITE.
- WASTE MATERIALS AND DEBRIS SHALL BE COMPLETELY DISPOSED OF AT THE CONTRACTOR'S EXPENSE. DEBRIS SHALL NOT BE BURIED, INCLUDING ORGANIC MATERIALS, BUT SHALL BE REMOVED COMPLETELY FROM THE SITE.

4. SITE PREPARATION

- BEFORE AND DURING PRELIMINARY GRADING AND FINISHED GRADING, ALL WEEDS AND GRASSES SHALL BE DUG OUT BY THE ROOTS AND DISPOSED OF IN ACCORDANCE WITH GENERAL WORK PROCEDURES OUTLINED HEREIN.
- ALL EXISTING TREES TO REMAIN SHALL BE PRUNED TO REMOVE ANY DAMAGED BRANCHES. THE ENTIRE LIMB OF ANY DAMAGED BRANCH SHALL BE REMOVED. ALL EXISTING TREES TO BE CONSTRUCTED SHALL BE CUTS ARE SMOOTH AND STRAIGHT. ANY EXPOSED ROOTS SHALL BE CUT BACK WITH CLEAN, SHARP TOOLS AND TOPSOIL SHALL BE PLACED AROUND THE REMAINDER OF THE ROOTS. EXISTING TREES SHALL BE MONITORED ON A REGULAR BASIS FOR ADDITIONAL DAMAGE. BRANCHES AND ROOTS OF EXISTING TREES SHALL NOT BE LEFT EXPOSED FOR MORE THAN ONE (1) DAY. CONTRACTOR SHALL WATER EXISTING TREES AS NEEDED TO PREVENT SHOCK OR DECLINE.
- CONTRACTOR SHALL ARRANGE TO HAVE A UTILITY STAKE-OUT TO LOCATE ALL UNDERGROUND UTILITIES PRIOR TO INSTALLATION OF ANY LANDSCAPE MATERIAL. UTILITY COMPANIES SHALL BE CONTACTED THREE (3) DAYS PRIOR TO THE BEGINNING OF WORK.

5. TREE PROTECTION

- CONTRACTOR SHALL BE RESPONSIBLE FOR THE PROTECTION OF ALL EXISTING TREES TO REMAIN. A TREE PROTECTION ZONE SHALL BE ESTABLISHED TO PROTECT THE TRUNK OF ALL EXISTING TREES TO REMAIN. THE PROTECTION SHALL BE GREATER, LOCAL STANDARDS THAT MAY REQUIRE A MORE STRICT TREE PROTECTION ZONE SHALL BE HONORED.
- A FORTY-EIGHT INCH (48") HIGH WOODEN SNOW FENCE OR ORANGE COLORED HIGH-DENSITY VISI-FENCE, OR APPROVED EQUAL, MOUNTED ON STEEL POSTS SHALL BE PLACED ALONG THE BOUNDARY OF THE TREE PROTECTION ZONE. POSTS SHALL BE LOCATED AT A MAXIMUM OF EIGHT FEET (8') ON CENTER OR AS INDICATED WITHIN THE TREE PROTECTION DETAIL.
- WHEN THE TREE PROTECTION FENCING HAS BEEN INSTALLED, IT SHALL BE INSPECTED BY THE APPROVING AGENCY PRIOR TO THE BEGINNING OF CONSTRUCTION. THE FENCING ALONG THE TREE PROTECTION ZONE SHALL BE REGULARLY INSPECTED BY THE LANDSCAPE CONTRACTOR AND MAINTAINED UNTIL ALL CONSTRUCTION ACTIVITY HAS BEEN COMPLETED.
- AT NO TIME SHALL MACHINERY, DEBRIS, LAWN TREES OR OTHER MATERIALS BE PLACED, STOCKPILED OR LEFT STANDING IN THE TREE PROTECTION ZONE.

6. SOIL MODIFICATIONS

- CONTRACTOR SHALL ATTAIN A SOIL TEST FOR ALL AREAS OF THE SITE PRIOR TO CONDUCTING ANY PLANTING. SOIL ANALYSIS SHALL BE BY A CERTIFIED SOIL LABORATORY.
- CONTRACTOR SHALL BE RESPONSIBLE TO RETAIN WATER AND NUTRIENTS, THOROUGHLY TILL ORGANIC MATTER INTO THE TOP 6-12", USE COMPOSTED BARK, COMPOSTED LEAF MULCH OR PEAT MOSS. ALL PRODUCTS SHOULD BE COMPOSTED TO A DARK COLOR AND BE FREE OF PIECES WITH IDENTIFIABLE LEAF OR WOOD STRUCTURE. AVOID MATERIAL WITH A PH HIGHER THAN 7.5.
- MODIFY FINE GRADE DRAINAGE, MODIFY HEAVY CLAY OR SILT (MORE THAN 40% CLAY OR SILT) BY ADDING COMPOSTED PINE BARK (UP TO 30% BY VOLUME) AND/OR AGRICULTURAL GYPSUM. COARSE SAND MAY BE USED IF ENOUGH IS ADDED TO BRING THE SAND CONTENT TO MORE THAN 60% OF THE TOTAL MIX. SUBSURFACE DRAINAGE LINES MAY NEED TO BE ADDED TO INCREASE TO A MINIMUM OF 10% TO 15% DRAINAGE (SEE SPECIFICATION 6.4.1).
- MODIFY EXTREMELY SANDY SOILS (MORE THAN 85%) BY ADDING ORGANIC MATTER AND/OR DRY, SHREDDED PLANT LOAM UP TO 30% OF THE TOTAL MIX.

7. FINISHED GRADING

- UNLESS OTHERWISE CONTRACTED, THE LANDSCAPE CONTRACTOR SHALL BE RESPONSIBLE FOR THE INSTALLATION OF TOPSOIL AND THE ESTABLISHMENT OF FINE-GRADE WITHIN THE DISTURBANCE AREA OF THE SITE.
- LANDSCAPE CONTRACTOR SHALL VERIFY THAT SUBGRADE FOR INSTALLATION OF TOPSOIL HAS BEEN ESTABLISHED. THE SLOPE OF THE FINISHED GRADING SHALL BE AS SPECIFIED ON THE LANDSCAPE PLAN.
- ALL LAWN AND PLANTING AREAS SHALL BE GRADED TO A SMOOTH, EVEN AND UNIFORM PLANE WITH NO ABRUPT CHANGE OF SURFACE AS DEPICTED WITHIN THIS SET OF CONSTRUCTION PLANS, UNLESS OTHERWISE DIRECTED BY THE PROJECT ENGINEER OR LANDSCAPE ARCHITECT.
- ALL PLANTING AREAS SHALL BE GRADED AND MAINTAINED TO ALLOW FREE FLOW OF SURFACE WATER IN AND AROUND THE PLANTING BEDS. STANDING WATER SHALL NOT BE PERMITTED IN PLANTING BEDS.

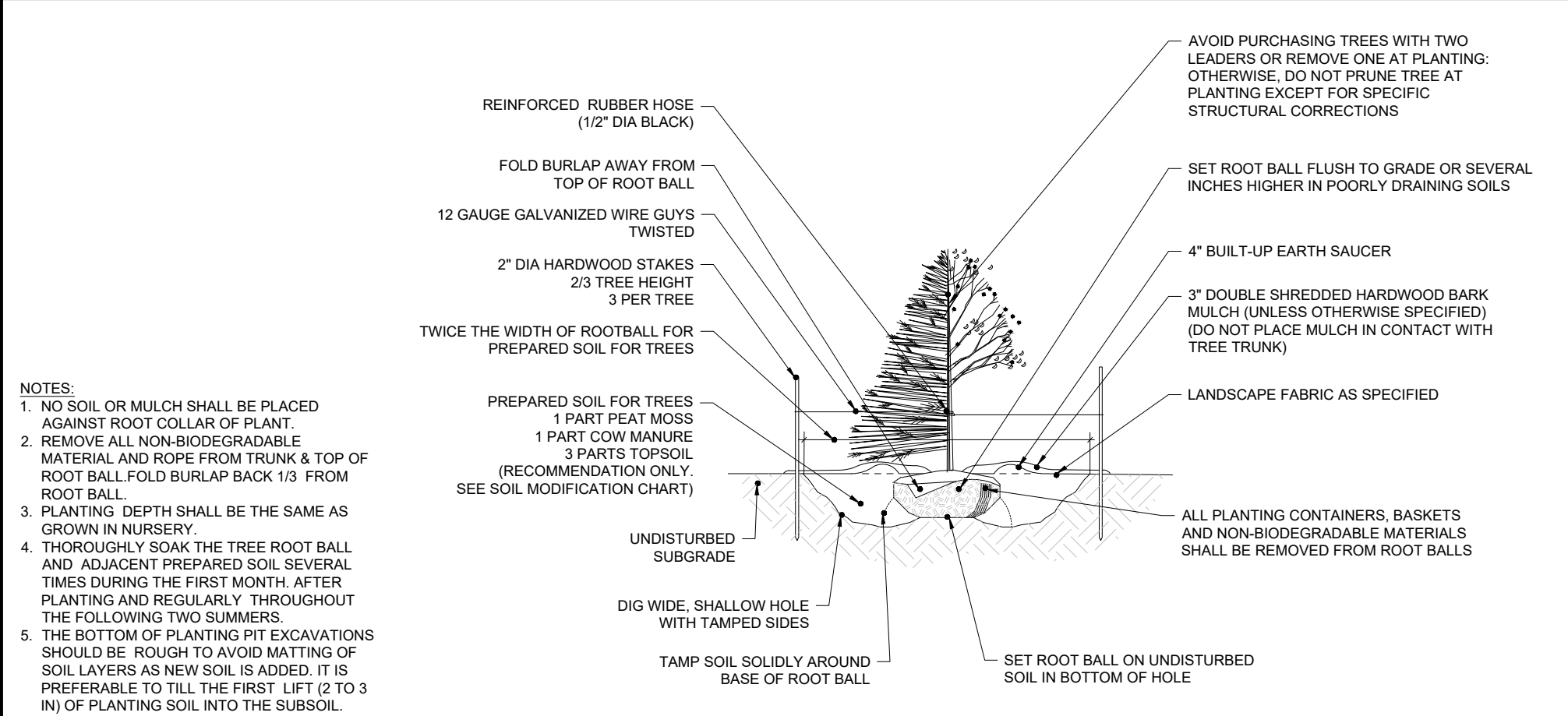
8. TOPSOILING

- CONTRACTOR SHALL PROVIDE A 6" THICK MINIMUM LAYER OF TOPSOIL, OR AS DIRECTED BY THE LOCAL ORDINANCE OR CLIENT, IN ALL PLANTING AREAS. TOPSOIL SHOULD BE SPREAD OVER A PREPARED SURFACE IN A UNIFORM LAYER TO ACHIEVE THE DESIRED COMPACTED THICKNESS.
- ON-SITE TOPSOIL MAY BE USED TO SUPPLEMENT THE TOTAL AMOUNT REQUIRED. TOPSOIL FROM THE SITE MAY BE REUSED IF IT HAS NOT BEEN CONTAMINATED, STORED AND PROTECTED PRIOR TO CONSTRUCTION.
- CONTRACTOR SHALL FURNISH TO THE APPROVING AGENCY AN ANALYSIS OF BOTH IMPORTED AND ON-SITE TOPSOIL TO BE UTILIZED IN ALL PLANTING AREAS. THE PH AND NUTRIENT LEVELS MAY NEED TO BE ADJUSTED THROUGH SOIL AMENDMENTS AS LONG AS THEY ARE NOT EXCEEDING THE SPECIFIED DELIVERY PLANTS THAT WILL NOT BE PLANTED IN AREAS.
- ALL LAWN AREAS ARE TO BE CULTIVATED TO A DEPTH OF SIX INCHES (6"). ALL DEBRIS EXPOSED FROM EXCAVATION AND ALL WASTE SHALL BE DISPOSED OF IN ACCORDANCE WITH GENERAL WORK PROCEDURES SECTION ABOVE.
- THE FOLLOWING SHALL BE FILLED INTO THE TOP FOUR INCHES (4") IN TWO DIRECTIONS (QUANTITIES BASED ON A 1,000 SQUARE FOOT AREA):
 - 20 POUNDS "GRO-POWER" OR APPROVED SOIL CONDITIONER/FERTILIZER
 - 20 POUNDS NITRO-FORM (COURSE) 38-0-0 BLUE CHIP OR APPROVED NITROGEN FERTILIZER
- THE SPREADINGS OF TOPSOIL SHALL NOT BE CONDUCTED UNDER MUDDY OR FROZEN CONDITIONS.

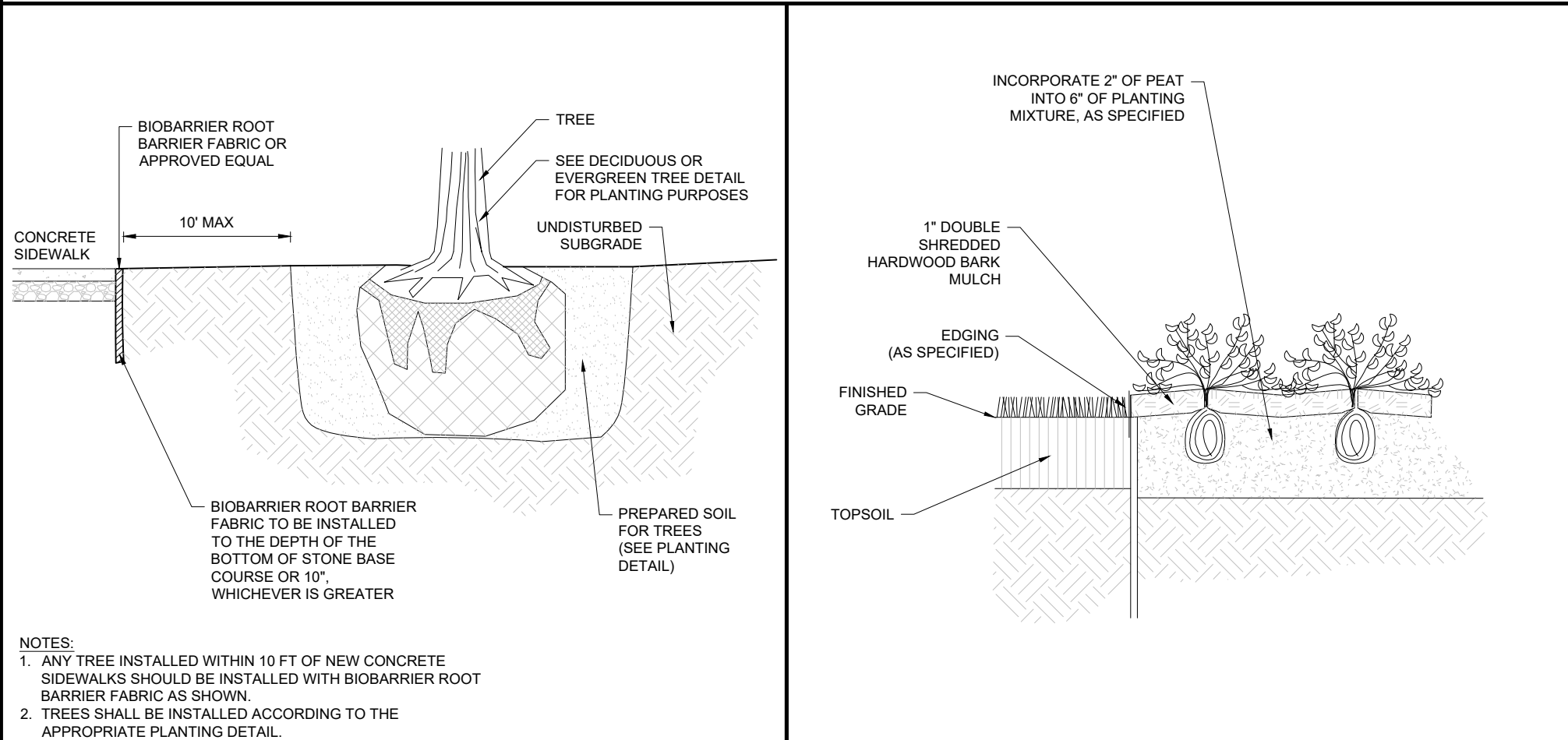
9. PLANTING

- INSOFAF THAT IT IS FEASIBLE, PLANT MATERIAL SHALL BE PLANTED ON THE DAY OF DELIVERY. IN THE EVENT THAT THIS IS NOT POSSIBLE, LANDSCAPE CONTRACTOR SHALL PROTECT UNINSTALLED PLANT MATERIAL. PLANTS SHALL NOT REMAIN EXPOSED FOR LONGER THAN THREE DAYS. PLANTS SHALL BE HEADED AND COVERED WITH TOPSOIL OR MULCH TO HELP PRESERVE ROOT MOISTURE.
- PLANTING OPERATIONS SHALL BE PERFORMED DURING PERIODS WITHIN THE PLANTING SEASON WHEN WEATHER AND SOIL CONDITIONS ARE SUITABLE AND IN ACCORDANCE WITH ACCEPTED LOCAL PRACTICE. PLANTS SHALL NOT BE INSTALLED IN TOPSOIL THAT IS IN A MUDDY OR FROZEN CONDITION.
- ANY INJURED ROOTS OR BRANCHES SHALL BE PRUNED TO MAKE CLEAN-CUT ENDS PRIOR TO PLANTING UTILIZING CLEAN, SHARP TOOLS. ONLY INJURED ROOTS SHALL BE REMOVED.
- ALL PLANTING CONTAINERS, BASKETS AND NON-BIODEGRADABLE MATERIALS SHALL BE REMOVED FROM ROOT BALLS DURING PLANTING. NATURAL FIBER BURLAP SHALL BE CUT FROM AROUND THE TRUNK OF THE TREE AND FOLDED DOWN TO THE FEET OF THE TREE.
- POSITION TREES AND SHRUBS AT THEIR INTENDED LOCATIONS AS PER THE PLANS AND SECURE THE APPROVAL OF THE LANDSCAPE ARCHITECT PRIOR TO EXCAVATING PITS, MAKING NECESSARY ADJUSTMENTS AS DIRECTED.
- PRIOR TO THE ISSUANCE OF ANY CERTIFICATE OF OCCUPANCY, THE PROPOSED LANDSCAPE, AS SHOWN ON THE PROPOSED LANDSCAPE PLAN, SHALL BE THOROUGHLY INSPECTED AND APPROVED BY THE APPROVING AGENCY. THE APPROVING AGENCY SHALL TAKE INTO ACCOUNT SEASONAL CONSIDERATIONS IN THIS REGARD AS FOLLOWS. THE PLANTING OF TREES, SHRUBS, VINES OR GROUND COVER SHALL OCCUR ONLY DURING THE FOLLOWING PLANTING SEASONS:
 - PLANTS: MARCH 15 TO DECEMBER 15
 - LAWN: MARCH 15 TO JUNE 15 OR SEPTEMBER 1 TO DECEMBER 1
- PLANKS REQUIRED FOR A CERTIFICATE OF OCCUPANCY SHALL BE PROVIDED DURING THE NEXT APPROPRIATE SEASON AT THE MUNICIPALITY'S DISCRETION. CONTRACTOR SHOULD CONTACT APPROVING AGENCY FOR POTENTIAL SUBSTITUTIONS.

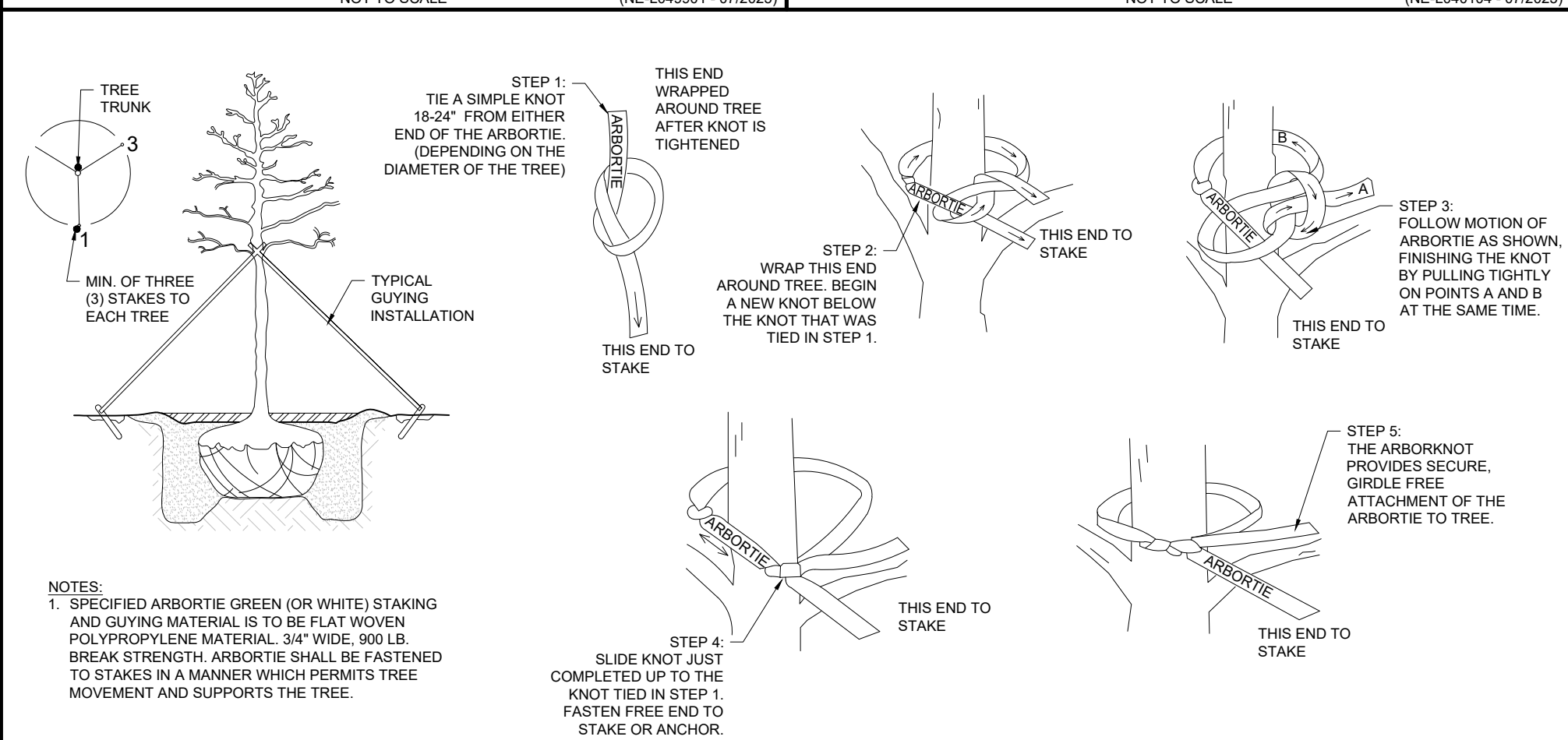
9. FURTHERMORE, THE FOLLOWING TREE VARIETIES ARE USUALLY SUSCEPTIBLE TO WINTER DIEBACK. WITH TRANSPLANT SHOCK AND THE SEASONAL LACK OF NITROGEN AVAILABILITY, THE RISK OF PLANT DEATH IS GREATLY INCREASED. IT IS NOT RECOMMENDED THAT THESE SPECIES BE PLANTED DURING THE FALL PLANTING SEASON:
- | | |
|-------------------------|-----------------------|
| ACER RUBRUM | PLATANUS X ACERIFOLIA |
| BETULA VARIETIES | POPULUS VARIETIES |
| CARPINUS VARIETIES | PRUNUS VARIETIES |
| CRATAEGUS VARIETIES | PYRUS VARIETIES |
| SOELETERRIA | QUERCUS VARIETIES |
| LIQUIDAMBAR STYRACIFLUA | TILIA TOMENTOSA |
| LINDENRODRUM TULIPIFERA | ZELKOVA VARIETIES |
10. PLANTING PITTS SHALL BE DUG WITH LEVEL BOTTOMS, WITH THE WIDTH TWICE THE DIAMETER OF ROOT BALL. THE ROOT BALL SHALL REST ON UNDISTURBED GRADE. EACH PLANT PIT SHALL BE BACKFILLED IN LAYERS WITH THE FOLLOWING PREPARED SOIL MIXED THOROUGHLY:
- 1 PART TOP SOIL MOSS
 - 1 PART COMPOSTED COW MANURE BY VOLUME
 - 3 PARTS PEAT BY VOLUME
 - 21 GRAMS AGRIFORM PLANTING TABLETS (OR APPROVED EQUAL) AS FOLLOWS:
 - 2 TABLETS PER 1 GALLON PLANT
 - 3 TABLETS PER 5 GALLON PLANT
 - 4 TABLETS PER 15 GALLON PLANT
 - LARGER PLANTS: 2 TABLETS PER 1/2" CALIPER OF TRUNK
11. FILL PREPARED SOIL AROUND BALL OF PLANT HALF-WAY AND INSERT PLANT TABLETS. COMPLETE BACKFILL AND WATER THOROUGHLY.
12. ALL PLANTS SHALL BE PLANTED SO THAT THE TOP OF THE ROOT BALL, THE POINT AT WHICH THE ROOT FLARE BEGINS, IS SET AT GROUND LEVEL, AND IN THE CENTER OF THE PIT. NO SLO. IS TO BE PLACED DIRECTLY ON TOP OF THE ROOT BALL. ALL EXISTING TREES TO BE REMOVED SHALL BE CUT AT AN ANGLE NEARLY HORIZONTAL OR DRIVEWAYS SHALL BE PRUNED AND MAINTAINED TO A MINIMUM BRANCHING HEIGHT OF 7' FROM GRADE.
13. GROUND COVER AREAS SHALL RECEIVE A 1/2" LAYER OF HUMUS RAKED INTO THE TOP 1" OF PREPARED SOIL PRIOR TO PLANTING. ALL GROUND COVER AREAS SHALL BE WEEDED AND TREATED WITH A PRE-EMERGENT CHEMICAL AS PER MANUFACTURER'S RECOMMENDATION.
14. NO PLANT, EXCEPT GROUND COVERS, GRASSES OR VINES, SHALL BE PLANTED LESS THAN TWO FEET (2') FROM EXISTING STRUCTURES AND SIDEWALKS.
15. ALL PLANTING AREAS AND PLANTING PITTS SHALL BE SPECIFIED HEREIN TO FILL THE ENTIRE BARE AREA OR SAUCER. NO MULCH IS TO TOUCH THE TRUNK OF THE TREE OR SHRUB.
16. ALL PLANTING AREAS SHALL BE WATERED IMMEDIATELY UPON INSTALLATION IN ACCORDANCE WITH THE WATERING SPECIFICATIONS AS LISTED HEREIN.
17. TRANSPLANTING (WHEN REQUIRED):
- ALL TRANSPLANTS SHALL BE DUG WITH INTACT ROOT BALLS CAPABLE OF SUSTAINING THE PLANT.
 - IF PLANTS ARE TO BE STOCKPILED BEFORE REPLANTING, THEY SHALL BE HEADED IN WITH MULCH OR SOIL, ADEQUATELY WATERED AND PROTECTED FROM EXTREME HEAT, SUN AND WIND.
 - TRANSPLANTS SHALL NOT BE DUG FOR TRANSPLANTING BETWEEN APRIL 10 AND JUNE 30.
 - UPON REPLANTING, BACKFILL SOIL SHALL BE AMENDED WITH FERTILIZER AND ROOT GROWTH HORMONE.
 - TRANSPLANTS SHALL BE GUARANTEED FOR THE LENGTH OF THE GUARANTEE PERIOD SPECIFIED HEREIN.
 - F TRANSPLANTS DIE, SHRUBS AND TREES LESS THAN SIX INCHES (6") DBH SHALL BE REPLACED IN KIND. TREES GREATER THAN SIX INCHES DBH ARE TO BE REPLACED IN ACCORDANCE WITH THE MUNICIPALITY'S TREE REPLACEMENT GUIDELINES.
18. WATERING
- NEW PLANTINGS OR LAWN AREAS SHALL BE ADEQUATELY IRRIGATED BEGINNING IMMEDIATELY AFTER PLANTING. WATER SHALL BE APPLIED TO EACH TREE AND SHRUB IN SUCH MANNER AS NOT TO DISTURB BACKFILL AND TO THE EXTENT THAT ALL MATERIALS IN THE PLANTING HOLES ARE THOROUGHLY SATURATED. WATERING SHALL CONTINUE AT LEAST UNTIL PLANTS ARE ESTABLISHED.
 - TRANSPLANTS SHALL BE SUPPLIED WITH WATER IF AVAILABLE ON SITE AT TIME OF PLANTING. IF WATER IS NOT AVAILABLE ON SITE, CONTRACTOR SHALL SUPPLY ALL NECESSARY WATER. THE USE OF WATERING BAGS IS RECOMMENDED FOR ALL NEWLY PLANTED TREES.
 - IF AN IRRIGATION SYSTEM HAS BEEN INSTALLED ON THE SITE, IT SHALL BE USED TO WATER PROPOSED PLANT MATERIAL, BUT ANY FAILURE OF THE SYSTEM DUE TO CONTRACTOR'S RESPONSIBILITY OF MAINTAINING THE DESIRED MOISTURE LEVEL FOR VIGOROUS, HEALTHY GROWTH.
19. GUARANTEE
- THE LANDSCAPE CONTRACTOR SHALL GUARANTEE ALL PLANTS FOR A PERIOD OF 1 YEAR FROM APPROVAL OF LANDSCAPE INSTALLATION BY THE APPROVING AGENCY. CONTRACTOR SHALL SUPPLY THE OWNER WITH A MAINTENANCE BOND FOR TEN PERCENT (10%) OF THE VALUE OF THE LANDSCAPE INSTALLATION WHICH WILL BE RELEASED AT THE CONCLUSION OF THE GUARANTEE PERIOD AND WHEN A FINAL INSPECTION HAS BEEN COMPLETED AND APPROVED BY THE OWNER OR AUTHORIZED REPRESENTATIVE.
 - ANY DEAD OR DYING PLANTS SHALL BE REPLACED FOR THE LENGTH OF THE GUARANTEE PERIOD.
 - REPLACEMENT OF PLANT MATERIAL SHALL BE CONDUCTED AT THE FIRST SUCCEEDING PLANTING SEASON. ANY DEBRIS SHALL BE DISPOSED OF OFF-SITE, WITHOUT EXCEPTION.
 - THE CONTRACTOR SHALL BE RESPONSIBLE TO THE CITY OF CHICAGO DURING CONSTRUCTION AND THROUGHOUT THE 90 DAY MAINTENANCE PERIOD AS SPECIFIED HEREIN. CULTIVATION, WEEDING, WATERING AND THE PREVENTATIVE TREATMENTS SHALL BE PERFORMED AS NECESSARY TO KEEP PLANT MATERIAL IN GOOD CONDITION AND FREE OF INSECTS AND DISEASE.
 - LAWN SHALL BE MAINTAINED THROUGH WATERING, FERTILIZING, WEEDING, MOWING, TRIMMING AND OTHER OPERATIONS SUCH AS ROLLING, REGARDING AND REPLANTING AS REQUIRED TO ESTABLISH A SMOOTH, ACCEPTABLE LAWN, FREE OF EROSION OR BARE AREAS.
20. CLEANUP
- UPON THE COMPLETION OF ALL LANDSCAPE INSTALLATION AND BEFORE THE FINAL ACCEPTANCE, THE CONTRACTOR SHALL REMOVE ALL UNUSED MATERIALS, EQUIPMENT AND DEBRIS FROM THE SITE. ALL PAVED AREAS ARE TO BE CLEANED.
 - THE SITE SHALL BE CLEANED AND LEFT IN A NEAT AND ACCEPTABLE CONDITION AS APPROVED BY THE OWNER OR AUTHORIZED REPRESENTATIVE.
21. MAINTENANCE (ALTERNATIVE BID):
- A 90 DAY MAINTENANCE PERIOD SHALL COMMENCE AT THE END OF ALL LANDSCAPE INSTALLATION OPERATIONS. THE 90 DAY MAINTENANCE PERIOD SHALL BE THE RESPONSIBILITY OF THE OWNER. OPERATOR SHALL BE THE NEWLY INSTALLED LANDSCAPING HAS BEEN MAINTAINED AS SPECIFIED ON THE APPROVED LANDSCAPE PLAN, UNTIL THE INITIAL 90 DAY MAINTENANCE PERIOD HAS EXPIRED. THE OPERATOR MAY REQUEST THAT BIDDERS SUBMIT AN ALTERNATE MAINTENANCE BID FOR A MONTHLY MAINTENANCE CONTRACT. THE ALTERNATE MAINTENANCE CONTRACT WILL ENCOMPASS ANY WORK THAT IS REQUIRED AND APPROVED BY THE CITY OF CHICAGO TO ENSURE THAT PLANT AND LAWN AREAS ARE HEALTHY AND MAINTAINED TO THE APPROVAL OF THE OPERATOR.



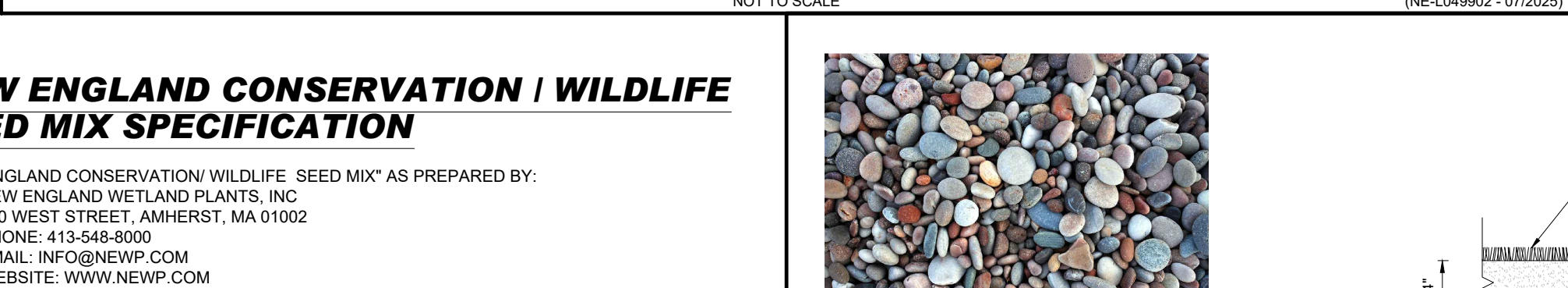
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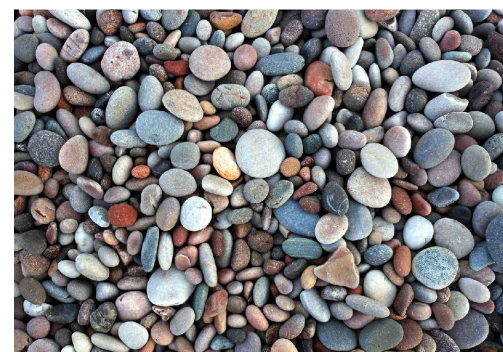


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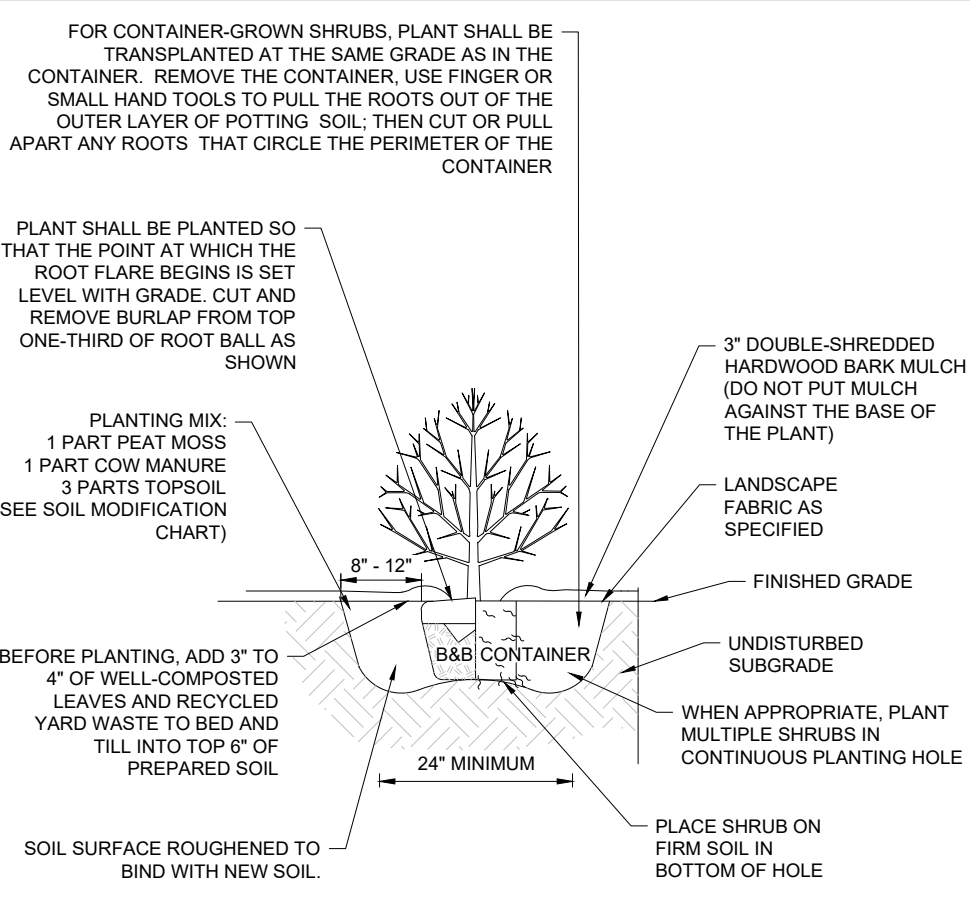
"NEW ENGLAND CONSERVATION/ WILDLIFE SEED MIX" AS PREPARED BY
NEW ENGLAND WETLAND PLANTS, INC
820 WEST STREET, AMHERST, MA 01002
PHONE: 413-548-8000
EMAIL: INFO@NEWP.COM
WEBSITE: WWW.NEWP.COM

- APPLICATION RATE: 1750 SFLB OR 25LSA/ACRE
1. MINIMUM ORDER: 2 LBS
2. SPECIES:
a) VIRGINIA WILD RYE (ELYMUS VIRGINICUS)
b) LITTLE BLUESTEM (SCYTHACHYRIUM SCOPARIUM)
c) BIG BLUESTEM (ANDROPOGON GERARDI)
d) RED FESCUE (FESTUCA RUBRA)
e) SWITCH GRASS (ANICHNUM SP.)
f) PARTRIDGE PEA (CHAMAECRISTA FASCICULATA)
g) PANICLELEAF TICK TREFOIL (DESMODIUM PANICULATUM)
h) INDIAN GRASS (SORGHASTRUM NUTANS)
i) BLUE VERVAIN (VERVAINA SP.)
j) BUTTERFLY MILKWEED (ASCLEPIAS TUBEROSA)
k) BLACK EYED SUSAN (RUBICECIA HIRTA)
l) COMMON SNEEZEWEED (HELIENIUM AUTUNALE)
m) WHITE ASTER (ASTER SP.)
n) EARLY GOLDENROD (SOLIDAGO JUNCEA)
o) UPLAND BENTGRASS (AGROSTIS PERENNANS)

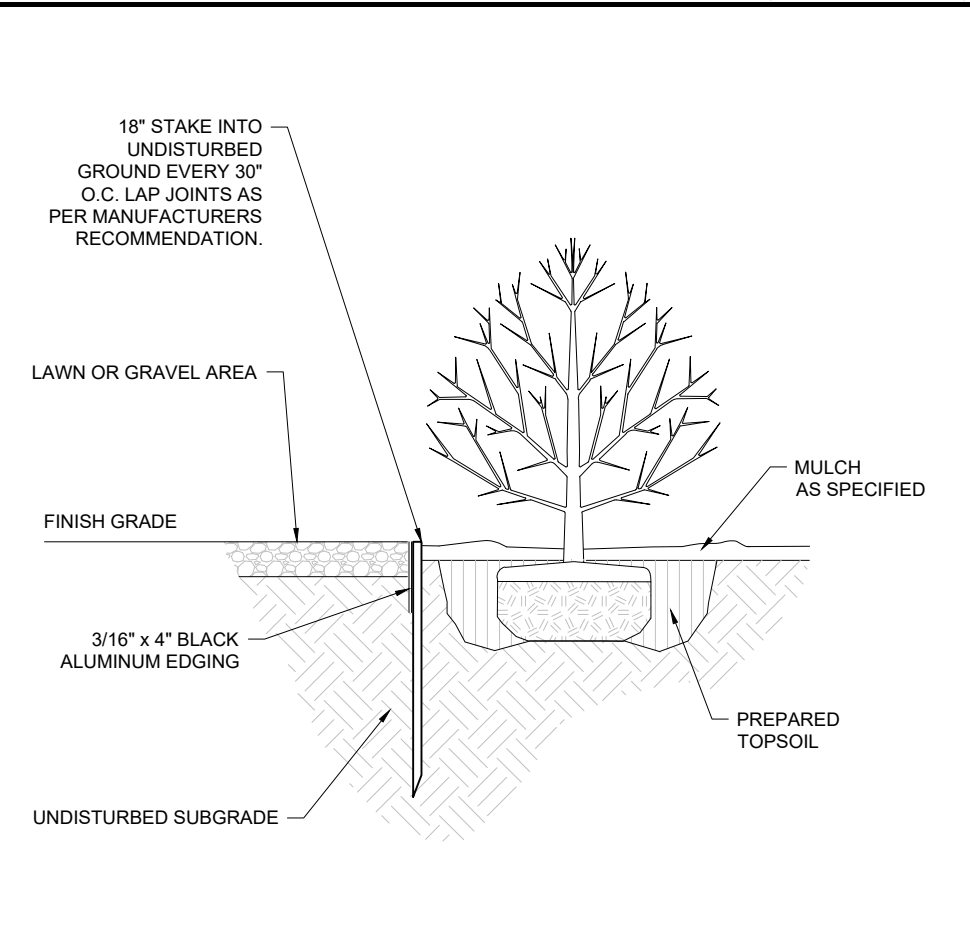


- SPECIFICATIONS**
NAME: 2'-3" RIVER BED STONE
COLOR: COLORS WILL BE BROWNS, GREYS, AND TANS WITH LITTLE LILACS OR BURGUNDY TONES.
SHAPE: PRIMARILY ROUND OR OVAL SCREENED STONE WITH NO SHARP ANGULAR SIDES OR FACES.
SIZE: STONE SIZES WILL RANGE FROM 2'-3" IN AT LEAST ONE DIMENSION. STONE SIZING SHOULD BE UNIFORM WITH LITTLE VARIATION FROM THIS RANGE.
SILT CONTENT: STONE NEEDS TO BE CLEAN OF DEBRIS AND SILT AT TIME OF DELIVERY.

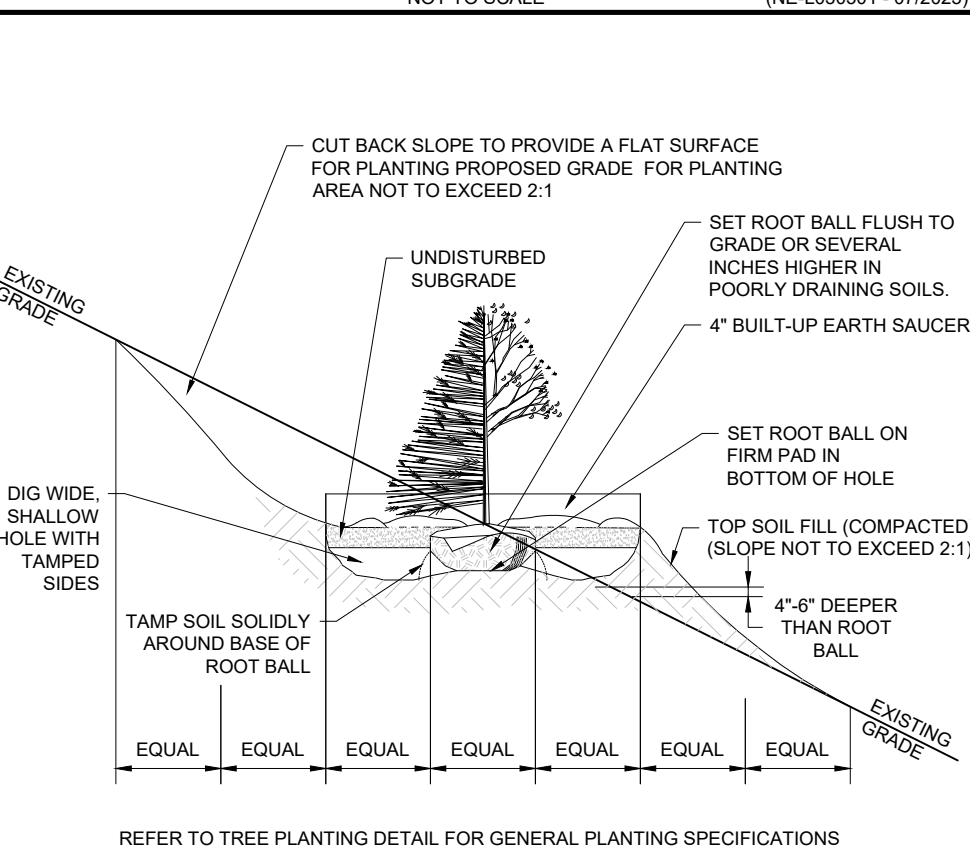
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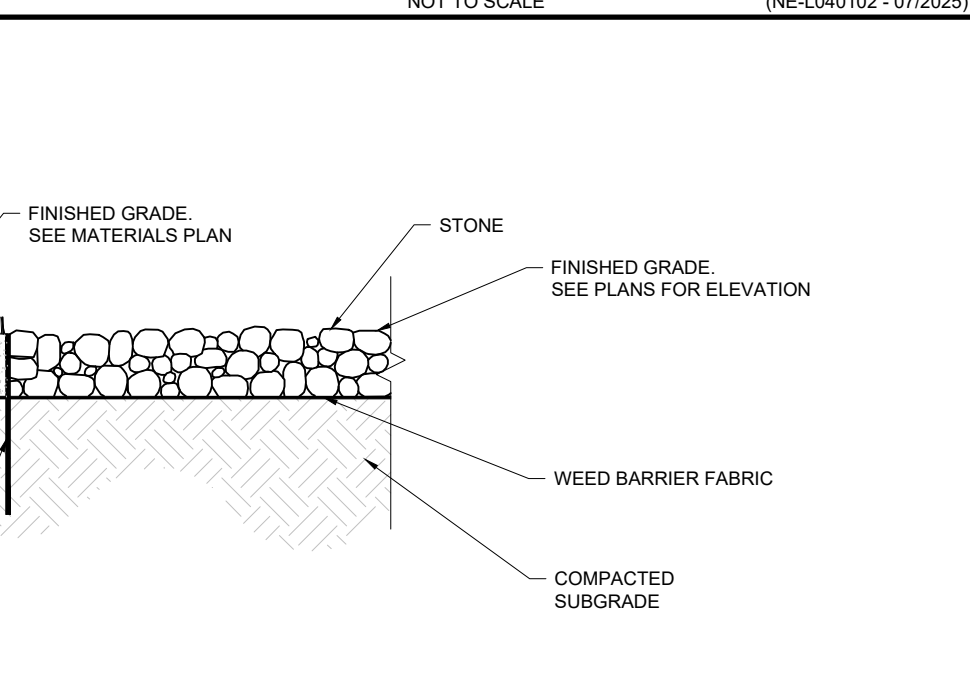
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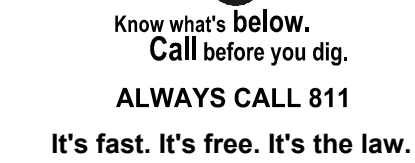
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REVISIONS

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PROJECT No.:	MAA250027.00-0E
DRAWN BY:	JWT/SJR
CHECKED BY:	NPD/NEM
DATE:	12/16/2025
AD I.D.:	P-CIVL-LSCP

PROJECT:

_____ FC

**FOXBORO
LEARNING,
LLC**

**PROPOSED
CHILD CARE CENTER**

**CAMBRIDGE STREET
TOWN OF BURLINGTON
MIDDLESEX COUNTY
MASSACHUSETTS
PARCEL ID: 35-115-0**



**50 WASHINGTON ST., SUITE 2000
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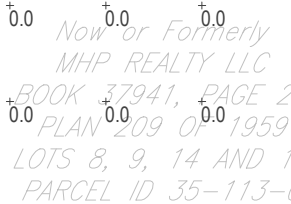


HEET TITLE:

HEET NUMBER

L-102

ORG. DATE - 12/16/2025



NOTE: FINAL LOCATION AND SPECIFICATION OF BUILDING MOUNTED LIGHTING PER ARCHITECTURAL PLANS. LIGHTS ARE SHOWN AND MODELED FOR ILLUSTRATIVE PURPOSES ONLY

1. THE GENERAL NOTES, FOUND ON THE NOTES PAGE OF THIS PLAN SET, MUST BE INCLUDED AS PART OF THIS ENTIRE DOCUMENT PACKAGE AND ARE PART OF THE CONTRACT DOCUMENTS. THE ELECTRICAL CONTRACTOR MUST BECOME FAMILIAR WITH, AND FULLY COMPLY WITH, THESE NOTES. IN THE EVENT OF A CONFLICT, THE NOTES SHALL PREVAIL.
2. THE ELECTRICAL CONTRACTOR MUST COMPLY WITH ALL APPLICABLE CONTRACTOR REQUIREMENTS INDICATED IN THIS LIGHTING PLAN, INCLUDING BUT NOT LIMITED TO GENERAL NOTES, GRADING AND UTILITY NOTES, SITE SAFETY, AND ALL APPLICABLE AGENCY AND GOVERNMENTAL REGULATIONS. THE LIGHTING PLAN DEPICTS PROPOSED, BUT NOT LIMITED TO, THE LOCATION, TYPE, AND SPECIFICATIONS OF THE NOTES MANUFACTURER(S) ACTUAL SUSTAINED SITE ILLUMINATION LEVELS AND PERFORMANCE OF LUMINAIRES MAY DIFFER FROM THE VALUES DEPICTED ON THIS PLAN DUE TO VARIATIONS IN WEATHER, ELECTRICAL, VOLTAGE, TOLERANCE IN LAMPS, MAINTENANCE, THE SERVICE LIFE OF EQUIPMENT AND LUMINAIRES, EXISTING AMBIENT LIGHT, AND OTHER FACTORS. THE CONTRACTOR SHALL VARY THE VARIABLE FIELD CONDITIONS.
3. THE LIGHTING VALUES AND CALCULATION POINTS DEPICTED ON THIS PLAN ARE ANALYZED ON A HORIZONTAL GEOMETRIC PLANE AT GROUND LEVEL UNLESS OTHERWISE NOTED. ILLUMINATION LEVELS ARE SHOWN IN FOOTCANDLES (FC). THE LIGHTING PLAN IS INTENDED TO SHOW THE LOCATIONS AND TYPE OF LUMINAIRES, POLES, AND CABLES, CONDUITS, AND WIRING. THE LOCATION OF ALL ELECTRICAL COMPONENTS ARE SOLELY THE ARCHITECT'S, ELECTRICAL ENGINEER'S AND/OR ELECTRICAL CONTRACTOR'S RESPONSIBILITY. AS INDICATED IN THE CONSTRUCTION CONTRACT DOCUMENTS, THE CONTRACTOR MUST COORDINATE WITH THE PROJECT ARCHITECT AND/OR ELECTRICAL ENGINEER REGARDING ALL OTHER SOURCES OF LIGHT AND TIMING OF ANY NEEDS TO MEET THE LIGHTING DESIGN. THESE ITEMS MUST BE INSTALLED AS REQUIRED BY FEDERAL, STATE AND LOCAL REGULATIONS. CONTRACTOR IS RESPONSIBLE FOR THE INSTALLATION OF LIGHTING FIXTURES AND APPURTENANCES IN ACCORDANCE WITH THE NATIONAL ELECTRIC CODE (NEC) AND ALL APPLICABLE CODES.
4. THE ELECTRICAL CONTRACTOR SHALL BE RESPONSIBLE FOR THE LOCATION OF ALL LIGHT LOCATIONS THAT CONFLICT WITH DRAINAGE, UTILITIES, OR ANY OTHER STRUCTURE(S) TO THE PROFESSIONAL OF RECORD'S ATTENTION, PRIOR TO THE START OF CONSTRUCTION.
5. THE ELECTRICAL CONTRACTOR IS RESPONSIBLE TO ENSURE THAT ALL LIGHTING IS INSTALLED PER THIS LIGHTING PLAN, INCLUDING THE LOCATION, ORIENTATION, SHIELDING, AND/OR ROTATED OPTICS IN ORDER TO ACHIEVE THE LIGHTING LEVELS DEPICTED ON THIS PLAN. EXISTING POLES AND FOUNDATIONS ARE NOT TO BE REUSED. UPON OWNER'S ACCEPTANCE OF THE COMPLETED PROJECT, THE OWNER SHALL BE RESPONSIBLE FOR THE MAINTENANCE OF THE LIGHTING SYSTEM AND ALL OF ITS COMPONENTS AND RELATED SYSTEMS. THE LUMINAIRES, LAMPS AND LENSES MUST BE REGULARLY INSPECTED/MAINTAINED TO ENSURE THAT THEY FUNCTION PROPERLY. THIS WORK SHOULD INCLUDE, BUT IS NOT LIMITED TO, VISUAL OBSERVATION, CLEANING OF LENSES, AND OTHER MAINTENANCE REQUIRED BY THE MANUFACTURER. THE CONTRACTOR SHALL BE RESPONSIBLE TO CORRECT OR REPLACE IN PROPER LIGHT DISTRIBUTION AND FAILURE TO COMPLY WITH THE APPROVED DESIGN.
6. THE LIGHT LOSS FACTORS (LLF) DEPICTED IN THE LUMINAIRE SCHEDULE ON THIS PLAN ARE BASED ON DATA PROVIDED BY THE MANUFACTURER FOLLOWING IES LM-80-21 TESTING (OR MOST RECENT EDITION) OF THE LUMINAIRE DEPICTED ON THIS PLAN. THE LLF WERE CALCULATED BASED ON THE LLF LISTED IN THE LUMINAIRE SCHEDULE.
7. PRIOR TO CONSTRUCTION, THE CONTRACTOR SHALL PROVIDE SUBMITTALS TO BOHLER FOR REVIEW AND APPROVAL. SUBSTITUTION REQUESTS MUST BE ACCOMPANIED BY A PHOTOGRAPHIC STUDY OF THE EXISTING CONDITIONS. THE PHOTOGRAPHIC STUDY QUESTION WILL MEET THE DESIGN INTENT OF THIS PLAN. SUBSTITUTION REQUESTS WITHOUT A PHOTOGRAPHIC STUDY WILL BE REJECTED.
8. LIGHT POLE FOUNDATIONS ARE SHOWN ON THE PLAN IN THE INTENDED LOCATION BASED ON THE LIGHTING CALCULATIONS, UNLESS OTHERWISE NOTED. LIGHT SYMBOLS ARE SHOWN LARGER THAN ACTUAL SIZE, HOWEVER FOUNDATION SIZE IS SHOWN AT ACTUAL SIZE.

REVISIONS



ALWAYS CALL 811
It's fast. It's free. It's the law.

THIS DRAWING IS NOT INTENDED AS A CONSTRUCTION DOCUMENT
UNLESS INDICATED OTHERWISE

PROJECT No.:	MAA250027.00-0E
DRAWN BY:	JWT/SJR
CHECKED BY:	NPD/NEM
DATE:	12/16/2025
CAD I.D.:	P-CIVL-LGHT

PROJECT:

**FOXBORO
LEARNING,
LLC**

**PROPOSED
CHILD CARE CENTER**

**CAMBRIDGE STREET
TOWN OF BURLINGTON
MIDDLESEX COUNTY
MASSACHUSETTS
PARCEL ID: 35-115-0**

BOHLERTM

**50 WASHINGTON ST., SUITE 2000
WESTBOROUGH, MA 01581**
Phone: (508) 480-9900

www.BohlerEngineering.com

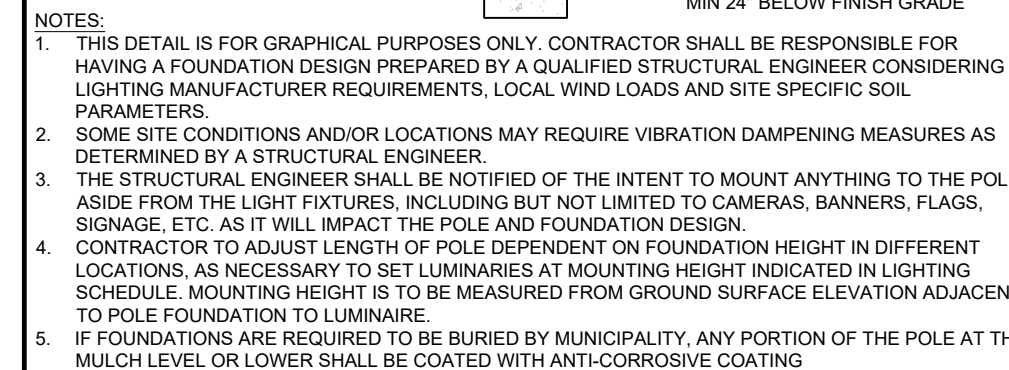


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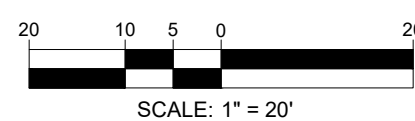
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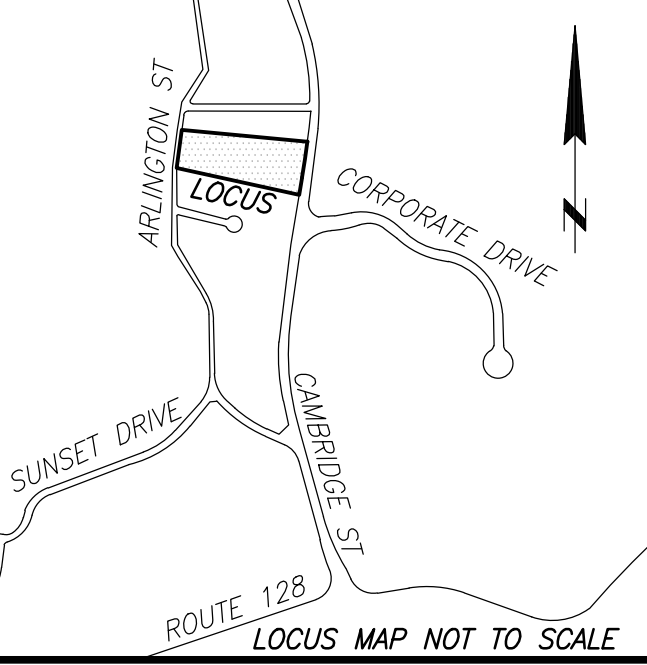
L-201

ORG. DATE - 12/16/2025



**THIS PLAN TO BE UTILIZED
FOR LIGHTING PURPOSES
ONLY**





I CERTIFY THAT THIS PLAN IS BASED ON AN ACTUAL FIELD SURVEY AND THE LATEST PLANS AND DEEDS OF RECORD.

Timothy R. Aguirre
TIMOTHY R. AGUIRRE, PLS (MAJ 52782)
TAGUIRRE@FELDMANGEO.COM
4-1-25
DATE



DRAWING NAME:

**EXISTING CONDITIONS
PLAN OF LAND**

0 CAMBRIDGE STREET
BURLINGTON, MASS.

DATE: MARCH 7, 2025

REVISIONS:

RESEARCH: TRA/TH	FIELD CHIEF: GH/AM
PROJ MGR: TRA	APPROVED: TH
CALC: TH	CAD: TH
FIELD CHK: CHD	FILE: 2500110

FILENAME: 2500110-EC.dwg

20 0 10 20 40
SCALE: 1"=20'

SHEET NO. 1 OF 1

ANNA (PUBLIC - 40' WIDE) ROAD

ARLINGTON STREET (PUBLIC - VARIABLE WIDTH)

CAMBRIDGE STREET (PUBLIC - VARIABLE WIDTH) (STATE ROUTE 128) (SALO NO. 300)

Now or Formerly
EDWARD R. BARKLEY
BOOK 46329, PAGE 83
PLAN 1736 OF 1951
LOT 1
PARCEL ID 35-119-0

Now or Formerly
MARIA LOTTATORE
NO LEGAL REFERENCE FOUND
PLAN 1736 OF 1951
LOT 1
PARCEL ID 35-118-0

Now or Formerly
GEORGE & ELIZABETH
2500K ITS
BOOK 45095, PAGE 64
PLAN 1736 OF 1951
LOT 2
PARCEL ID 35-117-0

Now or Formerly
RAMESH K. MANDU
BOOK 29652, PAGE 107
PLAN 1736 OF 1951
LOT 1
PARCEL ID 35-116-0

Now or Formerly
MICHAEL CHAPIN & J. MADDEN
BOOK 72987, PAGE 370
PLAN 1796 OF 1947
LOT 1
PARCEL ID 35-115-0
**AREA=166,584 SQ. FT.
3.824 ACRES**

Now or Formerly
SUE A. MAYO
NO LEGAL REFERENCE FOUND
PLAN 209 OF 1959
LOT 1
PARCEL ID 35-126-0

Now or Formerly
MIP REALTY LLC
BOOK 37941, PAGE 23
PLAN 209 OF 1959
LOTS 8, 9, 14 AND 15
PARCEL ID 35-113-0

NOTES:

- BENCH MARK INFORMATION:
BENCH MARK USED:
ELEVATIONS WERE OBTAINED BY GPS OBSERVATIONS ON FEBRUARY 18, 2025
TEMPORARY BENCH MARKS SET:
TBM-AM1: REAL ROAD SPIKE SET IN UTILITY LIGHT POLE
ELEVATION = 219.41
TBM-AM2: X-CUT ON RIGHT HYDRANT BOLT ABOVE MAIN OUTLET
ELEVATION = 217.88
- ELEVATIONS REFER TO THE NORTH AMERICAN VERTICAL DATUM OF 1988 (NAVD83).
- CONTOUR INTERVAL EQUALS ONE (1) FOOT.
- BY GRAPHIC PLOTTING ONLY, THE PARCEL SHOWN HEREON LIES WITHIN A ZONE "X" (UNSHADED), AN AREA OUTSIDE OF THE 0.2% ANNUAL CHANCE FLOOD, AS SHOWN ON THE FEDERAL EMERGENCY MANAGEMENT AGENCY (FEMA) FLOOD INSURANCE RATE MAP (FIRM) FOR MIDDLESEX COUNTY, MASSACHUSETTS, MAP NUMBER 25017C0402E, TOWN OF BURLINGTON COMMUNITY NUMBER 250185, PANEL NUMBER 0402E, HAVING AN EFFECTIVE DATE OF JUNE 4, 2010.
- UTILITY INFORMATION SHOWN IS BASED ON BOTH A FIELD SURVEY AND PLANS OF RECORD. THE LOCATIONS OF UNDERGROUND PIPES AND CONDUITS HAVE BEEN DETERMINED FROM THE AFOREMENTIONED RECORD PLANS AND ARE APPROXIMATE ONLY. WE CANNOT ASSUME RESPONSIBILITY FOR DAMAGES INCURRED AS A RESULT OF UTILITIES THAT ARE OMITTED OR INCORRECTLY SHOWN ON SAID RECORD PLANS, SINCE SUBSURFACE UTILITIES CANNOT BE VISIBLY VERIFIED BEFORE PLANNING FUTURE CONNECTIONS. THE PROPER UTILITY ENGINEERING DEPARTMENT SHOULD BE CONSULTED AND THE ACTUAL LOCATION OF SUBSURFACE STRUCTURES SHOULD BE DETERMINED IN THE FIELD. CALL, TOLL FREE, THE DIG SAFE CALL CENTER AT 1-888-344-7233 SEVENTY-TWO HOURS PRIOR TO EXCAVATION.
- THIS PLAN WAS PREPARED WITHOUT THE BENEFIT OF A TITLE REPORT AND IS SUBJECT TO ANY FINDINGS SUCH A REPORT MIGHT DISCLOSE.
- WETLAND SHOWN WERE FIELD DELINEATED BY OXROW ASSOCIATES, INC. P.O. BOX 971, ACTON, MA 01720, DATED JUNE 29, 2018.
- THIS DOCUMENT IS AN INSTRUMENT OF SERVICE OF FELDMAN GEOSPATIAL ISSUED TO OUR CLIENT FOR PURPOSES RELATED DIRECTLY AND SOLELY TO FELDMAN GEOSPATIAL'S SCOPE OF SERVICES UNDER CONTRACT TO OUR CLIENT FOR THIS PROJECT. ANY USE OR REUSE OF THIS DOCUMENT FOR ANY REASON BY ANY PARTY FOR PURPOSES UNRELATED DIRECTLY AND SOLELY TO SAID CONTRACT SHALL BE AT THE USER'S SOLE AND EXCLUSIVE RISK AND LIABILITY, INCLUDING LIABILITY FOR VIOLATION OF COPYRIGHT LAWS, UNLESS WRITTEN CONSENT IS PROVIDED BY FELDMAN GEOSPATIAL.

REFERENCES

MIDDLESEX COUNTY REGISTRY OF DEEDS
PLAN NO. 1796 OF 1947
PLAN NO. 1736 OF 1951
PLAN NO. 209 OF 1959
PLAN NO. 1499 OF 1964
PLAN NO. 1351 OF 1965
PLAN NO. 69 OF 1971
PLAN NO. 48 OF 1993
MASSACHUSETTS LAND COURT
LOC 26423-D
MASSACHUSETTS HIGHWAY DEPARTMENT
SHLO NO. 3050

LEGEND

- SEWER MANHOLE
- DRAIN MANHOLE
- HYDRANT
- WATER SHUT OFF/WATER GATE
- GAS SHUT OFF/GAS GATE
- CATCH BASIN
- TRAFFIC SIGNAL
- UTILITY POLE
- MAIL BOX
- SIGN
- OBSERVATION WELL
- WETLANDS
- BORDERING VEGETATED WETLANDS
- CALCULATED
- NOT TO SCALE
- RECORD
- VERTICAL GRANITE CURB
- WETLAND FLAG NUMBER
- OVERHEAD WIRES
- COMMUNICATIONS SERVICE
- DRAIN
- GAS
- SEWER
- WATER
- POLYVINYL CHLORIDE
- REINFORCED CONCRETE PIPE
- STONE WALL

ATTACHMENT C – Wildlife Habitat Evaluation

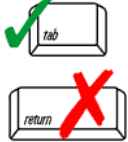


Wildlife Habitat Protection Guidance

Appendix A: Simplified Wildlife Habitat Evaluation

Project Information

Important:
When filling out forms on the computer, use only the tab key to move your cursor - do not use the return key.



"0" Cambridge Street, Burlington, Massachusetts

Project Location (from NOI)

Ryan Clapp, TRC Environmental Corporation

Name of Person Completing Form

1/20/2026

Date

Important Habitat Features

Direct alterations to the following important habitat features in resource areas may be permitted only if they will have no adverse effect (refer to Section V).

- ☐ Habitat for state-listed animal species (receipt of a positive opinion or permit from MNHESP shall be presumed to be correct. Do not refer to Section V).
- ☐ Sphagnum hummocks and pools suitable to serve as nesting habitat for four-toed salamanders
- ☐ Trees with large cavities (≥ 18 " tree diameter at cavity entrance)
- ☐ Existing beaver, mink or otter dens
- ☐ Areas within 100 feet of existing beaver, mink or otter dens (if significant disturbance)
- ☐ Existing nest trees for birds that traditionally reuse nests (bald eagle, osprey, great blue heron)
- ☐ Land containing freshwater mussel beds
- ☐ Wetlands and waterbodies known to contain open water in winter with the capacity to serve as waterfowl winter habitat
- ☐ Turtle nesting areas
- ☐ Vertical sandy banks (bank swallows, rough-winged swallows or kingfishers)

The following habitat characteristics when not commonly encountered in the surrounding area:

- ☐ Stream bed riffle zones (e.g. in eastern MA)
- ☐ Springs
- ☐ Gravel stream bottoms (trout and salmon nesting substrate)
- ☐ Plunge pools (deep holes) in rivers or streams
- ☐ Medium to large, flat rock substrates in streams

None of the above Important Habitat Features are present at the Project Site.



Wildlife Habitat Protection Guidance

Appendix A: Simplified Wildlife Habitat Evaluation

Activities

When any one of the following activities is proposed within resource areas, applicants should complete a Detailed Wildlife Habitat Evaluation (refer to Appendix B).

- ☐ Activities located in mapped “Habitat of Potential Regional or Statewide Importance”
- ☐ Activities affecting certified or documented vernal pool habitat, including habitat within 100’ of a certified or documented vernal pool when within a resource area
- ☐ Activities in bank, land under water, bordering land subject to flooding (presumed significant) where alterations are more than twice the size of thresholds
- ☐ Activities affecting vegetated wetlands >5000 sq. ft. occurring in resource areas other than Bordering Vegetated Wetland
- ☐ Activities affecting the sole connector between habitats >50 acres in size
- ☐ Installation of structures that prevent animal movement
- ☐ Activities for the purpose of bank stabilization using hard structure solutions that significantly affect ability of stream channel to shift and meander, or disrupt continuity in cover that would inhibit animal passage
- ☐ Dredging (greater than 5,000 sf)

ATTACHMENT D – Stormwater Report

DRAINAGE REPORT

For

FOXBOROUGH LEARNING, LLC

PROPOSED

“CHILD CARE CENTER”

***Cambridge Street
Burlington, Massachusetts
Middlesex County***

Prepared by:

BOHLER ENGINEERING
50 Washington Street
Westborough, MA 01581
(508) 480-9900 TEL.



John A. Kucich
Massachusetts P.E. Lic. #41530

BOHLER //

December 16, 2025
#MAA250027

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- NCRS CUSTOM SOIL RESOURCE REPORT
REPORT OF GEOTECHNICAL INVESTIGATION
-

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- EXISTING CONDITIONS DRAINAGE MAP
- EXISTING CONDITIONS HYDROCAD COMPUTATIONS

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- PROPOSED CONDITIONS DRAINAGE MAP
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- MA STANDARD #3 – RECHARGE AND DRAWDOWN TIME
- MA STANDARD #4 – WATER QUALITY AND TSS REMOVAL
- NOAA RAINFALL DATA
- PIPE AND INLET SIZING
- PHOSPHORUS REMOVAL CALCULATIONS

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- STORMWATER OPERATION AND MAINTENANCE PLAN
- INSPECTION REPORT
- INSPECTION AND MAINTENANCE LOG FORM
- LONG-TERM POLLUTION PREVENTION PLAN
- ILLICIT DISCHARGE STATEMENT
- SPILL PREVENTION
- PROPOSED BMP MAP
- ISOLATOR ROW O&M

I. EXECUTIVE SUMMARY

This report examines the changes in drainage that can be expected as the result of the development of a proposed child care center located on the westerly side of Cambridge Street in the Town of Burlington, Massachusetts. The site, which contains approximately 3.82 acres of land, is currently undeveloped and contains existing wooded area.

The proposed project includes the construction of a new 2-story, 11,000 sf freestanding child care center along with new paved parking areas, landscaping, storm water management components and associated utilities. This report addresses a comparative analysis of the pre- and post-development site runoff conditions. Additionally, this report provides calculations documenting the design of the proposed stormwater conveyance/management system as illustrated within the accompanying Site Development Plans prepared by Bohler. The project will also provide erosion and sedimentation controls during the demolition and construction periods, as well as long term stabilization of the site.

For the purposes of this analysis the pre- and post-development drainage conditions were analyzed at one (1) “design point” where stormwater runoff currently drains to under existing conditions. These design points are described in further detail in **Section II** below. A summary of the existing and proposed conditions peak runoff rates for the 2-, 10-, 25-, and 100-year storms can be found in **Table 1.1** below. In addition, the project has been designed to meet or exceed the Stormwater Management Standards as detailed herein.

Table 1.1: Design Point Peak Runoff Rate Summary

Point of Analysis	2-Year Storm			10-Year Storm			25-Year Storm			100-Year Storm		
	Pre	Post	Δ	Pre	Post	Δ	Pre	Post	Δ	Pre	Post	Δ
DP1	2.07	1.59	-0.48	5.13	5.05	-0.08	7.78	7.48	-0.30	12.38	11.19	-1.19

**Flows are represented in cubic feet per second (cfs)*

II. EXISTING SITE CONDITIONS

Existing Site Description

The site consists of approximately 3.82 acres of land located along the westerly side of Cambridge Street in the Town of Burlington, Massachusetts. The entirety of the site consists of undeveloped wooded area with wetlands large portion of the wooded area.

On-Site Soil Information

The majority of the soils at the site are mapped as Montauk fine sandy loam which is classified by the Natural Resource Conservation Service (NRCS) as Hydrologic Soil Group (HSG) “C”. There is a small portion of the westerly and easterly side of the site that is mapped as Udorthents – urban land complex with an undetermined HSG. Based upon on-site geotechnical testing performed in March 2025, the site has been analyzed as HSG “C” for the purposes of this analysis. Refer to **Appendix C** for additional information.

Existing Collection and Conveyance

The entirety of the site drains from the eastern portion along Cambridge street to west and discharges into the existing wetlands on site. Slopes on site range from 1%-50% with on-site elevations ranging from 217 along Cambridge Street to 182 near westerly the discharge to the wetlands.

Existing Watersheds and Design Point Information

The site was subdivided into one (1) sub catchment for the existing conditions as described below to analyze existing and proposed flow rates at each design point. The minimum time of concentration for all proposed areas is calculated as 6 minutes (0.1 hr).

Subcatchment EX-1 in total is 1.74 acres with existing woods to remain. This area flows overland from east to west across the site where it then discharges to an existing wetland. The stormwater runoff from this Subcatchment is not treated or attenuated.

The pre- and post-development drainage conditions for the site were then analyzed at one (1) “design points” where stormwater runoff currently drains to under existing conditions.

Design Point #1 (DP1) is the existing wetlands on site. Under existing conditions, this design point receives stormwater flows from approximately 1.74 acres of land, designated as watershed “EX-1”. This watershed includes areas of woodland. This area has a calculated curve number of 70 and a calculated time of concentration of 16.9 minutes.

Refer to **Table 1.1, 1.2, 5.1, and 5.2** for the calculated existing conditions peak rates of runoff and volumes. For additional hydrologic information, refer to **Appendix D** and the Drainage Area Maps in the appendices of this report for a graphical representation of the existing drainage areas.

III. PROPOSED SITE CONDITIONS

Proposed Development Description

The proposed project consists of construction of a new 2-story, 11,000 sf freestanding child care center including paved parking areas, landscaping, associated utilities, and a new stormwater management system. The site, including the proposed parking areas, has been designed to drain to deep-sump, hooded catch basins. The catch basins will capture and convey stormwater runoff, via an underground pipe system, to a proposed underground infiltration system. Pretreatment of stormwater runoff will be provided by a combination of the deep-sump, hooded catch basins and a water quality unit prior to discharge into the proposed infiltration system. Rooftop runoff and runoff from the proposed play area has been designed to flow to the underground system as well.

Proposed Development Collection and Conveyance

Deep sump hooded catch basins are proposed to collect and route runoff from the paved parking areas to the proposed underground infiltration system. Pipes have been designed for the 25-year storm using Rational Method. Pipe, inlet, and outlet protection sizing calculations are included in **Appendix F**.

The best management practices (BMPs) incorporated into the proposed stormwater management system have been designed to meet the total suspended solid (TSS) removal requirements as set forth in the Massachusetts Department of Environmental Protection Stormwater Handbook standards. Refer to **Appendix F** for calculations. In addition, a Stormwater Operation and

Maintenance (O&M) Plan, attached in **Appendix G**, has been developed which includes scheduled maintenance and periodic inspections of stormwater management structures [i.e catch basins and infiltration basins].

Proposed Watersheds and Design Point Information

The project has been designed to maintain existing drainage watersheds to the greatest extent possible, with the same design points described in **Section II** above. The site was subdivided into three (3) separate sub catchments for the proposed conditions as described below. The minimum time of concentration for all proposed areas is calculated as 6 minutes (0.1 hr).

Subcatchment PR-1 consists of 0.13 acres of entirely area consisting of rooftop. This area drains to a series of proposed roof drains that discharge into a proposed underground infiltration system. The model used the minimum time of concentration of 6 minutes.

Subcatchment PR-2 consists of 0.55 acres of entirely area consisting of grass and paved parking. This area drains to a series of proposed catch basins that is then piped through a water quality unit for pretreatment prior to discharge into a proposed underground infiltration system. The model used the minimum time of concentration of 6 minutes.

Subcatchment PR-3 consists of 1.06 acres of entirely area consisting of grass and woodland. This area drains overland from east to west where it then discharges into the existing wetlands. The model calculated a time of concentration of 16.3 minutes.

Refer to **Table 1.1 and 6.1** for the calculated proposed conditions peak rates of runoff. For additional hydrologic information, refer to **Appendix D** and the Drainage Area Maps in the appendices of this report for a graphical representation of the proposed drainage areas.

IV. METHODOLOGY

Peak Flow Calculations

Methodology utilized to design the proposed stormwater management system includes compliance with the guidelines set forth in the latest edition of the Massachusetts DEP Stormwater Handbook. The pre- and post-development runoff rates being discharged from the site were computed using the HydroCAD computer program. The drainage area and outlet information were entered into

the program, which routes storm flows based on NRCS TR-20 and TR-55 methods. The other components of the model were determined following standard NRCS procedures for Curve Numbers (CNs) and times of concentrations documented in the appendices of this report. The rainfall data utilized and listed below in table 4.1 below for stormwater calculations is based on NOAA. Refer to **Appendix F** for more information.

Table 4.1: NOAA Rainfall Intensities

Frequency	2 year	10 year	25 year	100 year
Rainfall* (inches)	4.00	6.37	8.25	11.4

*Values derived from NOAA ATLAS on 10/27/2025

The proposed stormwater management as designed will provide a decrease in peak rates of runoff from the proposed facility for the 2-, 10-, 25- and 100-year design storm events. Additionally, the proposed project meets, or exceeds, the MADEP Stormwater Management standards. Compliance with these standards is described further below.

V. STORMWATER MANAGEMENT STANDARDS

Standard #1: No New Untreated Discharges

The project has been designed so that proposed impervious areas (including the building roof and paved parking/driveway areas) shall be collected and passed through the proposed drainage system for treatment prior to discharge.

Standard #2: Peak Rate Attenuation

As outlined in **Table 1.1** and **Table 5.1**, the development of the site and the proposed stormwater management system, have been designed so that post-development peak rates of runoff are below pre-development conditions for the 2-, 10-, 25- and 100-year storm events at all design points.

Standard #3: Recharge

The stormwater runoff from the project will be collected and diverted to a proposed underground infiltration system. The project as proposed will involve the creation of 27,311 square feet of new impervious area and is required to infiltrate 569 cubic feet of stormwater as defined in Stormwater Standard 3. The proposed infiltration basin will provide 4,177 cubic feet of volume below the lowest outlet for groundwater recharge. Refer to **Appendix F** of this report for calculations documenting required and provided recharge volumes.

The DEP Stormwater Standards require that the infiltration BMP drains completely within 72 hours of the end of the storm event. Calculations showing that the proposed infiltration basin will drain within 70.8 hours are included in **Appendix F** of this report.

A four (4) foot separation to estimated seasonal high groundwater is provided and a groundwater mounding analysis is not required.

Standard #4: Water Quality

Water quality treatment is provided via deep sump catch basins, a water quality unit, and an infiltration basin. TSS removal calculations are included in **Appendix F** of this report. Phosphorus removal calculations have also been included as Burlington requires 60% of the average annual load of Total Phosphorus removal. The project as proposed will involve the creation of 27,311 square feet of new impervious area and is required to treat 2,276 cubic feet of water quality volume

as defined in Stormwater Standard 4. The proposed infiltration basin provides 4,177 cubic feet of water quality volume below the lowest outlet for water quality treatment. Refer to **Appendix F** of this report for calculations documenting required and provided water quality volumes.

Standard #5: Land Use with Higher Potential Pollutant Loads

Not Applicable for this project.

Standard #6: Critical Areas

Not Applicable for this project..

Standard #7: Redevelopment

Not Applicable for this project.

Standard #8: Construction Period Pollution Prevention and Erosion and Sedimentation Control

The proposed project will provide construction period erosion and sedimentation controls as indicated within the site plan set provided for this project. This includes a proposed construction exit, protection for stormwater inlets, protection around temporary material stock piles and various other techniques as outlined on the erosion and sediment control sheets. Additionally, the project is required to file a Notice of Intent with the US EPA and implement a Stormwater Pollution Prevention Plan (SWPPP) during the construction period. The SWPPP will be prepared prior to the start of construction and will be implemented by the site contractor under the guidance and responsibility of the project's proponent.

Standard #9: Operation and Maintenance Plan (O&M Plan)

An Operation and Maintenance (O&M) Plan for this site has been prepared and is included in **Appendix G** of this report. The O&M Plan outlines procedures and time tables for the long term operation and maintenance of the proposed site stormwater management system, including initial inspections upon completion of construction, and periodic monitoring of the system components, in accordance with established practices and the manufacturer's recommendations. The O&M Plan includes a list of responsible parties and an estimated budget for inspections and maintenance.

Standard #10: Prohibition of Illicit Discharges

The proposed stormwater system will only convey allowable non-stormwater discharges (firefighting waters, irrigation, air conditioning condensates, etc.) and will not contain any illicit discharges from prohibited sources. An Illicit Discharge Statement is included in **Appendix G** of this report.

VI. SUMMARY

In summary, the proposed stormwater management system illustrated on the drawings prepared by Bohler Engineering results in a reduction in peak rates of runoff from the subject site when compared to pre-development conditions for the 2-, 10-, 25- and 100-year storm frequencies. In addition, the proposed best management practices will result in an effective removal of total suspended solids from the post-development runoff. The pre-development versus post-development stormwater discharge comparisons are contained in **Table 6.1** below:

Table 6.1: Design Point Peak Runoff Rate Summary

Point of Analysis	2-Year Storm			10-Year Storm			25-Year Storm			100-Year Storm		
	Pre	Post	Δ	Pre	Post	Δ	Pre	Post	Δ	Pre	Post	Δ
DP1	2.07	1.59	-0.48	5.13	5.05	-0.08	7.78	7.48	-0.30	12.38	11.19	-1.19

**Flows are represented in cubic feet per second (cfs)*

As outlined in the table above, the proposed stormwater management system as designed will provide a decrease in peak rates of runoff from the proposed facility for the 2-, 10-, 25- and 100-year storm events. Additionally, the project meets, or exceeds the MADEP Stormwater Management Standards as described further herein.

APPENDIX A: MASSACHUSETTS STORMWATER MANAGEMENT CHECKLIST



Checklist for Stormwater Report

A. Introduction

Important: When filling out forms on the computer, use only the tab key to move your cursor - do not use the return key.



A Stormwater Report must be submitted with the Notice of Intent permit application to document compliance with the Stormwater Management Standards. The following checklist is NOT a substitute for the Stormwater Report (which should provide more substantive and detailed information) but is offered here as a tool to help the applicant organize their Stormwater Management documentation for their Report and for the reviewer to assess this information in a consistent format. As noted in the Checklist, the Stormwater Report must contain the engineering computations and supporting information set forth in Volume 3 of the [Massachusetts Stormwater Handbook](#). The Stormwater Report must be prepared and certified by a Registered Professional Engineer (RPE) licensed in the Commonwealth.

The Stormwater Report must include:

- The Stormwater Checklist completed and stamped by a Registered Professional Engineer (see page 2) that certifies that the Stormwater Report contains all required submittals.¹ This Checklist is to be used as the cover for the completed Stormwater Report.
- Applicant/Project Name
- Project Address
- Name of Firm and Registered Professional Engineer that prepared the Report
- Long-Term Pollution Prevention Plan required by Standards 4-6
- Construction Period Pollution Prevention and Erosion and Sedimentation Control Plan required by Standard 8²
- Operation and Maintenance Plan required by Standard 9

In addition to all plans and supporting information, the Stormwater Report must include a brief narrative describing stormwater management practices, including environmentally sensitive site design and LID techniques, along with a diagram depicting runoff through the proposed BMP treatment train. Plans are required to show existing and proposed conditions, identify all wetland resource areas, NRCS soil types, critical areas, Land Uses with Higher Potential Pollutant Loads (LUHPPL), and any areas on the site where infiltration rate is greater than 2.4 inches per hour. The Plans shall identify the drainage areas for both existing and proposed conditions at a scale that enables verification of supporting calculations.

As noted in the Checklist, the Stormwater Management Report shall document compliance with each of the Stormwater Management Standards as provided in the Massachusetts Stormwater Handbook. The soils evaluation and calculations shall be done using the methodologies set forth in Volume 3 of the Massachusetts Stormwater Handbook.

To ensure that the Stormwater Report is complete, applicants are required to fill in the Stormwater Report Checklist by checking the box to indicate that the specified information has been included in the Stormwater Report. If any of the information specified in the checklist has not been submitted, the applicant must provide an explanation. The completed Stormwater Report Checklist and Certification must be submitted with the Stormwater Report.

¹ The Stormwater Report may also include the Illicit Discharge Compliance Statement required by Standard 10. If not included in the Stormwater Report, the Illicit Discharge Compliance Statement must be submitted prior to the discharge of stormwater runoff to the post-construction best management practices.

² For some complex projects, it may not be possible to include the Construction Period Erosion and Sedimentation Control Plan in the Stormwater Report. In that event, the issuing authority has the discretion to issue an Order of Conditions that approves the project and includes a condition requiring the proponent to submit the Construction Period Erosion and Sedimentation Control Plan before commencing any land disturbance activity on the site.



Checklist for Stormwater Report

B. Stormwater Checklist and Certification

The following checklist is intended to serve as a guide for applicants as to the elements that ordinarily need to be addressed in a complete Stormwater Report. The checklist is also intended to provide conservation commissions and other reviewing authorities with a summary of the components necessary for a comprehensive Stormwater Report that addresses the ten Stormwater Standards.

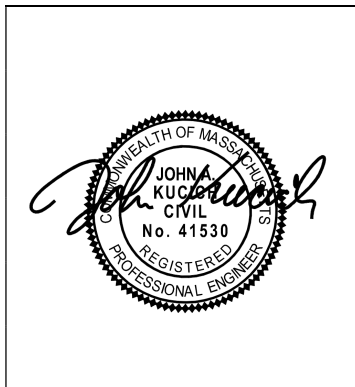
Note: Because stormwater requirements vary from project to project, it is possible that a complete Stormwater Report may not include information on some of the subjects specified in the Checklist. If it is determined that a specific item does not apply to the project under review, please note that the item is not applicable (N.A.) and provide the reasons for that determination.

A complete checklist must include the Certification set forth below signed by the Registered Professional Engineer who prepared the Stormwater Report.

Registered Professional Engineer's Certification

I have reviewed the Stormwater Report, including the soil evaluation, computations, Long-term Pollution Prevention Plan, the Construction Period Erosion and Sedimentation Control Plan (if included), the Long-term Post-Construction Operation and Maintenance Plan, the Illicit Discharge Compliance Statement (if included) and the plans showing the stormwater management system, and have determined that they have been prepared in accordance with the requirements of the Stormwater Management Standards as further elaborated by the Massachusetts Stormwater Handbook. I have also determined that the information presented in the Stormwater Checklist is accurate and that the information presented in the Stormwater Report accurately reflects conditions at the site as of the date of this permit application.

Registered Professional Engineer Block and Signature



12/16/2025

Signature and Date

Checklist

Project Type: Is the application for new development, redevelopment, or a mix of new and redevelopment?

- ☒ New development
- ☐ Redevelopment
- ☐ Mix of New Development and Redevelopment



Checklist for Stormwater Report

Checklist (continued)

LID Measures: Stormwater Standards require LID measures to be considered. Document what environmentally sensitive design and LID Techniques were considered during the planning and design of the project:

- ☐ No disturbance to any Wetland Resource Areas
- ☐ Site Design Practices (e.g. clustered development, reduced frontage setbacks)
- ☐ Reduced Impervious Area (Redevelopment Only)
- ☐ Minimizing disturbance to existing trees and shrubs
- ☐ LID Site Design Credit Requested:
 - ☐ Credit 1
 - ☐ Credit 2
 - ☐ Credit 3
- ☐ Use of “country drainage” versus curb and gutter conveyance and pipe
- ☐ Bioretention Cells (includes Rain Gardens)
- ☐ Constructed Stormwater Wetlands (includes Gravel Wetlands designs)
- ☐ Treebox Filter
- ☐ Water Quality Swale
- ☐ Grass Channel
- ☐ Green Roof
- ☐ Other (describe): Underground Infiltration System

Standard 1: No New Untreated Discharges

- ☒ No new untreated discharges
- ☒ Outlets have been designed so there is no erosion or scour to wetlands and waters of the Commonwealth
- ☒ Supporting calculations specified in Volume 3 of the Massachusetts Stormwater Handbook included.



Checklist for Stormwater Report

Checklist (continued)

Standard 2: Peak Rate Attenuation

- ☐ Standard 2 waiver requested because the project is located in land subject to coastal storm flowage and stormwater discharge is to a wetland subject to coastal flooding.
- ☐ Evaluation provided to determine whether off-site flooding increases during the 100-year 24-hour storm.
- ☒ Calculations provided to show that post-development peak discharge rates do not exceed pre-development rates for the 2-year and 10-year 24-hour storms. If evaluation shows that off-site flooding increases during the 100-year 24-hour storm, calculations are also provided to show that post-development peak discharge rates do not exceed pre-development rates for the 100-year 24-hour storm.

Standard 3: Recharge

- ☒ Soil Analysis provided.
- ☒ Required Recharge Volume calculation provided.
- ☐ Required Recharge volume reduced through use of the LID site Design Credits.
- ☒ Sizing the infiltration, BMPs is based on the following method: Check the method used.
 - ☐ Static
 - ☒ Simple Dynamic
 - ☐ Dynamic Field¹
- ☒ Runoff from all impervious areas at the site discharging to the infiltration BMP.
- ☐ Runoff from all impervious areas at the site is *not* discharging to the infiltration BMP and calculations are provided showing that the drainage area contributing runoff to the infiltration BMPs is sufficient to generate the required recharge volume.
- ☒ Recharge BMPs have been sized to infiltrate the Required Recharge Volume.
- ☐ Recharge BMPs have been sized to infiltrate the Required Recharge Volume *only* to the maximum extent practicable for the following reason:
 - ☐ Site is comprised solely of C and D soils and/or bedrock at the land surface
 - ☐ M.G.L. c. 21E sites pursuant to 310 CMR 40.0000
 - ☐ Solid Waste Landfill pursuant to 310 CMR 19.000
 - ☐ Project is otherwise subject to Stormwater Management Standards only to the maximum extent practicable.
- ☒ Calculations showing that the infiltration BMPs will drain in 72 hours are provided.
- ☐ Property includes a M.G.L. c. 21E site or a solid waste landfill and a mounding analysis is included.

¹ 80% TSS removal is required prior to discharge to infiltration BMP if Dynamic Field method is used.



Checklist for Stormwater Report

Checklist (continued)

Standard 3: Recharge (continued)

- ☐ The infiltration BMP is used to attenuate peak flows during storms greater than or equal to the 10-year 24-hour storm and separation to seasonal high groundwater is less than 4 feet and a mounding analysis is provided.
- ☐ Documentation is provided showing that infiltration BMPs do not adversely impact nearby wetland resource areas.

Standard 4: Water Quality

The Long-Term Pollution Prevention Plan typically includes the following:

- Good housekeeping practices;
 - Provisions for storing materials and waste products inside or under cover;
 - Vehicle washing controls;
 - Requirements for routine inspections and maintenance of stormwater BMPs;
 - Spill prevention and response plans;
 - Provisions for maintenance of lawns, gardens, and other landscaped areas;
 - Requirements for storage and use of fertilizers, herbicides, and pesticides;
 - Pet waste management provisions;
 - Provisions for operation and management of septic systems;
 - Provisions for solid waste management;
 - Snow disposal and plowing plans relative to Wetland Resource Areas;
 - Winter Road Salt and/or Sand Use and Storage restrictions;
 - Street sweeping schedules;
 - Provisions for prevention of illicit discharges to the stormwater management system;
 - Documentation that Stormwater BMPs are designed to provide for shutdown and containment in the event of a spill or discharges to or near critical areas or from LUHPPL;
 - Training for staff or personnel involved with implementing Long-Term Pollution Prevention Plan;
 - List of Emergency contacts for implementing Long-Term Pollution Prevention Plan.
- ☒ A Long-Term Pollution Prevention Plan is attached to Stormwater Report and is included as an attachment to the Wetlands Notice of Intent.
 - ☒ Treatment BMPs subject to the 44% TSS removal pretreatment requirement and the one inch rule for calculating the water quality volume are included, and discharge:
 - ☐ is within the Zone II or Interim Wellhead Protection Area
 - ☐ is near or to other critical areas
 - ☐ is within soils with a rapid infiltration rate (greater than 2.4 inches per hour)
 - ☐ involves runoff from land uses with higher potential pollutant loads.
 - ☐ The Required Water Quality Volume is reduced through use of the LID site Design Credits.
 - ☒ Calculations documenting that the treatment train meets the 80% TSS removal requirement and, if applicable, the 44% TSS removal pretreatment requirement, are provided.



Checklist for Stormwater Report

Checklist (continued)

Standard 4: Water Quality (continued)

- ☒ The BMP is sized (and calculations provided) based on:
 - ☒ The ½" or 1" Water Quality Volume or
 - ☐ The equivalent flow rate associated with the Water Quality Volume and documentation is provided showing that the BMP treats the required water quality volume.
- ☒ The applicant proposes to use proprietary BMPs, and documentation supporting use of proprietary BMP and proposed TSS removal rate is provided. This documentation may be in the form of the propriety BMP checklist found in Volume 2, Chapter 4 of the Massachusetts Stormwater Handbook and submitting copies of the TARP Report, STEP Report, and/or other third party studies verifying performance of the proprietary BMPs.
- ☐ A TMDL exists that indicates a need to reduce pollutants other than TSS and documentation showing that the BMPs selected are consistent with the TMDL is provided.

Standard 5: Land Uses With Higher Potential Pollutant Loads (LUHPPLs)

- ☐ The NPDES Multi-Sector General Permit covers the land use and the Stormwater Pollution Prevention Plan (SWPPP) has been included with the Stormwater Report.
- ☒ The NPDES Multi-Sector General Permit covers the land use and the SWPPP will be submitted **prior to** the discharge of stormwater to the post-construction stormwater BMPs.
- ☐ The NPDES Multi-Sector General Permit does **not** cover the land use.
- ☐ LUHPPLs are located at the site and industry specific source control and pollution prevention measures have been proposed to reduce or eliminate the exposure of LUHPPLs to rain, snow, snow melt and runoff, and been included in the long term Pollution Prevention Plan.
- ☐ All exposure has been eliminated.
- ☐ All exposure has **not** been eliminated and all BMPs selected are on MassDEP LUHPPL list.
- ☐ The LUHPPL has the potential to generate runoff with moderate to higher concentrations of oil and grease (e.g. all parking lots with >1000 vehicle trips per day) and the treatment train includes an oil grit separator, a filtering bioretention area, a sand filter or equivalent.

Standard 6: Critical Areas

- ☐ The discharge is near or to a critical area and the treatment train includes only BMPs that MassDEP has approved for stormwater discharges to or near that particular class of critical area.
- ☐ Critical areas and BMPs are identified in the Stormwater Report.



Checklist for Stormwater Report

Checklist (continued)

Standard 7: Redevelopments and Other Projects Subject to the Standards only to the maximum extent practicable

- ☐ The project is subject to the Stormwater Management Standards only to the maximum Extent Practicable as a:
 - ☐ Limited Project
 - ☐ Small Residential Projects: 5-9 single family houses or 5-9 units in a multi-family development provided there is no discharge that may potentially affect a critical area.
 - ☐ Small Residential Projects: 2-4 single family houses or 2-4 units in a multi-family development with a discharge to a critical area
 - ☐ Marina and/or boatyard provided the hull painting, service and maintenance areas are protected from exposure to rain, snow, snow melt and runoff
 - ☐ Bike Path and/or Foot Path
 - ☐ Redevelopment Project
 - ☐ Redevelopment portion of mix of new and redevelopment.
- ☐ Certain standards are not fully met (Standard No. 1, 8, 9, and 10 must always be fully met) and an explanation of why these standards are not met is contained in the Stormwater Report.
- ☐ The project involves redevelopment and a description of all measures that have been taken to improve existing conditions is provided in the Stormwater Report. The redevelopment checklist found in Volume 2 Chapter 3 of the Massachusetts Stormwater Handbook may be used to document that the proposed stormwater management system (a) complies with Standards 2, 3 and the pretreatment and structural BMP requirements of Standards 4-6 to the maximum extent practicable and (b) improves existing conditions.

Standard 8: Construction Period Pollution Prevention and Erosion and Sedimentation Control

A Construction Period Pollution Prevention and Erosion and Sedimentation Control Plan must include the following information:

- Narrative;
 - Construction Period Operation and Maintenance Plan;
 - Names of Persons or Entity Responsible for Plan Compliance;
 - Construction Period Pollution Prevention Measures;
 - Erosion and Sedimentation Control Plan Drawings;
 - Detail drawings and specifications for erosion control BMPs, including sizing calculations;
 - Vegetation Planning;
 - Site Development Plan;
 - Construction Sequencing Plan;
 - Sequencing of Erosion and Sedimentation Controls;
 - Operation and Maintenance of Erosion and Sedimentation Controls;
 - Inspection Schedule;
 - Maintenance Schedule;
 - Inspection and Maintenance Log Form.
- ☒ A Construction Period Pollution Prevention and Erosion and Sedimentation Control Plan containing the information set forth above has been included in the Stormwater Report.



Checklist for Stormwater Report

Checklist (continued)

Standard 8: Construction Period Pollution Prevention and Erosion and Sedimentation Control (continued)

- ☐ The project is highly complex and information is included in the Stormwater Report that explains why it is not possible to submit the Construction Period Pollution Prevention and Erosion and Sedimentation Control Plan with the application. A Construction Period Pollution Prevention and Erosion and Sedimentation Control has **not** been included in the Stormwater Report but will be submitted **before** land disturbance begins.
- ☐ The project is **not** covered by a NPDES Construction General Permit.
- ☐ The project is covered by a NPDES Construction General Permit and a copy of the SWPPP is in the Stormwater Report.
- ☒ The project is covered by a NPDES Construction General Permit but no SWPPP been submitted. The SWPPP will be submitted BEFORE land disturbance begins.

Standard 9: Operation and Maintenance Plan

- ☒ The Post Construction Operation and Maintenance Plan is included in the Stormwater Report and includes the following information:
 - ☒ Name of the stormwater management system owners;
 - ☒ Party responsible for operation and maintenance;
 - ☒ Schedule for implementation of routine and non-routine maintenance tasks;
 - ☐ Plan showing the location of all stormwater BMPs maintenance access areas;
 - ☒ Description and delineation of public safety features;
 - ☒ Estimated operation and maintenance budget; and
 - ☒ Operation and Maintenance Log Form.
- ☐ The responsible party is **not** the owner of the parcel where the BMP is located and the Stormwater Report includes the following submissions:
 - ☐ A copy of the legal instrument (deed, homeowner's association, utility trust or other legal entity) that establishes the terms of and legal responsibility for the operation and maintenance of the project site stormwater BMPs;
 - ☐ A plan and easement deed that allows site access for the legal entity to operate and maintain BMP functions.

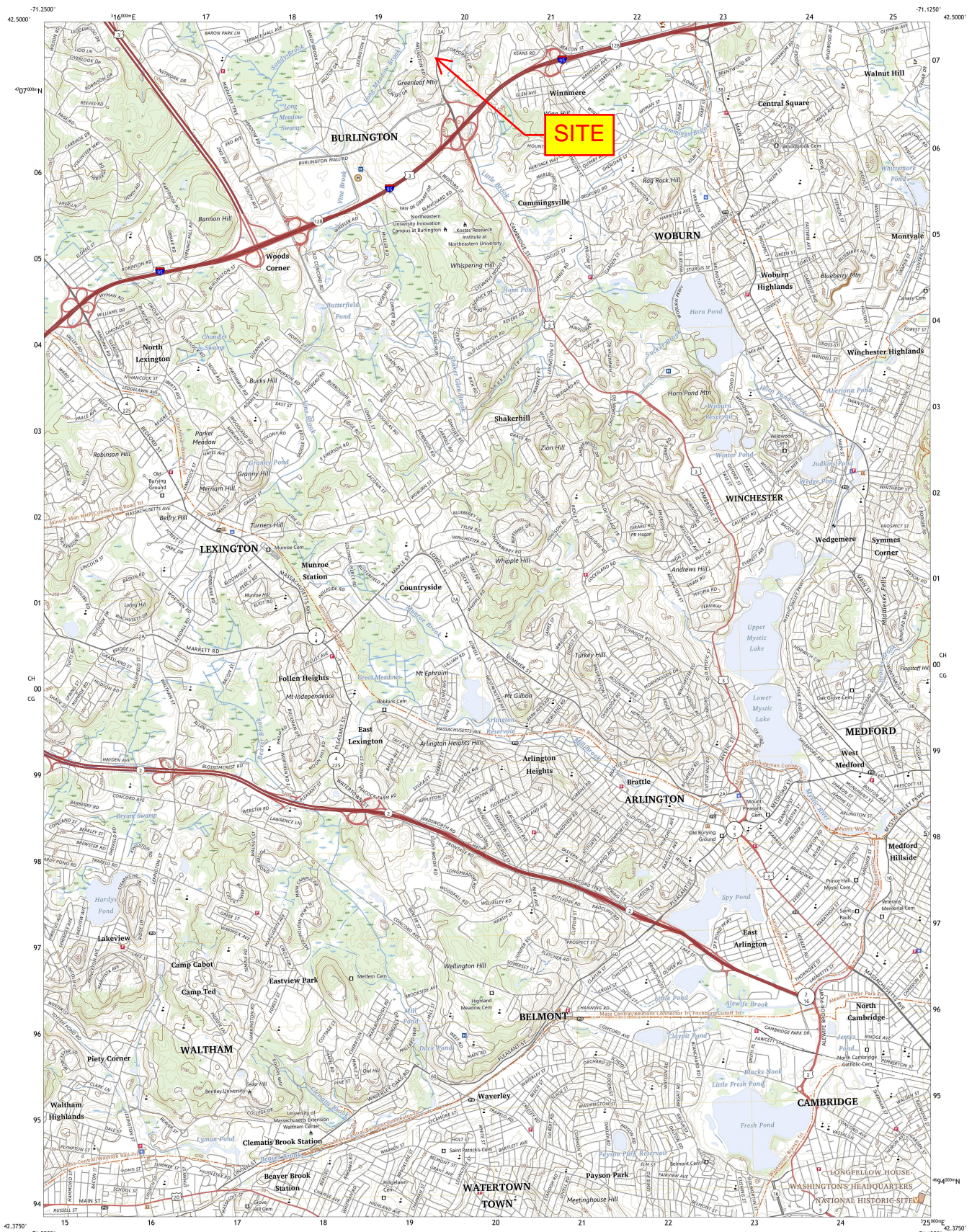
Standard 10: Prohibition of Illicit Discharges

- ☒ The Long-Term Pollution Prevention Plan includes measures to prevent illicit discharges;
- ☒ An Illicit Discharge Compliance Statement is attached;
- ☐ NO Illicit Discharge Compliance Statement is attached but will be submitted **prior to** the discharge of any stormwater to post-construction BMPs.

APPENDIX B: PROJECT LOCATION MAPS

➤ USGS MAP

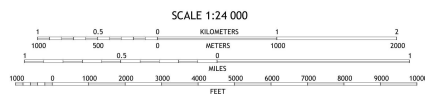
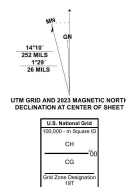
➤ FEMA FIRMETTE



Produced by the United States Geological Survey

North American Datum of 1983 (NAD83)
World Geodetic System of 1984 (WGS84) Projection and
1,000-meter grid Universal Transverse Mercator, Zone 18T
This map is not a legal document. Boundaries may be
generalized for this map scale. Private lands with government
reservations may not be shown. Obtain permission before
entering private lands.

Imagery: NAIP, October 2018 - October 2018
Roads: U.S. Census Bureau, 2018
Names: National Hydrography Dataset, 2004 - 2004
Contours: National Elevation Dataset, 2003
Boundaries: Multiple sources; see metadata file 2020 - 2022
Wetlands: FWS National Wetlands Inventory 1991 - 2013



CONTOUR INTERVAL 10 FEET
NORTH AMERICAN DATUM OF 1983
This map was produced to conform with the
National Geospatial Program US Topo Product Standard.



1	2	3
4	5	6
7	8	9

ADJOINING QUADRANGLES



LEXINGTON, MA
2024

National Flood Hazard Layer FIRMMette



71°12'1"W 42°30'8"N



1:6,000

71°11'24"W 42°29'42"N

Basemap Imagery Source: USGS National Map 2023

Legend

SEE FIS REPORT FOR DETAILED LEGEND AND INDEX MAP FOR FIRM PANEL LAYOUT

SPECIAL FLOOD HAZARD AREAS		Without Base Flood Elevation (BFE) Zone A, V, A99
		With BFE or Depth Zone AE, AO, AH, VE, AR
		Regulatory Floodway
OTHER AREAS OF FLOOD HAZARD		0.2% Annual Chance Flood Hazard, Areas of 1% annual chance flood with average depth less than one foot or with drainage areas of less than one square mile Zone X
		Future Conditions 1% Annual Chance Flood Hazard Zone X
		Area with Reduced Flood Risk due to Levee. See Notes. Zone X
		Area with Flood Risk due to Levee Zone D
OTHER AREAS		NO SCREEN Area of Minimal Flood Hazard Zone X
		Effective LOMRs
GENERAL STRUCTURES		Area of Undetermined Flood Hazard Zone D
		Channel, Culvert, or Storm Sewer
		Levee, Dike, or Floodwall
OTHER FEATURES		20.2 Cross Sections with 1% Annual Chance Water Surface Elevation
		17.5 Cross Sections with 1% Annual Chance Water Surface Elevation
		Coastal Transect
		Base Flood Elevation Line (BFE)
		Limit of Study
		Jurisdiction Boundary
		Coastal Transect Baseline
		Profile Baseline
		Hydrographic Feature
MAP PANELS		Digital Data Available
		No Digital Data Available
		Unmapped



The pin displayed on the map is an approximate point selected by the user and does not represent an authoritative property location.

This map complies with FEMA's standards for the use of digital flood maps if it is not void as described below. The basemap shown complies with FEMA's basemap accuracy standards

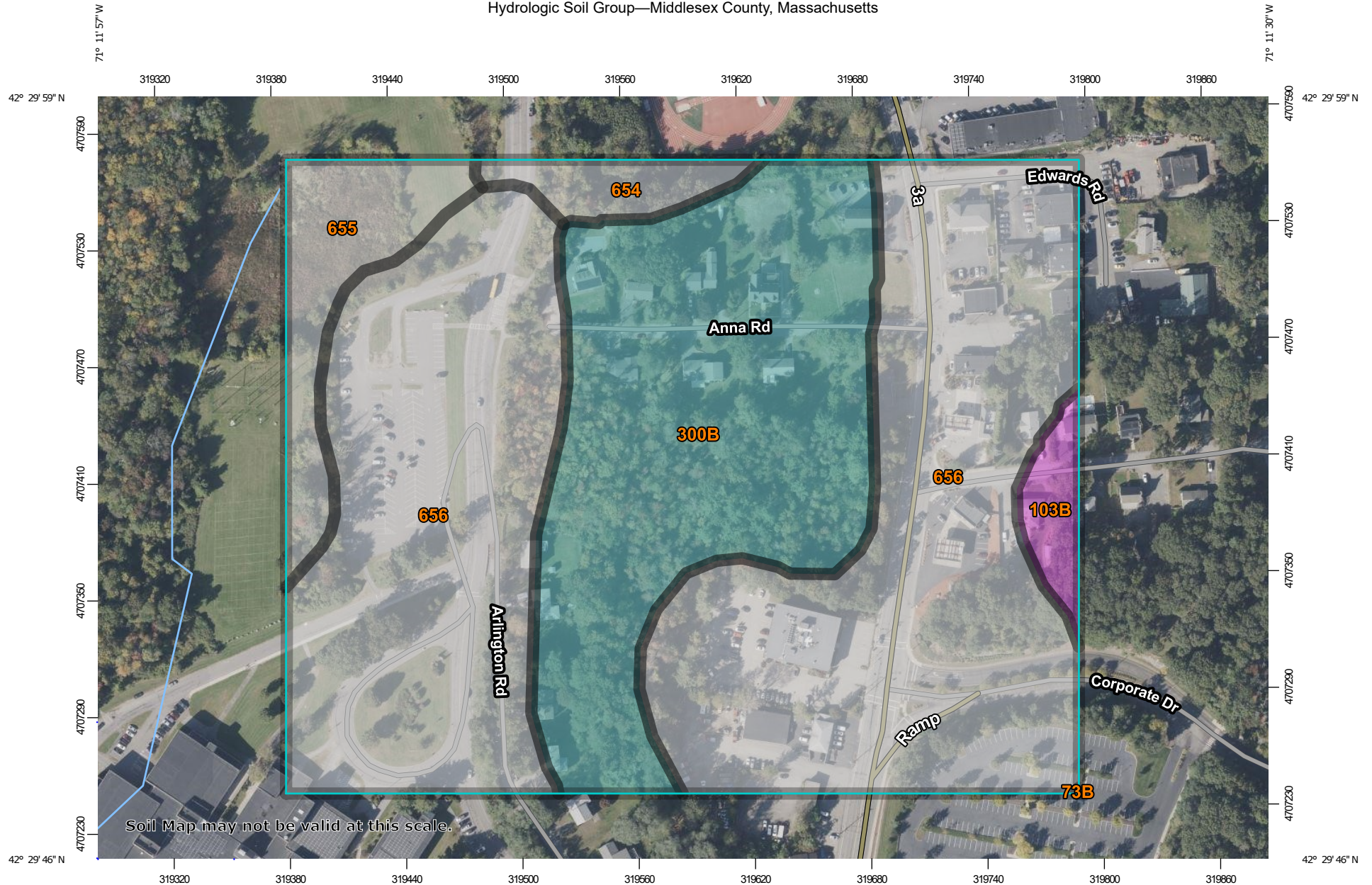
The flood hazard information is derived directly from the authoritative NFHL web services provided by FEMA. This map was exported on **12/16/2025 at 3:53 PM** and does not reflect changes or amendments subsequent to this date and time. The NFHL and effective information may change or become superseded by new data over time.

This map image is void if the one or more of the following map elements do not appear: basemap imagery, flood zone labels, legend, scale bar, map creation date, community identifiers, FIRM panel number, and FIRM effective date. Map images for unmapped and unmodernized areas cannot be used for regulatory purposes.

APPENDIX C: SOIL AND WETLAND INFORMATION

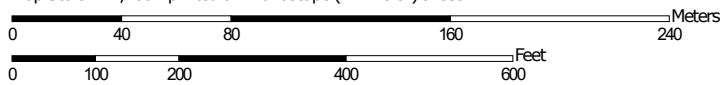
- NCRS CUSTOM SOIL RESOURCE REPORT
- REPORT OF GEOTECHNICAL INVESTIGATION

Hydrologic Soil Group—Middlesex County, Massachusetts



Soil Map may not be valid at this scale.

Map Scale: 1:2,760 if printed on A landscape (11" x 8.5") sheet.



Map projection: Web Mercator Corner coordinates: WGS84 Edge tics: UTM Zone 19N WGS84



**Natural Resources
Conservation Service**

Web Soil Survey
National Cooperative Soil Survey

10/27/2025
Page 1 of 4

MAP LEGEND

Area of Interest (AOI)









 Area of Interest (AOI)

Soils

Soil Rating Polygons





 A
 A/D
 B
 B/D
 C
 C/D
 D
 Not rated or not available

Soil Rating Lines


 A
 A/D
 B
 B/D
 C
 C/D
 D
 Not rated or not available

Soil Rating Points

 A
 A/D
 B
 B/D

 C
 C/D
 D
 Not rated or not available


Water Features

 Streams and Canals

Transportation

 Rails
 Interstate Highways
 US Routes
 Major Roads
 Local Roads

Background

 Aerial Photography

MAP INFORMATION

The soil surveys that comprise your AOI were mapped at 1:24,000.

Warning: Soil Map may not be valid at this scale.

Enlargement of maps beyond the scale of mapping can cause misunderstanding of the detail of mapping and accuracy of soil line placement. The maps do not show the small areas of contrasting soils that could have been shown at a more detailed scale.

Please rely on the bar scale on each map sheet for map measurements.

Source of Map: Natural Resources Conservation Service
 Web Soil Survey URL:
 Coordinate System: Web Mercator (EPSG:3857)

Maps from the Web Soil Survey are based on the Web Mercator projection, which preserves direction and shape but distorts distance and area. A projection that preserves area, such as the Albers equal-area conic projection, should be used if more accurate calculations of distance or area are required.

This product is generated from the USDA-NRCS certified data as of the version date(s) listed below.

Soil Survey Area: Middlesex County, Massachusetts
 Survey Area Data: Version 25, Sep 5, 2025

Soil map units are labeled (as space allows) for map scales 1:50,000 or larger.

Date(s) aerial images were photographed: Mar 1, 2023—Sep 1, 2023

The orthophoto or other base map on which the soil lines were compiled and digitized probably differs from the background imagery displayed on these maps. As a result, some minor shifting of map unit boundaries may be evident.

Hydrologic Soil Group

Map unit symbol	Map unit name	Rating	Acres in AOI	Percent of AOI
73B	Whitman fine sandy loam, 0 to 3 percent slopes, extremely stony	D	0.0	0.0%
103B	Charlton-Hollis-Rock outcrop complex, 3 to 8 percent slopes	A	0.6	1.9%
300B	Montauk fine sandy loam, 3 to 8 percent slopes	C	9.6	29.1%
654	Udorthents, loamy		0.8	2.5%
655	Udorthents, wet substratum		2.0	6.0%
656	Udorthents-Urban land complex		20.0	60.5%
Totals for Area of Interest			33.1	100.0%

Description

Hydrologic soil groups are based on estimates of runoff potential. Soils are assigned to one of four groups according to the rate of water infiltration when the soils are not protected by vegetation, are thoroughly wet, and receive precipitation from long-duration storms.

The soils in the United States are assigned to four groups (A, B, C, and D) and three dual classes (A/D, B/D, and C/D). The groups are defined as follows:

Group A. Soils having a high infiltration rate (low runoff potential) when thoroughly wet. These consist mainly of deep, well drained to excessively drained sands or gravelly sands. These soils have a high rate of water transmission.

Group B. Soils having a moderate infiltration rate when thoroughly wet. These consist chiefly of moderately deep or deep, moderately well drained or well drained soils that have moderately fine texture to moderately coarse texture. These soils have a moderate rate of water transmission.

Group C. Soils having a slow infiltration rate when thoroughly wet. These consist chiefly of soils having a layer that impedes the downward movement of water or soils of moderately fine texture or fine texture. These soils have a slow rate of water transmission.

Group D. Soils having a very slow infiltration rate (high runoff potential) when thoroughly wet. These consist chiefly of clays that have a high shrink-swell potential, soils that have a high water table, soils that have a claypan or clay layer at or near the surface, and soils that are shallow over nearly impervious material. These soils have a very slow rate of water transmission.

If a soil is assigned to a dual hydrologic group (A/D, B/D, or C/D), the first letter is for drained areas and the second is for undrained areas. Only the soils that in their natural condition are in group D are assigned to dual classes.

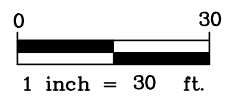
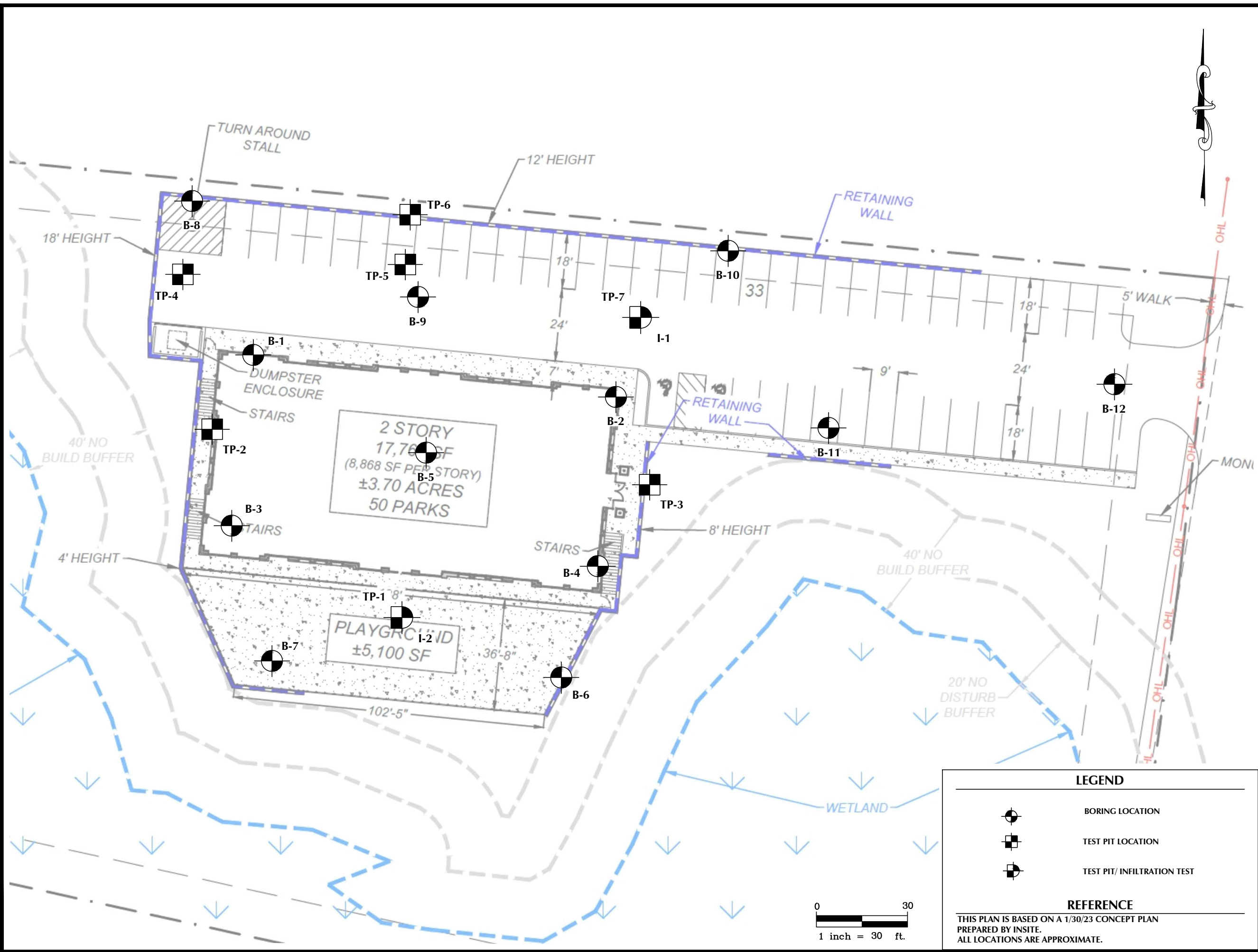
Rating Options

Aggregation Method: Dominant Condition

Component Percent Cutoff: None Specified

Tie-break Rule: Higher

N:\Job Folders\2025\2523022GM\Drawings and Plans\CAD\GM2523022.000.dwg



LEGEND

- BORING LOCATION
- TEST PIT LOCATION
- TEST PIT/ INFILTRATION TEST

REFERENCE

THIS PLAN IS BASED ON A 1/30/23 CONCEPT PLAN
PREPARED BY INSITE.
ALL LOCATIONS ARE APPROXIMATE.

WHITESTONE
An Employee-Owned Company

352 TURNPIKE ROAD, SUITE 105, SOUTHBOROUGH, MA 01772
508.485.0755 WHITESTONEASSOC.COM

DRAWING TITLE: TEST LOCATION PLAN	
CLIENT: BOHLER, LLC	
PROJECT: PROPOSED DAYCARE CENTER BETWEEN 101 AND 109 CAMBRIDGE STREET BURLINGTON, MIDDLESEX COUNTY, MASSACHUSETTS	
PROJECT #: GM2523022.000	
DESIGNED BY: MR	PROJ. MGR.: RR
DATE: 3/18/25	FIGURE: 1
SCALE: 1" = 30'	

APPENDIX A
Records of Subsurface Exploration
(Borings B-1 through B-12;
Test Pits TP-1 through TP-7)



RECORD OF SUBSURFACE EXPLORATION

Boring No.: B-1

Page 1 of 1

Project:		Proposed Daycare Center		WAI Project No.:		GM2523022.000			
Location:		Between 101 and 109 Cambridge Street, Burlington, Middlesex County, Massachusetts				Client:		Bohler, LLC	
Surface Elevation:		± NS		feet Above NAVD88		Date Started:		3/6/2025	
Termination Depth:		18.0		feet bgs		Date Completed:		3/6/2025	
Proposed Location:		Building				Logged By:		TG	
Drill / Test Method:		HSA / SPT (Autohammer)				Contractor:		SE	
						Equipment:		Diedrich D-70	
						Water Depth Elevation		Cave-In Depth Elevation	
						(feet bgs) (ft NAVD88)		(feet bgs) (ft NAVD88)	
						During:		0.0 --	
						At Completion:		-- --	
						24 Hours:		-- --	
								At Completion: -- --	
								24 Hours: -- --	

SAMPLE INFORMATION						DEPTH (feet)	STRATA	DESCRIPTION OF MATERIALS (Classification)	REMARKS
Depth (feet)	No	Type	Blows Per 6"	Rec. (in.)	N				
						0.0	TS	5" Topsoil	
0 - 2	S-1		1 - 3 - 3 - 10	20	6	1.5	SUBSOIL	13" Subsoil, Roots	
2 - 4	S-2		15 - 18 - 21 - 25	18	39		GLACIAL TILL	Brown, Dense, Silty Sand with Gravel (SM)	
5 - 7	S-3		11 - 10 - 7 - 5	19	17			As Above, Medium Dense (SM)	
7 - 9	S-4		4 - 8 - 13 - 20	13	21			As Above (SM)	
10 - 12	S-5		9 - 13 - 18 - 21	21	31			As Above, Dense (SM)	
15 - 17	S-6		25 - 26 - 19 - 23	22	45			As Above (SM)	
17 - 18	S-7		25 - 50	12	100			As Above, Very Dense (SM)	
									Cobbles & Boulders
						20.0		Boring Log B-1 Terminated at Depth of 18 feet below ground surface.	
						25.0			

NOTES: bgs = below ground surface, msl = mean sea level, NA = Not Applicable, NE = Not Encountered, NS = Not Surveyed, P = Perched

RECORD OF SUBSURFACE EXPLORATION

Boring No.: **B-2**

Page 1 of 1

Project: Proposed Daycare Center		WAI Project No.: GM2523022.000	
Location: Between 101 and 109 Cambridge Street, Burlington, Middlesex County, Massachusetts		Client: Bohler, LLC	
Surface Elevation: ± NS feet Above NAVD88	Date Started: 3/5/2025	Water Depth Elevation (feet bgs) (ft NAVD88)	Cave-In Depth Elevation (feet bgs) (ft NAVD88)
Termination Depth: 20.4 feet bgs	Date Completed: 3/5/2025	During: 5.0 -- ▼	At Completion: -- -- ▼
Proposed Location: Building	Logged By: JB	24 Hours: -- -- ▼	At Completion: -- -- ▼
Drill / Test Method: HSA / SPT (Autohammer)	Contractor: SE	24 Hours: -- -- ▼	At Completion: -- -- ▼
	Equipment: Diedrich D-70		

SAMPLE INFORMATION						DEPTH	STRATA	DESCRIPTION OF MATERIALS (Classification)	REMARKS
Depth (feet)	No	Type	Blows Per 6"	Rec. (in.)	N	(feet)			
						0.0	TS	7" Topsoil	
0 - 2	S-1		1 - 1 - 1 - 3	14	2		SUBSOIL	23" Subsoil, Roots	
2 - 4	S-2		7 - 24 - 40 - 30	20	64	2.5		Gray-Brown, Very Dense, Silty Sand with Gravel (SM)	
						5.0		As Above (SM)	
5 - 7	S-3		20 - 26 - 28 - 54	18	54			As Above (SM)	
7 - 9	S-4		36 - 32 - 30 - 46	20	62			As Above (SM)	
						10.0	GLACIAL TILL	As Above, Medium Dense (SM)	
15 - 17	S-6		23 - 29 - 36 - 43	20	65	15.0		As Above, Very Dense (SM)	
20 - 20.4	S-7		50/5"	4	-	20.0		As Above (SM)	Cobbles & Boulders
								Boring Log B-2 Terminated at Depth of 20.4 feet below ground surface.	
						25.0			

NOTES: bgs = below ground surface, msl = mean sea level, NA = Not Applicable, NE = Not Encountered, NS = Not Surveyed, P = Perched



RECORD OF SUBSURFACE EXPLORATION

Boring No.: **B-3**

Page 1 of 1

Project: Proposed Daycare Center		WAI Project No.: GM2523022.000	
Location: Between 101 and 109 Cambridge Street, Burlington, Middlesex County, Massachusetts		Client: Bohler, LLC	
Surface Elevation: ± NS feet Above NAVD88	Date Started: 3/5/2025	Water Depth Elevation (feet bgs) (ft NAVD88)	Cave-In Depth Elevation (feet bgs) (ft NAVD88)
Termination Depth: 22.0 feet bgs	Date Completed: 3/5/2025	During: 2.0 -- ▾	At Completion: -- -- ▾
Proposed Location: Building	Logged By: JB	At Completion: -- -- ▾	At Completion: -- -- ▾
Drill / Test Method: HSA / SPT (Autohammer)	Contractor: SE	24 Hours: -- -- ▾	24 Hours: -- -- ▾
	Equipment: Diedrich D-70		

SAMPLE INFORMATION						DEPTH	STRATA	DESCRIPTION OF MATERIALS (Classification)	REMARKS
Depth (feet)	No	Type	Blows Per 6"	Rec. (in.)	N	(feet)			
						0.0	TS	8" Topsoil	
0 - 2	S-1		1 - 2 - 2 - 8	20	4	2.0	SUBSOIL	16" Subsoil, Roots	
2 - 4	S-2		17 - 23 - 30 - 30	10	53			Brown, Very Dense, Silty Sand with Gravel (SM)	
5 - 7	S-3		6 - 5 - 4 - 12	10	9			Gray, Loose, Sandy Silt with Gravel (ML)	
7 - 9	S-4		5 - 12 - 10 - 10	18	22			As Above, Medium Dense (ML)	
10 - 12	S-5		8 - 16 - 19 - 26	20	35		GLACIAL TILL	As Above, Dense (SM)	
15 - 17	S-6		19 - 24 - 25 - 29	18	49			As Above (SM)	
20 - 22	S-7		31 - 39 - 37 - 43	24	76			As Above, Very Dense (SM)	
								Boring Log B-3 Terminated at Depth of 22 feet below ground surface.	

NOTES: bgs = below ground surface, msl = mean sea level, NA = Not Applicable, NE = Not Encountered, NS = Not Surveyed, P = Perched

RECORD OF SUBSURFACE EXPLORATION

 Boring No.: **B-4**

 Page 1 of 1

Project: Proposed Daycare Center			WAI Project No.: GM2523022.000		
Location: Between 101 and 109 Cambridge Street, Burlington, Middlesex County, Massachusetts			Client: Bohler, LLC		
Surface Elevation: ± NS feet Above NAVD88			Date Started: 3/5/2025		Water Depth Elevation
Termination Depth: 22.0 feet bgs			Date Completed: 3/5/2025		Cave-In Depth Elevation
Proposed Location: Building			Logged By: JB		During: 5.0 --
Drill / Test Method: HSA / SPT (Autohammer)			Contractor: SE		At Completion: -- --
			Equipment: Diedrich D-70		24 Hours: -- --

SAMPLE INFORMATION						DEPTH	STRATA	DESCRIPTION OF MATERIALS (Classification)	REMARKS
Depth (feet)	No	Type	Blows Per 6"	Rec. (in.)	N	(feet)			
						0.0	TS		7" Topsoil
0 - 2	S-1		1 - 2 - 7 - 13	16	9		SUBSOIL		17" Subsoil, Roots
2 - 4	S-2		25 - 22 - 25 - 21	24	47		GLACIAL TILL		Gray-Brown, Dense, Silty Sand with Gravel (SM)
						5.0			As Above (SM)
5 - 7	S-3		20 - 17 - 14 - 10	18	31				As Above, Medium Dense (SM)
7 - 9	S-4		8 - 6 - 8 - 11	24	14				As Above, Dense (SM)
10 - 11.8	S-5		15 - 13 - 18 - 50/ 4"	16	31				As Above, Very Dense (SM)
						15.0			As Above, Medium Dense (SM)
15 - 17	S-6		9 - 14 - 13 - 11	18	27				
						20.0			
20 - 22	S-7		19 - 27 - 28 - 23	22	55				
						25.0			
									Boring Log B-4 Terminated at Depth of 22 feet below ground surface.

NOTES: bgs = below ground surface, msl = mean sea level, NA = Not Applicable, NE = Not Encountered, NS = Not Surveyed, P = Perched



RECORD OF SUBSURFACE EXPLORATION

Boring No.: **B-5**

Page 1 of 1

Project: Proposed Daycare Center		WAI Project No.: GM2523022.000	
Location: Between 101 and 109 Cambridge Street, Burlington, Middlesex County, Massachusetts		Client: Bohler, LLC	
Surface Elevation: ± NS feet Above NAVD88		Date Started: 3/5/2025	Water Depth Elevation (feet bgs) (ft NAVD88)
Termination Depth: 22.0 feet bgs		Date Completed: 3/5/2025	Cave-In Depth Elevation (feet bgs) (ft NAVD88)
Proposed Location: Building		Logged By: JB	During: 7.0 -- ▾
Drill / Test Method: HSA / SPT (Autohammer)		Contractor: SE	At Completion: -- -- ▾
		Equipment: Diedrich D-70	24 Hours: -- -- ▾
			At Completion: -- -- ▾
			24 Hours: -- -- ▾

SAMPLE INFORMATION						DEPTH	STRATA	DESCRIPTION OF MATERIALS (Classification)	REMARKS
Depth (feet)	No	Type	Blows Per 6"	Rec. (in.)	N	(feet)			
						0.0	TS	6" Topsoil	
0 - 2	S-1		1 - 1 - 2 - 3	12	3		SUBSOIL	18" Subsoil, Roots	
2 - 4	S-2		21 - 20 - 31 - 21	22	51			Gray-Brown, Very Dense, Silty Sand with Gravel (SM)	
						5.0		As Above, Medium Dense (SM)	
5 - 7	S-3		15 - 12 - 14 - 24	12	26			As Above (SM)	
7 - 9	S-4		18 - 12 - 13 - 10	12	25			As Above (SM)	
						10.0	GLACIAL TILL	As Above (SM)	
						15.0		As Above, Very Dense (SM)	
15 - 17	S-6		29 - 38 - 32 - 26	14	70			As Above (SM)	
						20.0			
20 - 22	S-7		32 - 31 - 34 - 30	20	65				
						25.0		Boring Log B-5 Terminated at Depth of 22 feet below ground surface.	

NOTES: bgs = below ground surface, msl = mean sea level, NA = Not Applicable, NE = Not Encountered, NS = Not Surveyed, P = Perched

RECORD OF SUBSURFACE EXPLORATION

Boring No.: **B-6**

Page 1 of 1

Project: Proposed Daycare Center		WAI Project No.: GM2523022.000	
Location: Between 101 and 109 Cambridge Street, Burlington, Middlesex County, Massachusetts		Client: Bohler, LLC	
Surface Elevation: ± NS feet Above NAVD88	Date Started: 3/5/2025	Water Depth Elevation (feet bgs) (ft NAVD88)	Cave-In Depth Elevation (feet bgs) (ft NAVD88)
Termination Depth: 17.0 feet bgs	Date Completed: 3/5/2025	During: 5.0 --	At Completion: -- --
Proposed Location: Retaining Wall	Logged By: JB	At Completion: -- --	At Completion: -- --
Drill / Test Method: HSA / SPT (Autohammer)	Contractor: SE	24 Hours: -- --	24 Hours: -- --
	Equipment: Diedrich D-70		

SAMPLE INFORMATION						DEPTH	STRATA	DESCRIPTION OF MATERIALS (Classification)	REMARKS
Depth (feet)	No	Type	Blows Per 6"	Rec. (in.)	N	(feet)			
						0.0	TS	6" Topsoil	
0 - 2	S-1		1 - 2 - 3 - 10	18	5		SUBSOIL	18" Subsoil, Roots	
2 - 4	S-2		22 - 41 - 28 - 23	18	69		GLACIAL TILL	Gray-Brown, Very Dense, Silty Sand with Gravel (SM)	
						5.0		As Above, Dense (SM)	
5 - 6.8	S-3		18 - 30 - 18 - 50/3"	8	48				Cobbles & Boulders
						10.0		As Above (SM)	
10 - 12	S-4		20 - 23 - 22 - 18	20	45				
						15.0		As Above (SM)	
15 - 17	S-5		22 - 24 - 25 - 27	20	49				
						20.0			
						25.0			
								Boring Log B-6 Terminated at Depth of 17 feet below ground surface.	

NOTES: bgs = below ground surface, msl = mean sea level, NA = Not Applicable, NE = Not Encountered, NS = Not Surveyed, P = Perched



RECORD OF SUBSURFACE EXPLORATION

Boring No.: **B-7**

Page 1 of 1

Project: Proposed Daycare Center		WAI Project No.: GM2523022.000	
Location: Between 101 and 109 Cambridge Street, Burlington, Middlesex County, Massachusetts		Client: Bohler, LLC	
Surface Elevation: ± NS feet Above NAVD88	Date Started: 3/5/2025	Water Depth Elevation (feet bgs) (ft NAVD88)	
Termination Depth: 17.0 feet bgs	Date Completed: 3/5/2025	Cave-In Depth Elevation (feet bgs) (ft NAVD88)	
Proposed Location: Retaining Wall	Logged By: JB	During: 2.0 -- ▽	
Drill / Test Method: HSA / SPT (Autohammer)	Contractor: SE	At Completion: -- -- ▽	
	Equipment: Diedrich D-70	24 Hours: -- -- ▽	
		At Completion: -- -- ▽	
		24 Hours: -- -- ▽	

SAMPLE INFORMATION						DEPTH	STRATA	DESCRIPTION OF MATERIALS (Classification)	REMARKS
Depth (feet)	No	Type	Blows Per 6"	Rec. (in.)	N	(feet)			
						0.0	TS	6" Topsoil	
0 - 2	S-1	X	2 - 1 - 2 - 9	18	3	2.0	SUBSOIL	18" Subsoil, Roots	
2 - 4	S-2	X	29 - 36 - 25 - 16	10	61	5.0		Brown-Gray, Very Dense, Silty Sand with Gravel (SM)	
5 - 7	S-3	X	5 - 4 - 6 - 6	18	10	8.0		Brown, Loose to Medium Dense, Sandy Silt with Gravel (ML)	
7 - 8.8	S-4	X	9 - 26 - 15 - 50/3"	20	41	10.0		As Above, Dense (ML)	
						15.0	GLACIAL TILL	Brown, Dense, Silty Sand with Gravel (SM)	Cobbles & Boulders
10 - 12	S-5	X	9 - 10 - 13 - 19	16	23			As Above, Medium Dense (SM)	
15 - 17	S-6	X	20 - 27 - 30 - 31	24	57			As Above, Brown to Gray, Very Dense (SM)	
						20.0		Boring Log B-7 Terminated at Depth of 17 feet below ground surface.	
						25.0			

NOTES: bgs = below ground surface, msl = mean sea level, NA = Not Applicable, NE = Not Encountered, NS = Not Surveyed, P = Perched



RECORD OF SUBSURFACE EXPLORATION

Boring No.: **B-8**

Page **1** of **1**

Project: Proposed Daycare Center		WAI Project No.: GM2523022.000	
Location: Between 101 and 109 Cambridge Street, Burlington, Middlesex County, Massachusetts		Client: Bohler, LLC	
Surface Elevation: ± NS feet Above NAVD88		Date Started: 3/6/2025	Water Depth Elevation (feet bgs) (ft NAVD88)
Termination Depth: 14.0 feet bgs		Date Completed: 3/6/2025	Cave-In Depth Elevation (feet bgs) (ft NAVD88)
Proposed Location: Retaining Wall		Logged By: TG	During: 0.0 -- ▾
Drill / Test Method: HSA / SPT (Autohammer)		Contractor: SE	At Completion: -- -- ▾
		Equipment: Diedrich D-70	24 Hours: -- -- ▾
			At Completion: -- -- ▾
			24 Hours: -- -- ▾

SAMPLE INFORMATION						DEPTH	STRATA	DESCRIPTION OF MATERIALS (Classification)	REMARKS
Depth (feet)	No	Type	Blows Per 6"	Rec. (in.)	N	(feet)			
						0.0	TS	4" Topsoil	
0 - 2	S-1		1 - 2 - 7 - 7	18	9	1.1	SUBSOIL	9" Subsoil, Roots	
								Brown, Medium Dense, Silty Sand with Gravel (SM)	
2 - 4	S-2		9 - 22 - 36 - 53	16	58			As Above, Very Dense (SM)	Cobbles & Boulders
						5.0			
5 - 7	S-3		26 - 17 - 9 - 7	13	26			As Above, Medium Dense (SM)	
							GLACIAL TILL		
7 - 9	S-4		13 - 10 - 15 - 22	11	25			As Above (SM)	
						10.0			
10 - 12	S-5		12 - 12 - 23 - 30	17	35			As Above, Dense (SM)	
12 - 14	S-6		34 - 42 - 41 - 53	12	83			As Above, Very Dense (SM)	
						15.0			
						20.0			
						25.0			
								Boring Log B-8 Terminated at Depth of 14 feet below ground surface.	

NOTES: bgs = below ground surface, msl = mean sea level, NA = Not Applicable, NE = Not Encountered, NS = Not Surveyed, P = Perched



RECORD OF SUBSURFACE EXPLORATION

Boring No.: **B-9**

Page **1** of **1**

Project: Proposed Daycare Center		WAI Project No.: GM2523022.000	
Location: Between 101 and 109 Cambridge Street, Burlington, Middlesex County, Massachusetts		Client: Bohler, LLC	
Surface Elevation: ± NS feet Above NAVD88		Date Started: 3/6/2025	Water Depth Elevation (feet bgs) (ft NAVD88)
Termination Depth: 9.0 feet bgs		Date Completed: 3/6/2025	Cave-In Depth Elevation (feet bgs) (ft NAVD88)
Proposed Location: Parking		Logged By: TG	During: 0.0 -- ▾
Drill / Test Method: HSA / SPT (Autohammer)		Contractor: SE	At Completion: -- -- ▾
		Equipment: Diedrich D-70	24 Hours: -- -- ▾
			At Completion: -- -- ▾
			24 Hours: -- -- ▾

SAMPLE INFORMATION						DEPTH	STRATA	DESCRIPTION OF MATERIALS (Classification)	REMARKS
Depth (feet)	No	Type	Blows Per 6"	Rec. (in.)	N	(feet)			
						0.0	TS	7" Topsoil	
0 - 2	S-1		1 - 1 - 2 - 8	14	3		SUBSOIL	17" Subsoil, Roots	
2 - 4	S-2		21 - 16 - 17 - 27	17	33			Brown, Dense, Silty Sand with Gravel (SM)	
						5.0	GLACIAL TILL	As Above (SM)	
5 - 7	S-3		23 - 25 - 12 - 9	11	37			As Above (SM)	
7 - 9	S-4		10 - 15 - 16 - 11	9	31			As Above (SM)	
						10.0		Boring Log B-9 Terminated at Depth of 9 feet below ground surface.	
						15.0			
						20.0			
						25.0			

NOTES: bgs = below ground surface, msl = mean sea level, NA = Not Applicable, NE = Not Encountered, NS = Not Surveyed, P = Perched

RECORD OF SUBSURFACE EXPLORATION
Bohler Gardner Daycare Burlington MA GM2523022 Boring Logs 3-5 and 6-25 4/1/2025



RECORD OF SUBSURFACE EXPLORATION

Boring No.: **B-11**

Page **1** of **1**

Project: Proposed Daycare Center		WAI Project No.: GM2523022.000	
Location: Between 101 and 109 Cambridge Street, Burlington, Middlesex County, Massachusetts		Client: Bohler, LLC	
Surface Elevation: ± <u>NS</u> feet Above NAVD88		Date Started: <u>3/6/2025</u>	Water Depth Elevation (feet bgs) (ft NAVD88)
Termination Depth: <u>13.8</u> feet bgs		Date Completed: <u>3/6/2025</u>	Cave-In Depth Elevation (feet bgs) (ft NAVD88)
Proposed Location: <u>Retaing Wall</u>		Logged By: <u>TG</u>	During: <u>0.5</u> --
Drill / Test Method: <u>HSA / SPT (Autohammer)</u>		Contractor: <u>SE</u>	At Completion: -- --
		Equipment: <u>Diedrich D-70</u>	24 Hours: -- --
			At Completion: -- --
			24 Hours: -- --

SAMPLE INFORMATION						DEPTH	STRATA	DESCRIPTION OF MATERIALS (Classification)	REMARKS
Depth (feet)	No	Type	Blows Per 6"	Rec. (in.)	N	(feet)			
						0.0			
							TS	4" Topsoil	
0 - 2	S-1		2 - 3 - 11 - 25	16	14	0.8	SUBSOIL	6" Subsoil, Roots	
								Brown, Medium Dense, Silty Sand with Gravel (SM)	
2 - 3.4	S-2		28 - 30 - 50/5"	8	60			As Above, Very Dense (SM)	Cobbles & Boulders
						5.0		As Above, Medium Dense (SM)	
5 - 7	S-3		11 - 13 - 15 - 12	15	28		GLACIAL TILL	As Above, Gray, Dense (SM)	
						10.0		As Above (SM)	
10 - 12	S-5		10 - 17 - 19 - 24	14	36			As Above, Very Dense (SM)	
12 - 13.8	S-6		22 - 33 - 54 - 54/3"	20	87				Cobbles & Boulders
						15.0		Boring Log B-11 Terminated at Depth of 13.8 feet below ground surface.	
						20.0			
						25.0			

NOTES: bgs = below ground surface, msl = mean sea level, NA = Not Applicable, NE = Not Encountered, NS = Not Surveyed, P = Perched

RECORD OF SUBSURFACE EXPLORATION
Bohler Gardner Daycare Burlington MA GM2523022 Boring Logs 3-5 and 6-25 4/1/2025

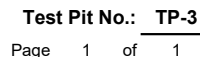


RECORD OF SUBSURFACE EXPLORATION

Test Pit No.: **TP-1**
Page 1 of 1

Project: Proposed Daycare Center		WAI Project No.: GM2523022.000	
Location: Between 101 & 109 Cambridge Street, Burlington, Middlesex County, Massachusetts		Client: Bohler, LLC	
Surface Elevation: ± NS feet NAVD88	Date Started: 3/4/2025	Water Depth Elevation (feet bgs) (ft NAVD88)	Cave-In Depth Elevation (feet bgs) (ft NAVD88)
Termination Depth: 9.5 feet bgs	Date Completed: 3/4/2025	During: 3.5 --	At Completion: -- --
Proposed Location: SWM Area	Logged By: JB	At Completion: -- --	At Completion: -- --
Excavating Method: Mini Excavator	Contractor: SE	24 Hours: -- --	
Test Method: Visual Observation	Rig Type: Caterpillar 308		

SAMPLE INFORMATION			DEPTH	STRATA	DESCRIPTION OF MATERIALS (Classification)	REMARKS
Depth (ft.)	Number	Type	(feet)			
			0.0	TOPSOIL	4" Topsoil	
				SUBSOIL	20" Subsoil, Roots	
			2.0			
					Gray, Sandy Silt (ML)	ESHWG @ 2.5 fbgs
			3.0			
						Infiltration test @ 3.5 fbgs
5	1	Grab	5.0	GLACIAL TILL	Gray-Brown, Silty Sand with Gravel, Cobbles, Boulders (SM)	
			10.0		Test Pit TP-1 Terminated at Depth of 9.5 feet below ground surface.	
			13.0			
			15.0			





RECORD OF SUBSURFACE EXPLORATION

Test Pit No.: **TP-4**
Page 1 of 1

Project: Proposed Daycare Center		WAI Project No.: GM2523022.000	
Location: Between 101 & 109 Cambridge Street, Burlington, Middlesex County, Massachusetts		Client: Bohler, LLC	
Surface Elevation: ± NS feet NAVD88	Date Started: 3/4/2025	Water Depth Elevation (feet bgs) (ft NAVD88)	Cave-In Depth Elevation (feet bgs) (ft NAVD88)
Termination Depth: 7.5 feet bgs	Date Completed: 3/4/2025	During: 3.0 --	At Completion: -- --
Proposed Location: SWM Area	Logged By: JB	At Completion: -- --	At Completion: -- --
Excavating Method: Mini Excavator	Contractor: SE	24 Hours: -- --	
Test Method: Visual Observation	Rig Type: Caterpillar 308		

SAMPLE INFORMATION			DEPTH	STRATA	DESCRIPTION OF MATERIALS (Classification)	REMARKS
Depth (ft.)	Number	Type	(feet)			
			0.0	TOPSOIL	6" Topsoil	ESHWG @ 2.5 fbgs
				SUBSOIL	24" Subsoil	
			2.5			
				GLACIAL TILL	Gray, Silty Sand with Gravel, Cobbles, Boulders (SM)	
					Test Pit TP-4 Terminated at Depth of 7.5 feet below ground surface. Refusal on boulder	
			10.0			
			13.0			
			15.0			

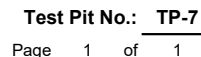


RECORD OF SUBSURFACE EXPLORATION

Test Pit No.: **TP-5**
Page 1 of 1

Project: Proposed Daycare Center		WAI Project No.: GM2523022.000	
Location: Between 101 & 109 Cambridge Street, Burlington, Middlesex County, Massachusetts		Client: Bohler, LLC	
Surface Elevation: ± NS feet NAVD88	Date Started: 3/4/2025	Water Depth Elevation (feet bgs) (ft NAVD88)	Cave-In Depth Elevation (feet bgs) (ft NAVD88)
Termination Depth: 6.5 feet bgs	Date Completed: 3/4/2025	During: 2.0 --	At Completion: -- --
Proposed Location: SWM Area	Logged By: JB	At Completion: -- --	At Completion: -- --
Excavating Method: Mini Excavator	Contractor: SE	24 Hours: -- --	
Test Method: Visual Observation	Rig Type: Caterpillar 308		

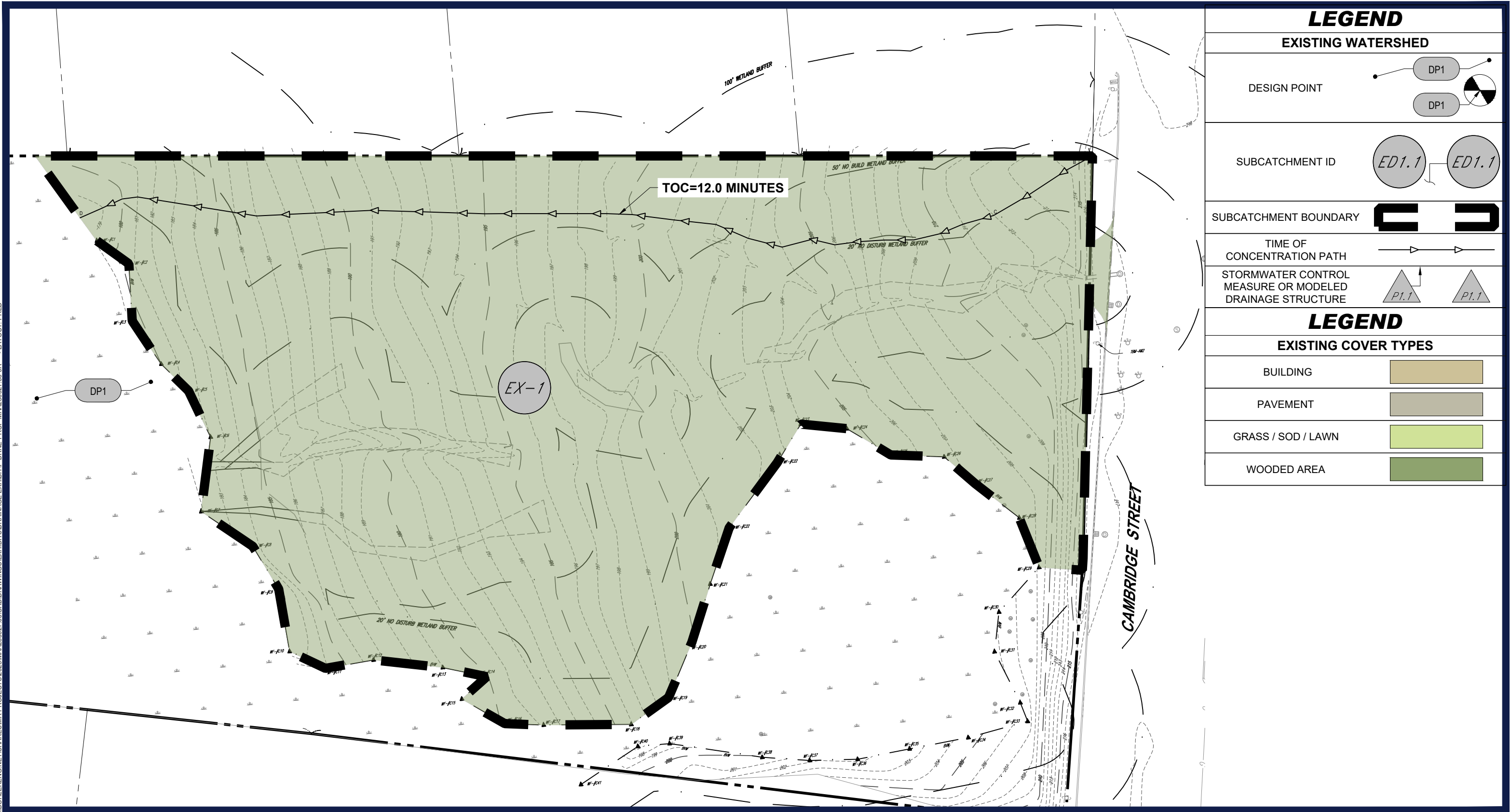
SAMPLE INFORMATION			DEPTH	STRATA	DESCRIPTION OF MATERIALS (Classification)	REMARKS
Depth (ft.)	Number	Type	(feet)			
			0.0	TOPSOIL	8" Topsoil	
				SUBSOIL	28" Subsoil, Roots	ESHWG @ 1.5 fbgs
			3.0	GLACIAL TILL	Gray, Silty Sand with Gravel, Cobbles, Boulders (SM)	
5	1	Grab	5.0			
					Test Pit TP-5 Terminated at Depth of 6.5 feet below ground surface.	
			10.0			
			13.0			
			15.0			



APPENDIX D: EXISTING CONDITIONS HYDROLOGIC ANALYSIS

- EXISTING CONDITIONS DRAINAGE MAP
- EXISTING CONDITIONS HYDROCAD COMPUTATIONS

\\BOHLER\NET\SHARES\MA-PROJECTS\2025\MAA250027.00\CAD\DRAWINGS\EXHIBITS\DRAINAGE EXHIBIT\TP-DRNE-PROP-MAA250027.00-0A----->LAYOUT-PRD



LEGEND

EXISTING WATERSHED

DESIGN POINT

DP1

DP1

SUBCATCHMENT ID

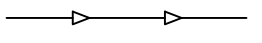
ED1.1

ED1.1

SUBCATCHMENT BOUNDARY



TIME OF
CONCENTRATION PATH



STORMWATER CONTROL
MEASURE OR MODELED
DRAINAGE STRUCTURE



LEGEND

EXISTING COVER TYPES

BUILDING



PAVEMENT



GRASS / SOD / LAWN



WOODED AREA



BOHLER™

50 WASHINGTON ST., SUITE 2000
WESTBOROUGH, MA 01581
Phone: (508) 480-9900

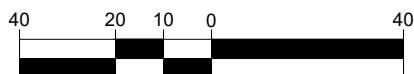
www.BohlerEngineering.com

EXISTING CONDITIONS WATERSHED MAP

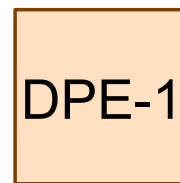
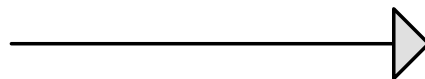
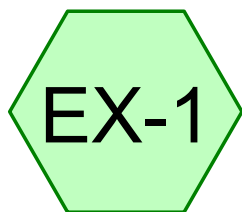
FOXBOROUGH LEARNING, LLC

BURLINGTON, MA 01803

12/16/2025 | JT | MAA250027.00 | REV 0a

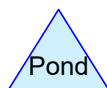
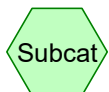


SCALE: 1" = 40'



to Wetlands

Wetlands



Routing Diagram for MAA250027 Model

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MAA250027 Model

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Rainfall Events Listing (selected events)

Event#	Event Name	Storm Type	Curve	Mode	Duration (hours)	B/B	Depth (inches)	AMC
1	2 yr	Type III 24-hr		Default	24.00	1	4.00	2
2	10 yr	Type III 24-hr		Default	24.00	1	6.37	2
3	25 yr	Type III 24-hr		Default	24.00	1	8.25	2
4	100 yr	Type III 24-hr		Default	24.00	1	11.40	2

MAA250027 Model

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Area Listing (selected nodes)

Area (acres)	CN	Description (subcatchment-numbers)
1.736	70	Woods, Good, HSG C (EX-1)
1.736	70	TOTAL AREA

MAA250027 Model

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Soil Listing (selected nodes)

Area (acres)	Soil Group	Subcatchment Numbers
0.000	HSG A	
0.000	HSG B	
1.736	HSG C	EX-1
0.000	HSG D	
0.000	Other	
1.736		TOTAL AREA

MAA250027 Model

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Ground Covers (selected nodes)

HSG-A (acres)	HSG-B (acres)	HSG-C (acres)	HSG-D (acres)	Other (acres)	Total (acres)	Ground Cover	Subcatchment Numbers
0.000	0.000	1.736	0.000	0.000	1.736	Woods, Good	EX-1
0.000	0.000	1.736	0.000	0.000	1.736	TOTAL AREA	

MAA250027 Model*Type III 24-hr 2 yr Rainfall=4.00"*

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Time span=0.00-24.00 hrs, dt=0.05 hrs, 481 points

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN

Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

Subcatchment EX-1: to WetlandsRunoff Area=75,601 sf 0.00% Impervious Runoff Depth>1.33"
Flow Length=451' Tc=12.0 min CN=70 Runoff=2.07 cfs 0.192 af**Reach DPE-1: Wetlands**Inflow=2.07 cfs 0.192 af
Outflow=2.07 cfs 0.192 af**Total Runoff Area = 1.736 ac Runoff Volume = 0.192 af Average Runoff Depth = 1.33"**
100.00% Pervious = 1.736 ac 0.00% Impervious = 0.000 ac

MAA250027 Model

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Type III 24-hr 2 yr Rainfall=4.00"

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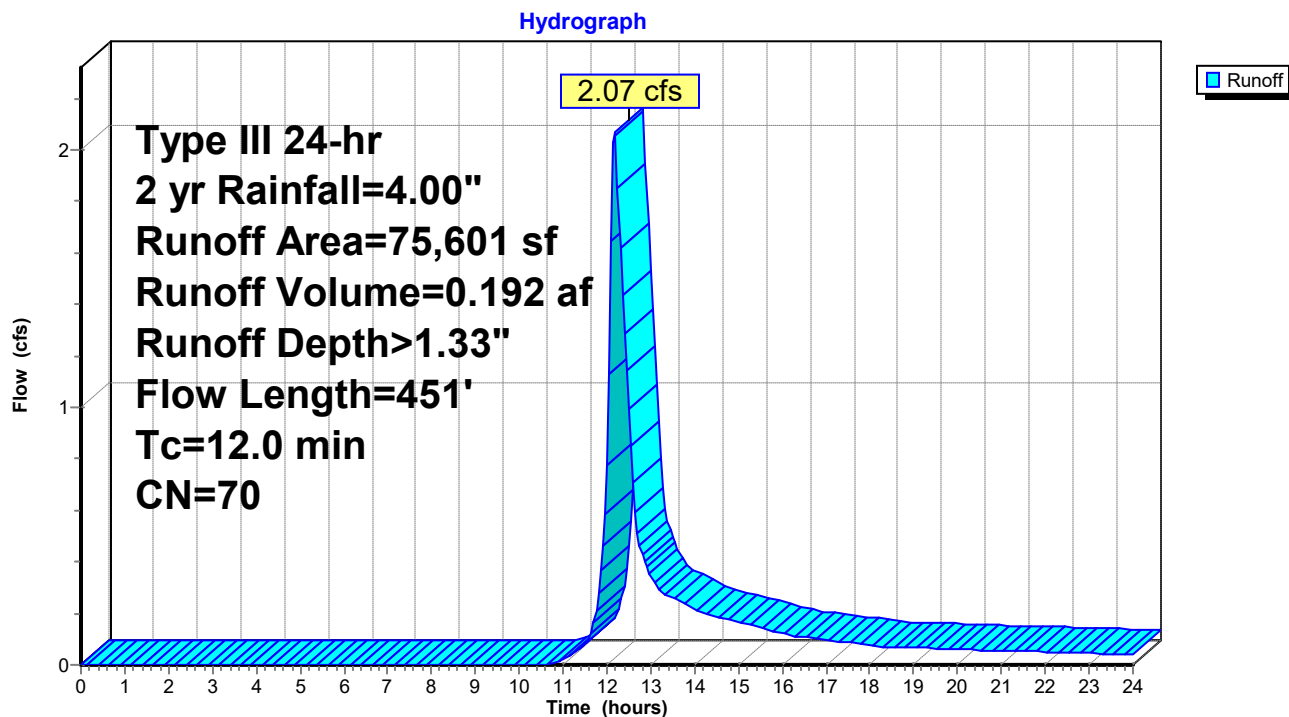
Summary for Subcatchment EX-1: to Wetlands

Runoff = 2.07 cfs @ 12.18 hrs, Volume= 0.192 af, Depth> 1.33"
 Routed to Reach DPE-1 : Wetlands

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs
 Type III 24-hr 2 yr Rainfall=4.00"

Area (sf)	CN	Description
75,601	70	Woods, Good, HSG C
75,601		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
0.9	7	0.2860	0.13		Sheet Flow, A to B
					Woods: Light underbrush n= 0.400 P2= 3.28"
6.5	43	0.0700	0.11		Sheet Flow, B to C
					Woods: Light underbrush n= 0.400 P2= 3.28"
4.6	401	0.0840	1.45		Shallow Concentrated Flow, C to D
					Woodland Kv= 5.0 fps
12.0	451	Total			

Subcatchment EX-1: to Wetlands

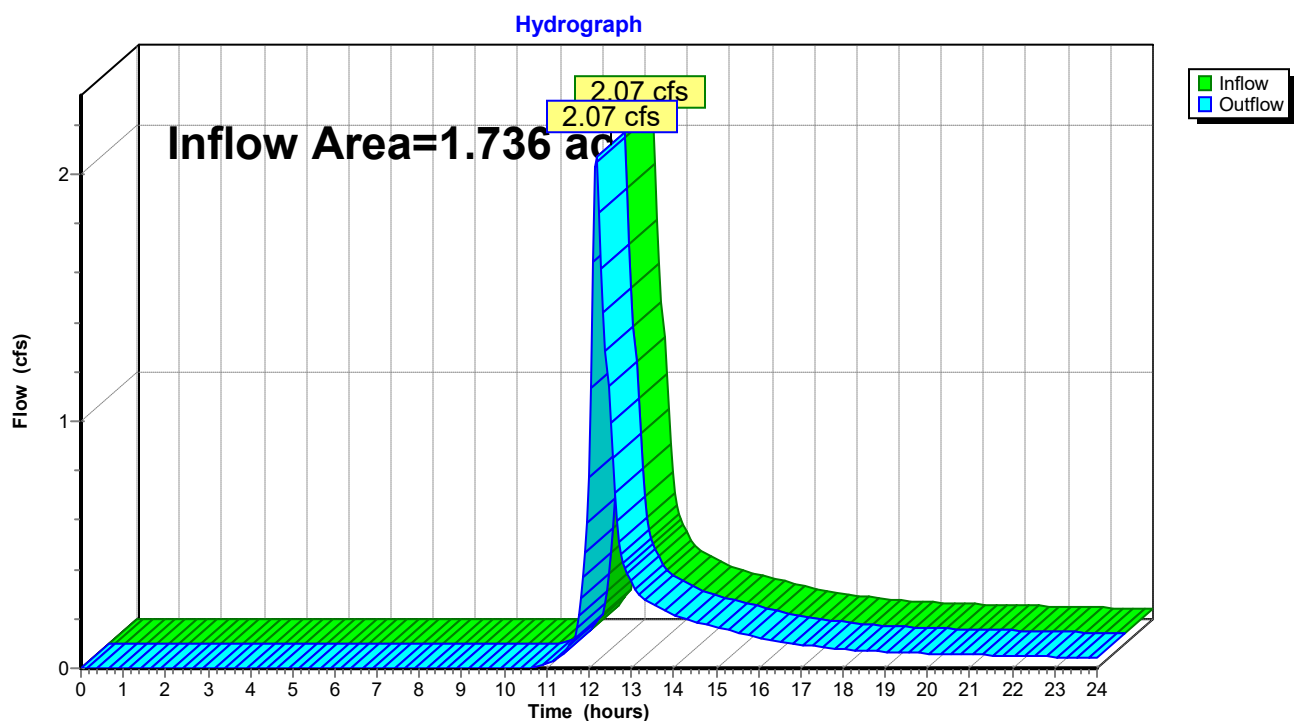
Summary for Reach DPE-1: Wetlands

[40] Hint: Not Described (Outflow=Inflow)

Inflow Area = 1.736 ac, 0.00% Impervious, Inflow Depth > 1.33" for 2 yr event
 Inflow = 2.07 cfs @ 12.18 hrs, Volume= 0.192 af
 Outflow = 2.07 cfs @ 12.18 hrs, Volume= 0.192 af, Atten= 0%, Lag= 0.0 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs

Reach DPE-1: Wetlands



MAA250027 Model*Type III 24-hr 10 yr Rainfall=6.37"*

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Time span=0.00-24.00 hrs, dt=0.05 hrs, 481 points

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN

Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

Subcatchment EX-1: to WetlandsRunoff Area=75,601 sf 0.00% Impervious Runoff Depth>3.09"
Flow Length=451' Tc=12.0 min CN=70 Runoff=5.13 cfs 0.448 af**Reach DPE-1: Wetlands**Inflow=5.13 cfs 0.448 af
Outflow=5.13 cfs 0.448 af**Total Runoff Area = 1.736 ac Runoff Volume = 0.448 af Average Runoff Depth = 3.09"**
100.00% Pervious = 1.736 ac 0.00% Impervious = 0.000 ac

MAA250027 Model

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Type III 24-hr 10 yr Rainfall=6.37"

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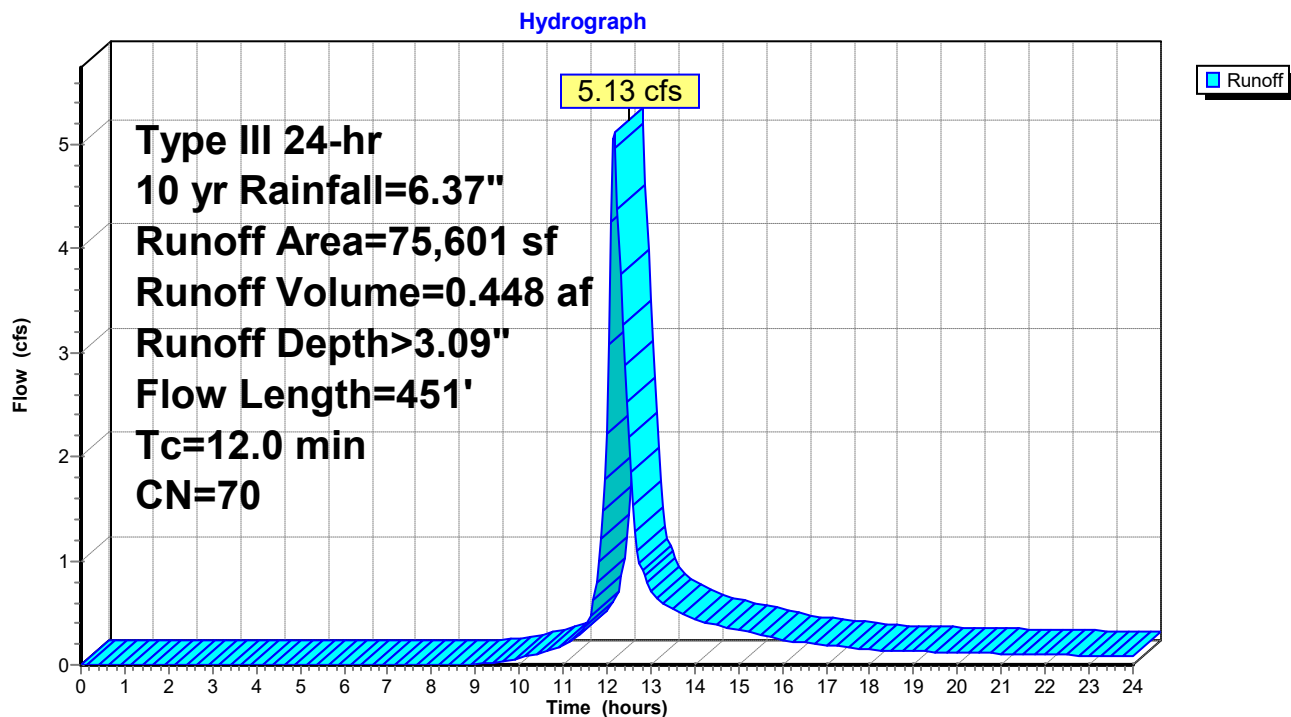
Summary for Subcatchment EX-1: to Wetlands

Runoff = 5.13 cfs @ 12.17 hrs, Volume= 0.448 af, Depth> 3.09"
 Routed to Reach DPE-1 : Wetlands

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs
 Type III 24-hr 10 yr Rainfall=6.37"

Area (sf)	CN	Description
75,601	70	Woods, Good, HSG C
75,601		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
0.9	7	0.2860	0.13		Sheet Flow, A to B
					Woods: Light underbrush n= 0.400 P2= 3.28"
6.5	43	0.0700	0.11		Sheet Flow, B to C
					Woods: Light underbrush n= 0.400 P2= 3.28"
4.6	401	0.0840	1.45		Shallow Concentrated Flow, C to D
					Woodland Kv= 5.0 fps
12.0	451	Total			

Subcatchment EX-1: to Wetlands

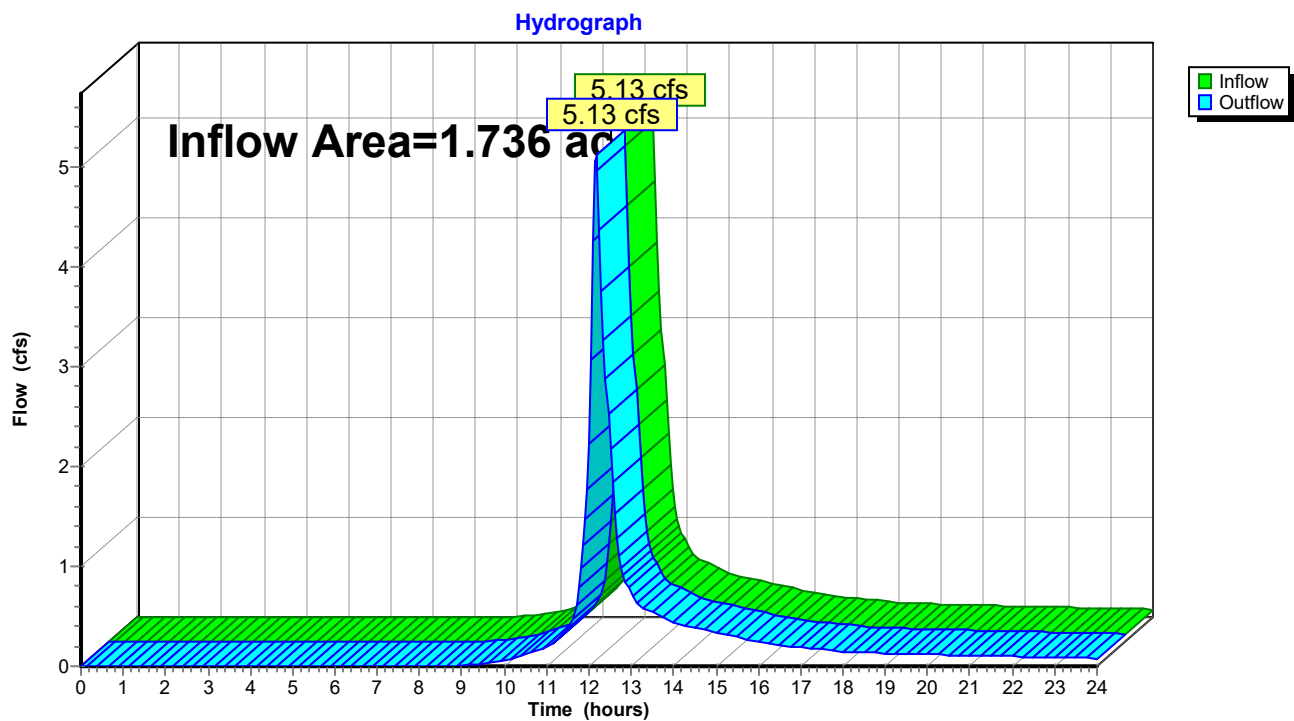
Summary for Reach DPE-1: Wetlands

[40] Hint: Not Described (Outflow=Inflow)

Inflow Area = 1.736 ac, 0.00% Impervious, Inflow Depth > 3.09" for 10 yr event
 Inflow = 5.13 cfs @ 12.17 hrs, Volume= 0.448 af
 Outflow = 5.13 cfs @ 12.17 hrs, Volume= 0.448 af, Atten= 0%, Lag= 0.0 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs

Reach DPE-1: Wetlands



MAA250027 Model*Type III 24-hr 25 yr Rainfall=8.25"*

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Time span=0.00-24.00 hrs, dt=0.05 hrs, 481 points

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN

Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

Subcatchment EX-1: to WetlandsRunoff Area=75,601 sf 0.00% Impervious Runoff Depth>4.67"
Flow Length=451' Tc=12.0 min CN=70 Runoff=7.78 cfs 0.675 af**Reach DPE-1: Wetlands**Inflow=7.78 cfs 0.675 af
Outflow=7.78 cfs 0.675 af**Total Runoff Area = 1.736 ac Runoff Volume = 0.675 af Average Runoff Depth = 4.67"**
100.00% Pervious = 1.736 ac 0.00% Impervious = 0.000 ac

MAA250027 Model

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Type III 24-hr 25 yr Rainfall=8.25"

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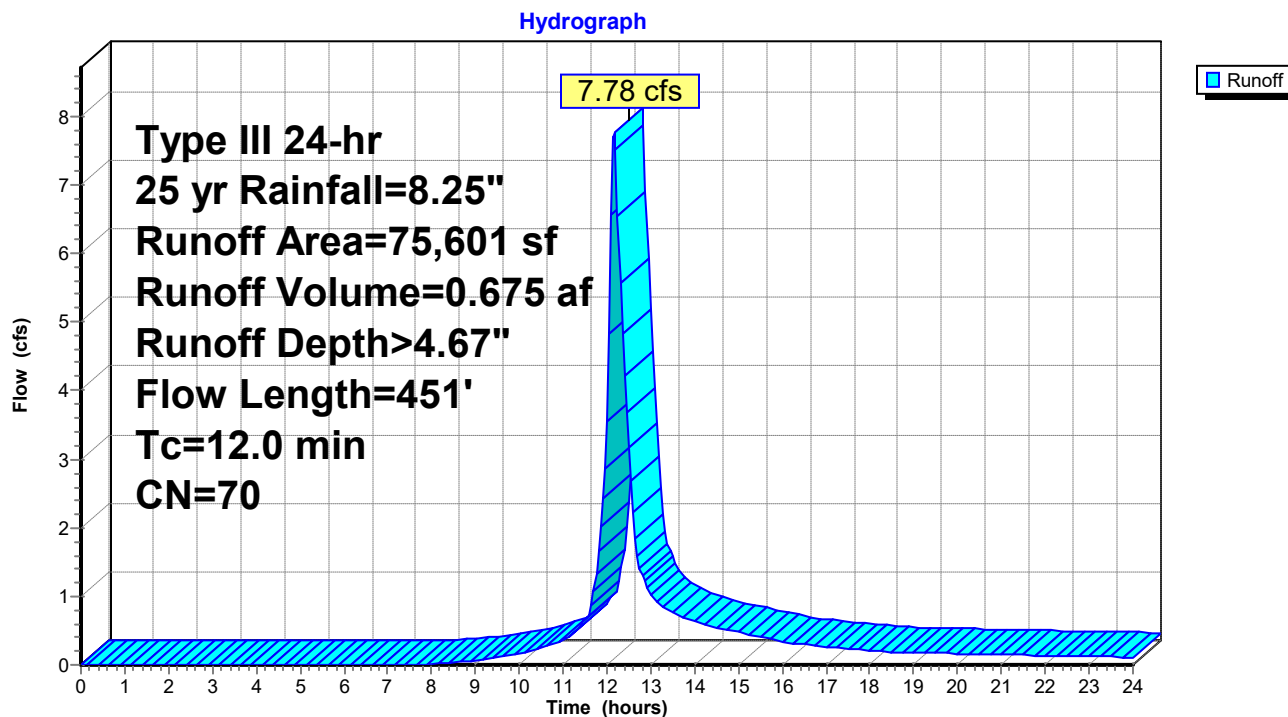
Summary for Subcatchment EX-1: to Wetlands

Runoff = 7.78 cfs @ 12.17 hrs, Volume= 0.675 af, Depth> 4.67"
 Routed to Reach DPE-1 : Wetlands

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs
 Type III 24-hr 25 yr Rainfall=8.25"

Area (sf)	CN	Description
75,601	70	Woods, Good, HSG C
75,601		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
0.9	7	0.2860	0.13		Sheet Flow, A to B
					Woods: Light underbrush n= 0.400 P2= 3.28"
6.5	43	0.0700	0.11		Sheet Flow, B to C
					Woods: Light underbrush n= 0.400 P2= 3.28"
4.6	401	0.0840	1.45		Shallow Concentrated Flow, C to D
					Woodland Kv= 5.0 fps
12.0	451	Total			

Subcatchment EX-1: to Wetlands

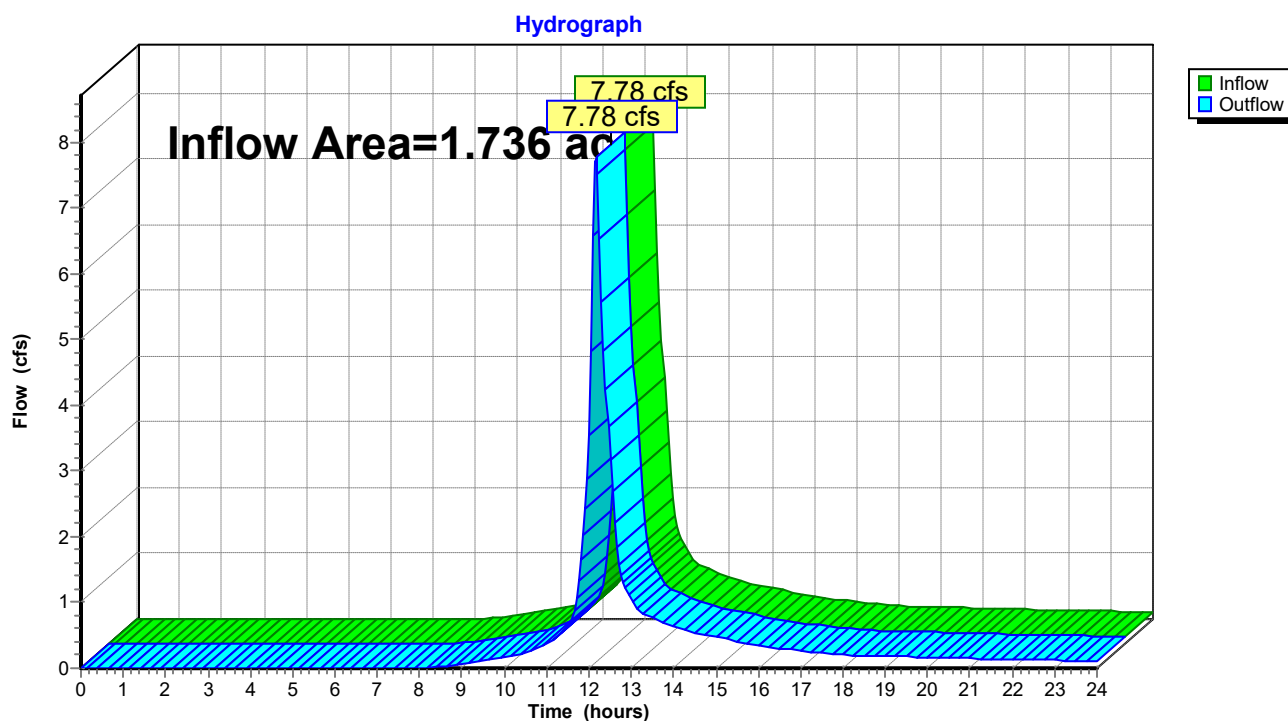
Summary for Reach DPE-1: Wetlands

[40] Hint: Not Described (Outflow=Inflow)

Inflow Area = 1.736 ac, 0.00% Impervious, Inflow Depth > 4.67" for 25 yr event
 Inflow = 7.78 cfs @ 12.17 hrs, Volume= 0.675 af
 Outflow = 7.78 cfs @ 12.17 hrs, Volume= 0.675 af, Atten= 0%, Lag= 0.0 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs

Reach DPE-1: Wetlands



MAA250027 Model*Type III 24-hr 100 yr Rainfall=11.40"*

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Time span=0.00-24.00 hrs, dt=0.05 hrs, 481 points

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN

Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

Subcatchment EX-1: to WetlandsRunoff Area=75,601 sf 0.00% Impervious Runoff Depth>7.48"
Flow Length=451' Tc=12.0 min CN=70 Runoff=12.38 cfs 1.082 af**Reach DPE-1: Wetlands**Inflow=12.38 cfs 1.082 af
Outflow=12.38 cfs 1.082 af**Total Runoff Area = 1.736 ac Runoff Volume = 1.082 af Average Runoff Depth = 7.48"**
100.00% Pervious = 1.736 ac 0.00% Impervious = 0.000 ac

MAA250027 Model

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Type III 24-hr 100 yr Rainfall=11.40"

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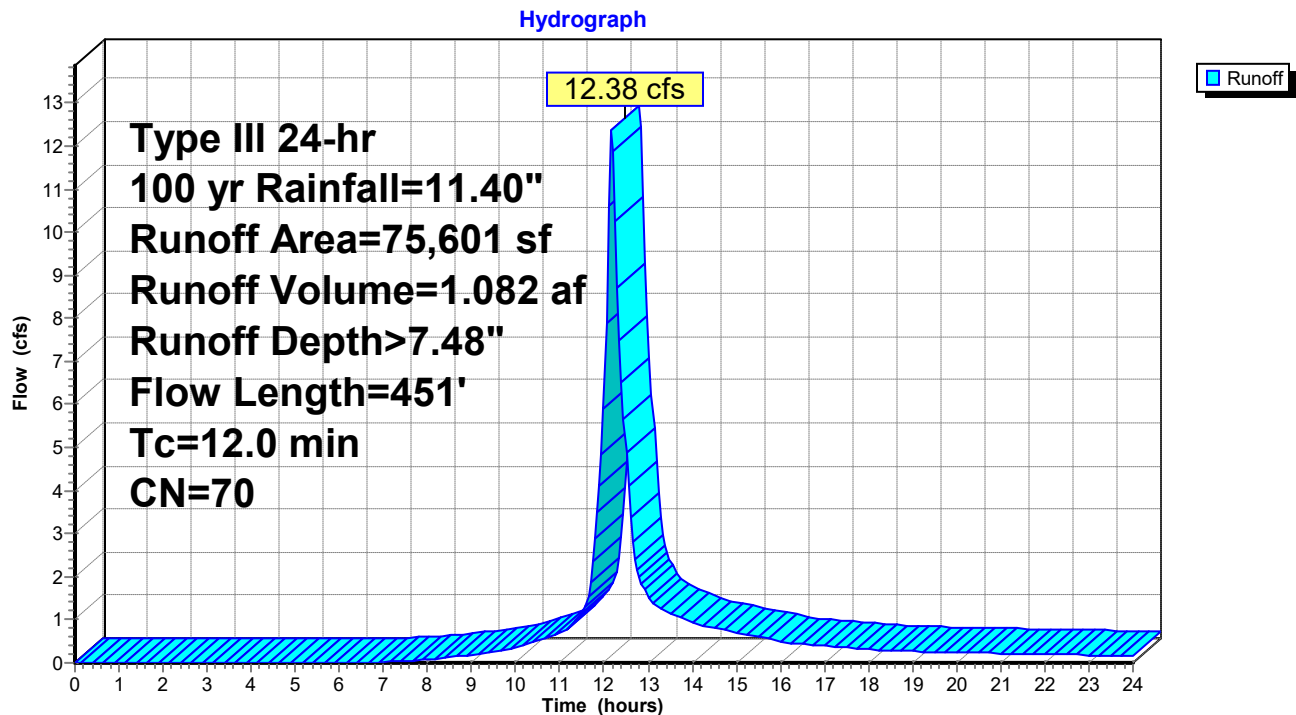
Summary for Subcatchment EX-1: to Wetlands

Runoff = 12.38 cfs @ 12.17 hrs, Volume= 1.082 af, Depth> 7.48"
 Routed to Reach DPE-1 : Wetlands

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs
 Type III 24-hr 100 yr Rainfall=11.40"

Area (sf)	CN	Description
75,601	70	Woods, Good, HSG C
75,601		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
0.9	7	0.2860	0.13		Sheet Flow, A to B
					Woods: Light underbrush n= 0.400 P2= 3.28"
6.5	43	0.0700	0.11		Sheet Flow, B to C
					Woods: Light underbrush n= 0.400 P2= 3.28"
4.6	401	0.0840	1.45		Shallow Concentrated Flow, C to D
					Woodland Kv= 5.0 fps
12.0	451	Total			

Subcatchment EX-1: to Wetlands

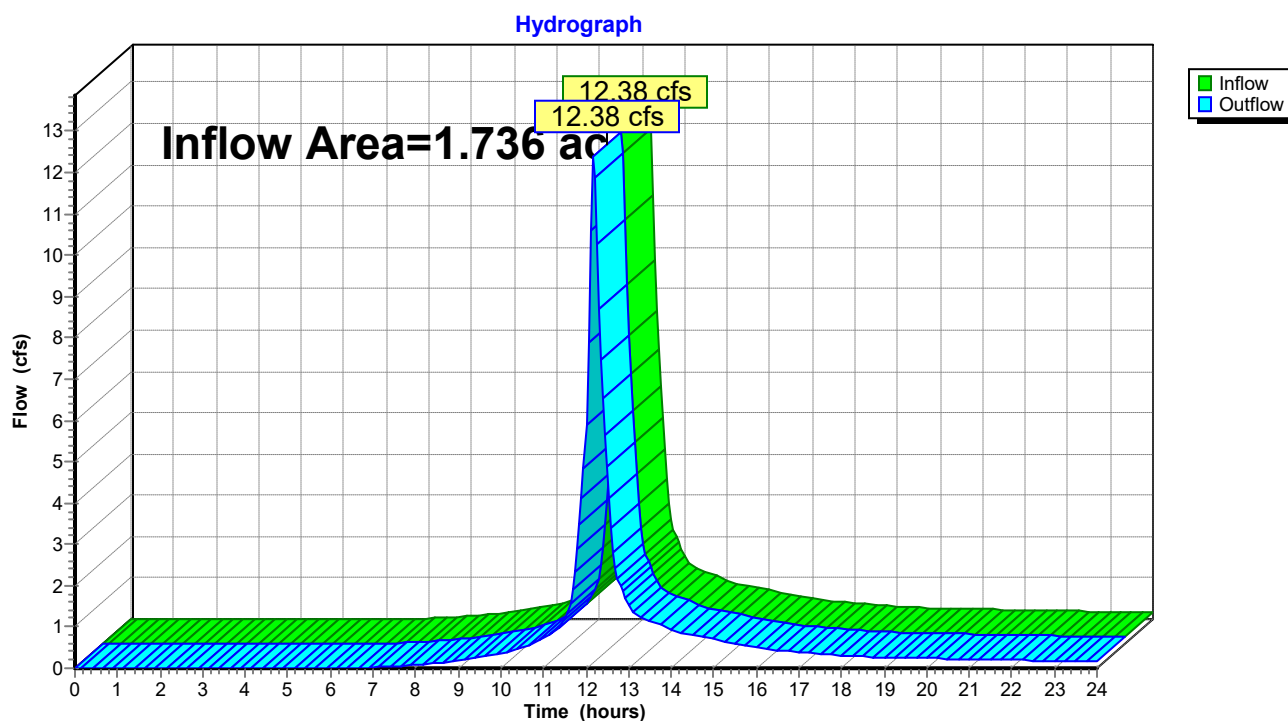
Summary for Reach DPE-1: Wetlands

[40] Hint: Not Described (Outflow=Inflow)

Inflow Area = 1.736 ac, 0.00% Impervious, Inflow Depth > 7.48" for 100 yr event
 Inflow = 12.38 cfs @ 12.17 hrs, Volume= 1.082 af
 Outflow = 12.38 cfs @ 12.17 hrs, Volume= 1.082 af, Atten= 0%, Lag= 0.0 min

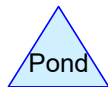
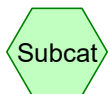
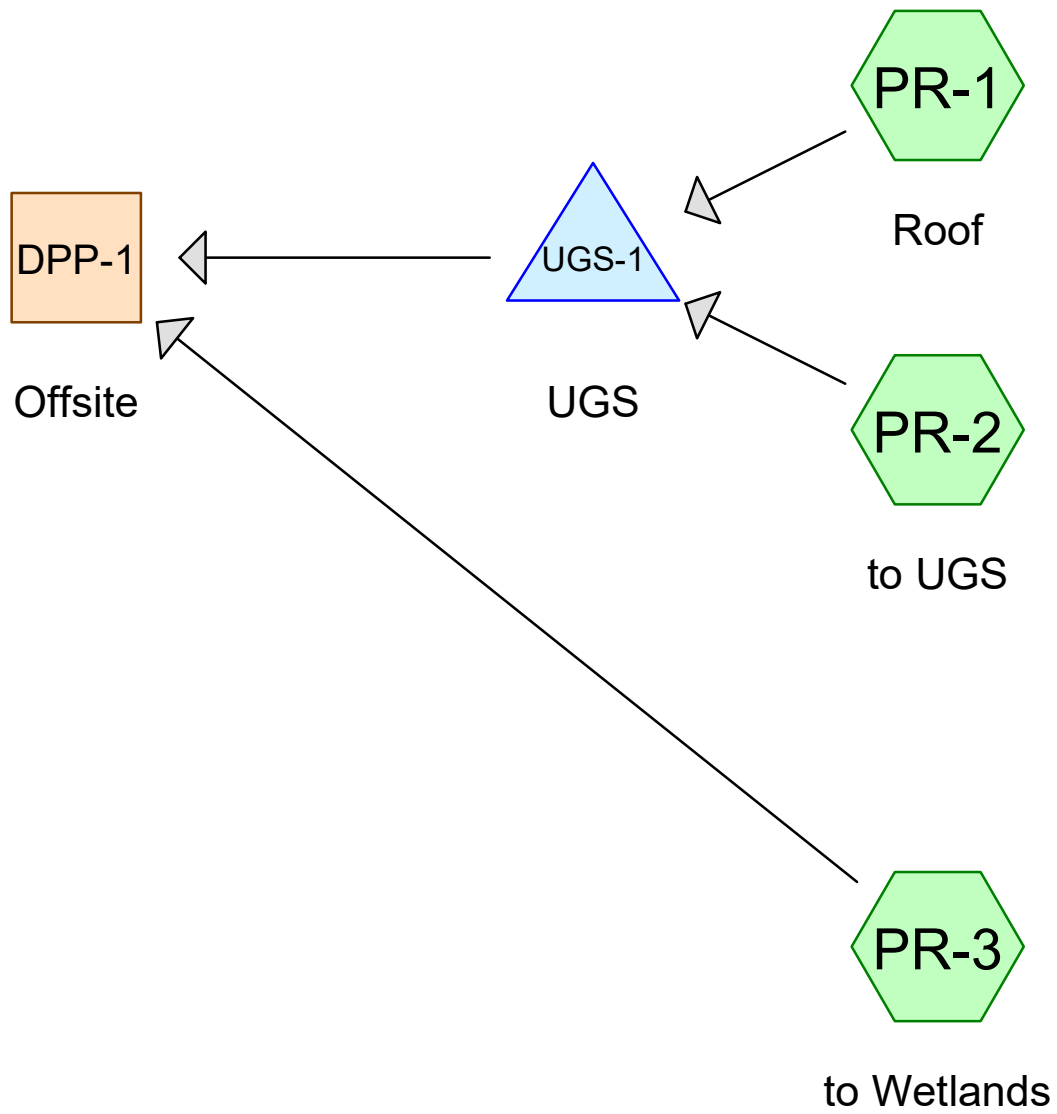
Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs

Reach DPE-1: Wetlands



APPENDIX E: PROPOSED CONDITIONS HYDROLOGIC ANALYSIS

- PROPOSED CONDITIONS DRAINAGE MAP
- PROPOSED CONDITIONS HYDROCAD CALCULATIONS



Routing Diagram for MAA250027 Model

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Rainfall Events Listing (selected events)

Event#	Event Name	Storm Type	Curve	Mode	Duration (hours)	B/B	Depth (inches)	AMC
1	2 yr	Type III 24-hr		Default	24.00	1	4.00	2
2	10 yr	Type III 24-hr		Default	24.00	1	6.37	2
3	25 yr	Type III 24-hr		Default	24.00	1	8.25	2
4	100 yr	Type III 24-hr		Default	24.00	1	11.40	2

MAA250027 Model

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Area Listing (selected nodes)

Area (acres)	CN	Description (subcatchment-numbers)
0.569	74	>75% Grass cover, Good, HSG C (PR-2, PR-3)
0.508	98	Paved parking, HSG A (PR-2)
0.126	98	Unconnected roofs, HSG A (PR-1)
0.533	70	Woods, Good, HSG C (PR-3)
1.736	82	TOTAL AREA

MAA250027 Model

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Soil Listing (selected nodes)

Area (acres)	Soil Group	Subcatchment Numbers
0.634	HSG A	PR-1, PR-2
0.000	HSG B	
1.102	HSG C	PR-2, PR-3
0.000	HSG D	
0.000	Other	
1.736		TOTAL AREA

MAA250027 Model

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Ground Covers (selected nodes)

HSG-A (acres)	HSG-B (acres)	HSG-C (acres)	HSG-D (acres)	Other (acres)	Total (acres)	Ground Cover	Subcatchment Numbers
0.000	0.000	0.569	0.000	0.000	0.569	>75% Grass cover, Good	PR-2, PR-3
0.508	0.000	0.000	0.000	0.000	0.508	Paved parking	PR-2
0.126	0.000	0.000	0.000	0.000	0.126	Unconnected roofs	PR-1
0.000	0.000	0.533	0.000	0.000	0.533	Woods, Good	PR-3
0.634	0.000	1.102	0.000	0.000	1.736	TOTAL AREA	

MAA250027 Model*Type III 24-hr 2 yr Rainfall=4.00"*

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Time span=0.00-24.00 hrs, dt=0.05 hrs, 481 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

Subcatchment PR-1: Roof

Runoff Area=5,500 sf 100.00% Impervious Runoff Depth>3.76"
Tc=6.0 min CN=98 Runoff=0.48 cfs 0.040 af

Subcatchment PR-2: to UGS

Runoff Area=24,437 sf 90.49% Impervious Runoff Depth>3.54"
Tc=6.0 min CN=96 Runoff=2.09 cfs 0.165 af

Subcatchment PR-3: to Wetlands

Runoff Area=45,664 sf 0.00% Impervious Runoff Depth>1.46"
Flow Length=461' Tc=8.0 min CN=72 Runoff=1.59 cfs 0.127 af

Reach DPP-1: Offsite

Inflow=1.59 cfs 0.205 af
Outflow=1.59 cfs 0.205 af

Pond UGS-1: UGS

Peak Elev=203.44' Storage=5,001 cf Inflow=2.57 cfs 0.205 af
Discarded=0.02 cfs 0.028 af Primary=0.46 cfs 0.078 af Outflow=0.47 cfs 0.106 af

Total Runoff Area = 1.736 ac Runoff Volume = 0.332 af Average Runoff Depth = 2.30"
63.47% Pervious = 1.102 ac 36.53% Impervious = 0.634 ac

MAA250027 Model

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Type III 24-hr 2 yr Rainfall=4.00"

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Summary for Subcatchment PR-1: Roof

Runoff = 0.48 cfs @ 12.09 hrs, Volume= 0.040 af, Depth> 3.76"
Routed to Pond UGS-1 : UGS

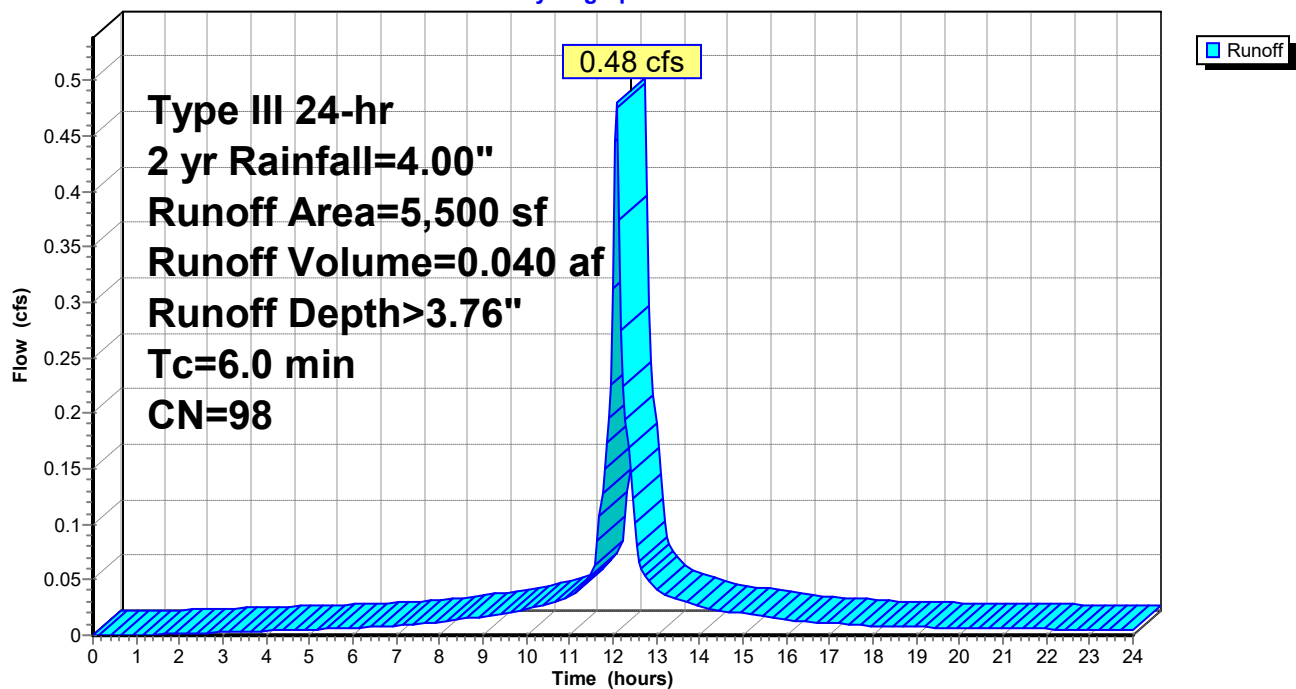
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs
Type III 24-hr 2 yr Rainfall=4.00"

Area (sf)	CN	Description
5,500	98	Unconnected roofs, HSG A
5,500		100.00% Impervious Area
5,500		100.00% Unconnected

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, Direct

Subcatchment PR-1: Roof

Hydrograph



MAA250027 Model

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Type III 24-hr 2 yr Rainfall=4.00"

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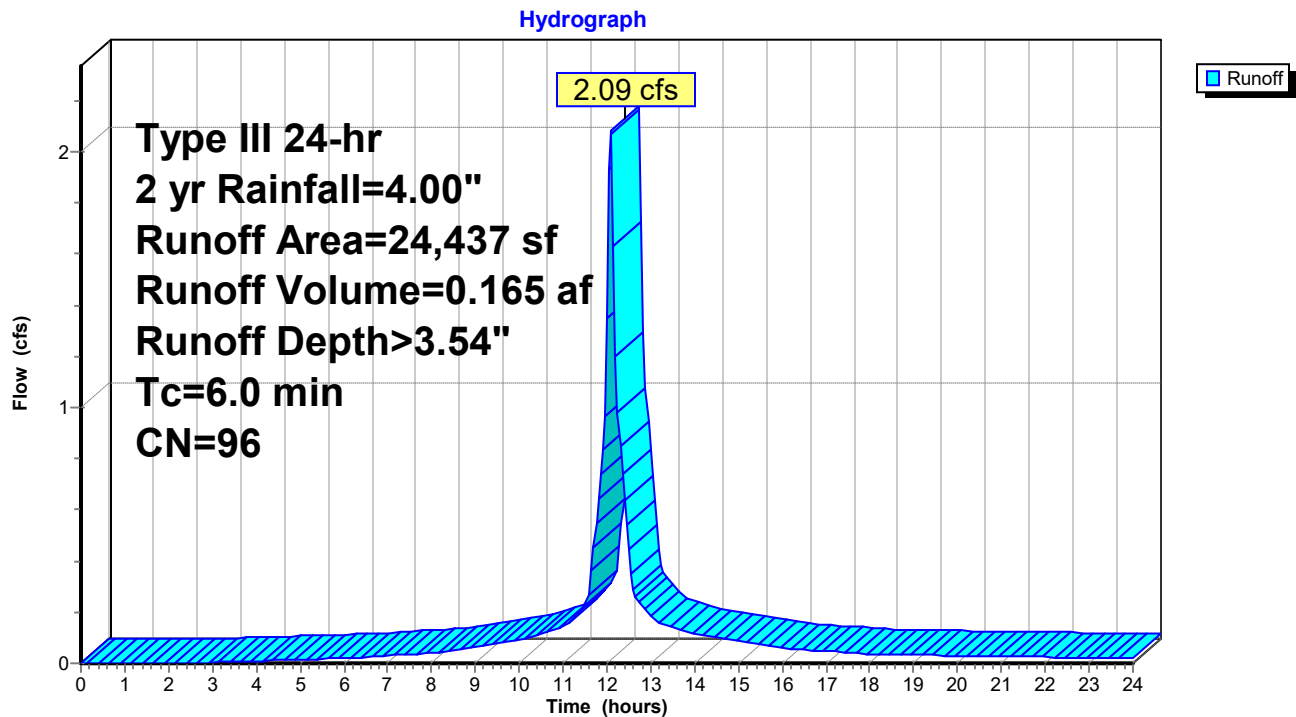
Summary for Subcatchment PR-2: to UGS

Runoff = 2.09 cfs @ 12.09 hrs, Volume= 0.165 af, Depth> 3.54"
Routed to Pond UGS-1 : UGS

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs
Type III 24-hr 2 yr Rainfall=4.00"

Area (sf)	CN	Description
2,323	74	>75% Grass cover, Good, HSG C
22,114	98	Paved parking, HSG A
24,437	96	Weighted Average
2,323		9.51% Pervious Area
22,114		90.49% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, Direct

Subcatchment PR-2: to UGS

MAA250027 Model

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Type III 24-hr 2 yr Rainfall=4.00"

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Summary for Subcatchment PR-3: to Wetlands

Runoff = 1.59 cfs @ 12.12 hrs, Volume= 0.127 af, Depth> 1.46"
 Routed to Reach DPP-1 : Offsite

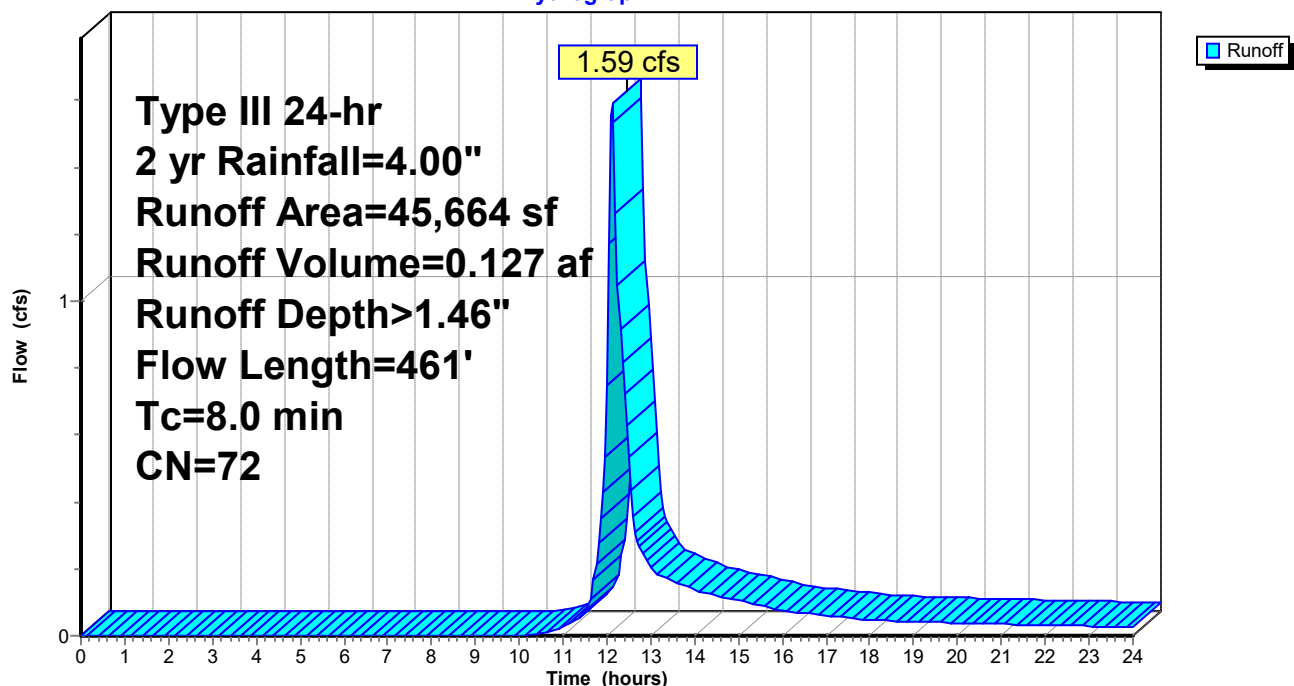
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs
 Type III 24-hr 2 yr Rainfall=4.00"

Area (sf)	CN	Description
22,467	74	>75% Grass cover, Good, HSG C
23,197	70	Woods, Good, HSG C
45,664	72	Weighted Average
45,664		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
0.9	12	0.2700	0.22		Sheet Flow, A to B
					Grass: Dense n= 0.240 P2= 3.28"
3.5	38	0.0920	0.18		Sheet Flow, B to C
					Grass: Dense n= 0.240 P2= 3.28"
3.4	388	0.0720	1.88		Shallow Concentrated Flow, C to D
					Short Grass Pasture Kv= 7.0 fps
0.2	23	0.1000	1.58		Shallow Concentrated Flow, C to D
					Woodland Kv= 5.0 fps
8.0	461	Total			

Subcatchment PR-3: to Wetlands

Hydrograph



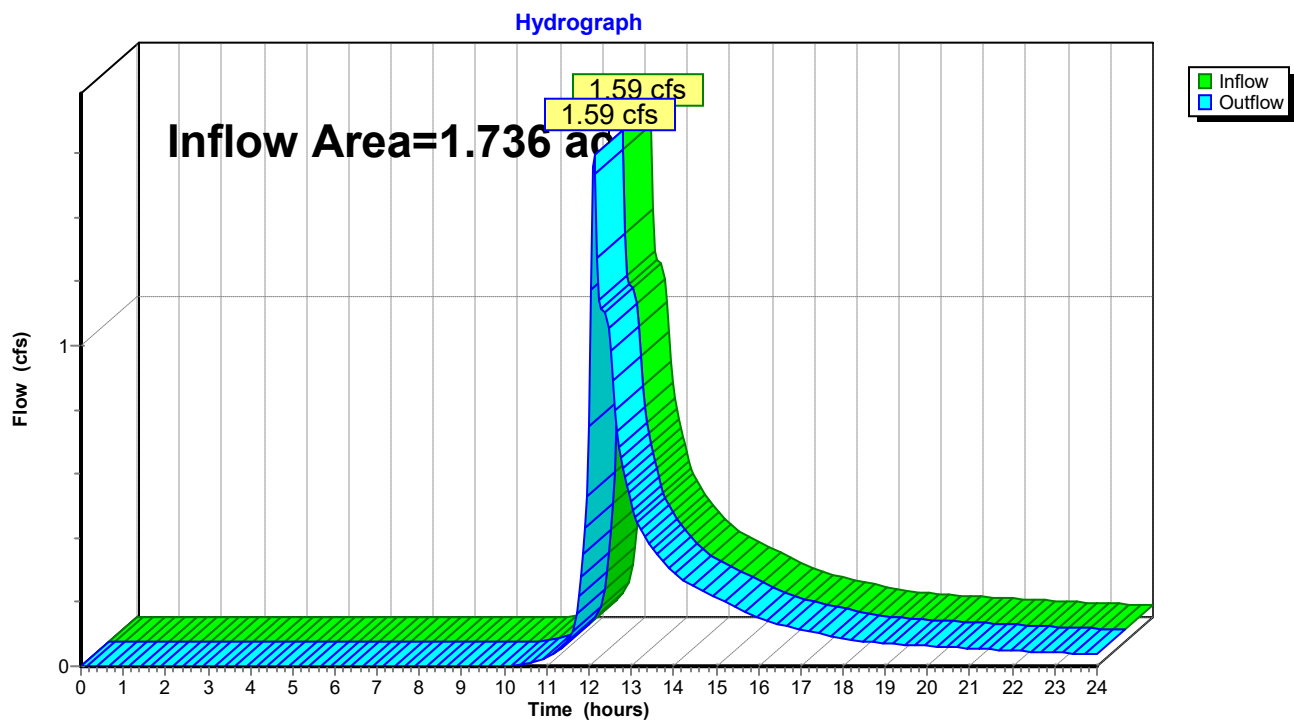
Summary for Reach DPP-1: Offsite

[40] Hint: Not Described (Outflow=Inflow)

Inflow Area = 1.736 ac, 36.53% Impervious, Inflow Depth > 1.42" for 2 yr event
 Inflow = 1.59 cfs @ 12.12 hrs, Volume= 0.205 af
 Outflow = 1.59 cfs @ 12.12 hrs, Volume= 0.205 af, Atten= 0%, Lag= 0.0 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs

Reach DPP-1: Offsite



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Type III 24-hr 2 yr Rainfall=4.00"

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Summary for Pond UGS-1: UGS

Inflow Area = 0.687 ac, 92.24% Impervious, Inflow Depth > 3.58" for 2 yr event
 Inflow = 2.57 cfs @ 12.09 hrs, Volume= 0.205 af
 Outflow = 0.47 cfs @ 12.54 hrs, Volume= 0.106 af, Atten= 82%, Lag= 27.0 min
 Discarded = 0.02 cfs @ 6.90 hrs, Volume= 0.028 af
 Primary = 0.46 cfs @ 12.54 hrs, Volume= 0.078 af
 Routed to Reach DPP-1 : Offsite

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs
 Peak Elev= 203.44' @ 12.54 hrs Surf.Area= 2,622 sf Storage= 5,001 cf

Plug-Flow detention time= 227.0 min calculated for 0.106 af (52% of inflow)
 Center-of-Mass det. time= 109.4 min (873.8 - 764.4)

Volume	Invert	Avail.Storage	Storage Description
#1A	200.75'	4,300 cf	28.50'W x 91.99'L x 6.75'H Field A 17,697 cf Overall - 6,946 cf Embedded = 10,751 cf x 40.0% Voids
#2A	201.50'	6,946 cf	ADS_StormTech MC-4500 b +Cap x 63 Inside #1 Effective Size= 90.4"W x 60.0"H => 26.46 sf x 4.03'L = 106.5 cf Overall Size= 100.0"W x 60.0"H x 4.33'L with 0.31' Overlap 63 Chambers in 3 Rows Cap Storage= 39.5 cf x 2 x 3 rows = 237.0 cf
		11,246 cf	Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Discarded	200.75'	0.270 in/hr Exfiltration over Surface area
#2	Primary	203.05'	8.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads
#3	Primary	207.25'	5.0' long x 1.0' breadth Broad-Crested Rectangular Weir Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 3.00 Coef. (English) 2.69 2.72 2.75 2.85 2.98 3.08 3.20 3.28 3.31 3.30 3.31 3.32

Discarded OutFlow Max=0.02 cfs @ 6.90 hrs HW=200.82' (Free Discharge)↑ **1=Exfiltration** (Exfiltration Controls 0.02 cfs)**Primary OutFlow** Max=0.45 cfs @ 12.54 hrs HW=203.44' TW=0.00' (Dynamic Tailwater)↑ **2=Orifice/Grate** (Orifice Controls 0.45 cfs @ 2.13 fps)↑ **3=Broad-Crested Rectangular Weir** (Controls 0.00 cfs)

Pond UGS-1: UGS - Chamber Wizard Field A**Chamber Model = ADS_StormTech MC-4500 b +Cap (ADS StormTech® MC-4500 with cap volume)**

Effective Size= 90.4"W x 60.0"H => 26.46 sf x 4.03'L = 106.5 cf

Overall Size= 100.0"W x 60.0"H x 4.33'L with 0.31' Overlap

Cap Storage= 39.5 cf x 2 x 3 rows = 237.0 cf

100.0" Wide + 9.0" Spacing = 109.0" C-C Row Spacing

21 Chambers/Row x 4.02' Long +2.73' Cap Length x 2 = 89.99' Row Length +12.0" End Stone x 2 = 91.99'
Base Length

3 Rows x 100.0" Wide + 9.0" Spacing x 2 + 12.0" Side Stone x 2 = 28.50' Base Width

9.0" Stone Base + 60.0" Chamber Height + 12.0" Stone Cover = 6.75' Field Height

63 Chambers x 106.5 cf + 39.5 cf Cap Volume x 2 x 3 Rows = 6,945.9 cf Chamber Storage

17,696.9 cf Field - 6,945.9 cf Chambers = 10,751.0 cf Stone x 40.0% Voids = 4,300.4 cf Stone Storage

Chamber Storage + Stone Storage = 11,246.3 cf = 0.258 af

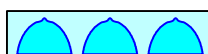
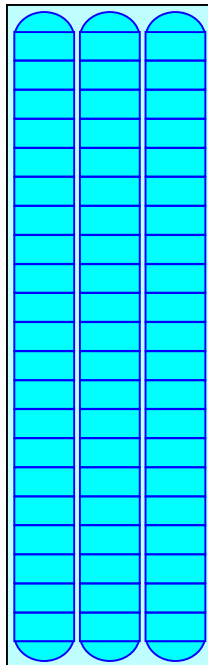
Overall Storage Efficiency = 63.5%

Overall System Size = 91.99' x 28.50' x 6.75'

63 Chambers

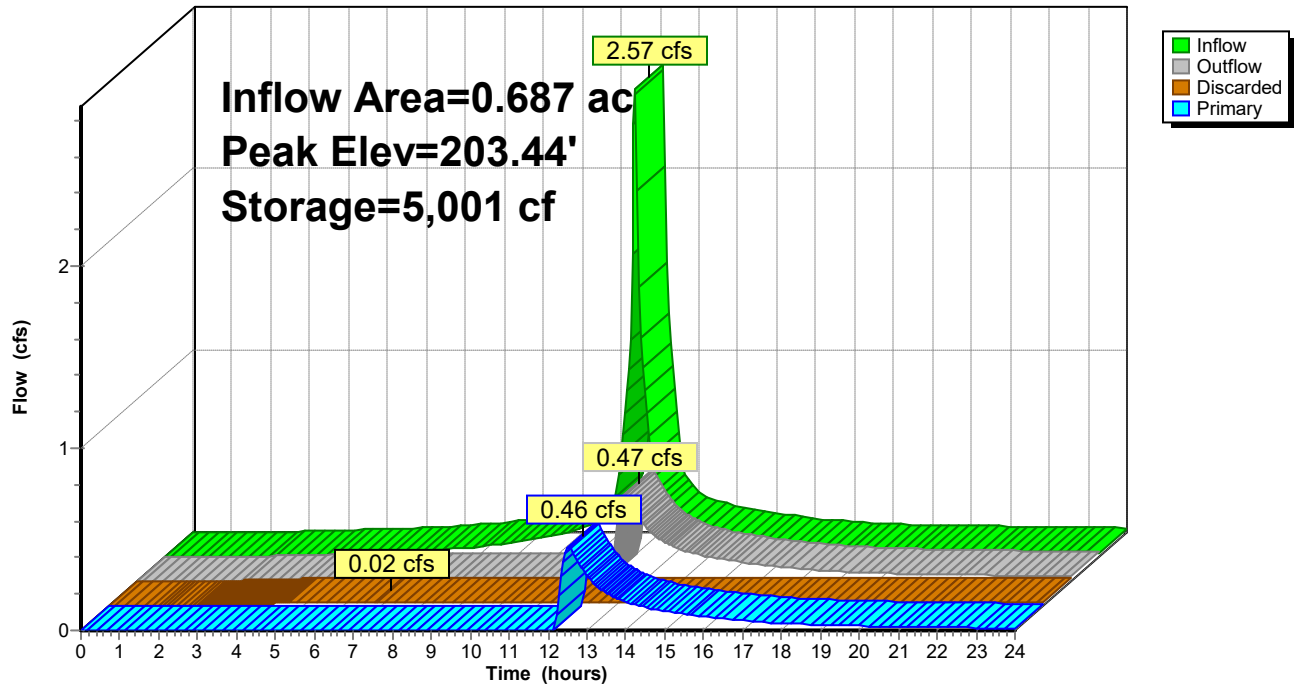
655.4 cy Field

398.2 cy Stone



Pond UGS-1: UGS

Hydrograph



MAA250027 Model*Type III 24-hr 10 yr Rainfall=6.37"*

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Time span=0.00-24.00 hrs, dt=0.05 hrs, 481 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

Subcatchment PR-1: Roof

Runoff Area=5,500 sf 100.00% Impervious Runoff Depth>6.13"
Tc=6.0 min CN=98 Runoff=0.77 cfs 0.064 af

Subcatchment PR-2: to UGS

Runoff Area=24,437 sf 90.49% Impervious Runoff Depth>5.89"
Tc=6.0 min CN=96 Runoff=3.39 cfs 0.275 af

Subcatchment PR-3: to Wetlands

Runoff Area=45,664 sf 0.00% Impervious Runoff Depth>3.29"
Flow Length=461' Tc=8.0 min CN=72 Runoff=3.73 cfs 0.288 af

Reach DPP-1: Offsite

Inflow=5.05 cfs 0.498 af
Outflow=5.05 cfs 0.498 af

Pond UGS-1: UGS

Peak Elev=204.34' Storage=6,797 cf Inflow=4.16 cfs 0.340 af
Discarded=0.02 cfs 0.030 af Primary=1.64 cfs 0.210 af Outflow=1.66 cfs 0.240 af

Total Runoff Area = 1.736 ac Runoff Volume = 0.628 af Average Runoff Depth = 4.34"
63.47% Pervious = 1.102 ac 36.53% Impervious = 0.634 ac

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Type III 24-hr 10 yr Rainfall=6.37"

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Summary for Subcatchment PR-1: Roof

Runoff = 0.77 cfs @ 12.09 hrs, Volume= 0.064 af, Depth> 6.13"
Routed to Pond UGS-1 : UGS

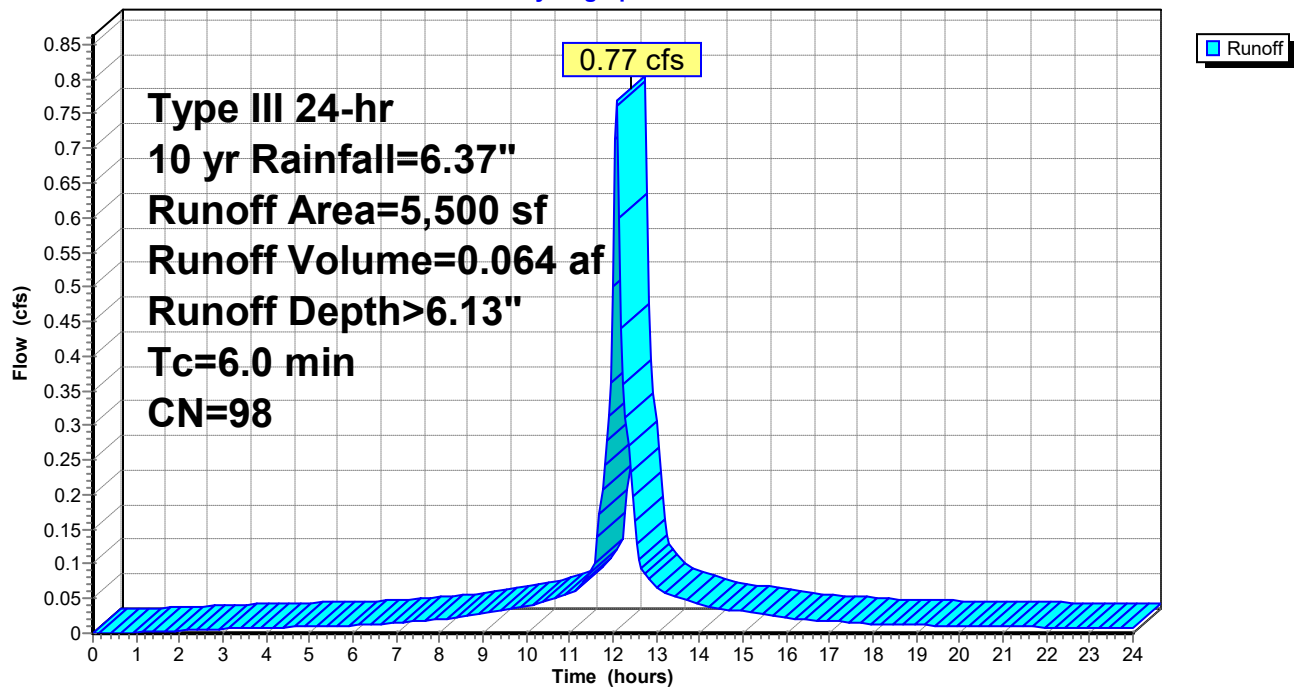
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs
Type III 24-hr 10 yr Rainfall=6.37"

Area (sf)	CN	Description
5,500	98	Unconnected roofs, HSG A
5,500		100.00% Impervious Area
5,500		100.00% Unconnected

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, Direct

Subcatchment PR-1: Roof

Hydrograph



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Type III 24-hr 10 yr Rainfall=6.37"

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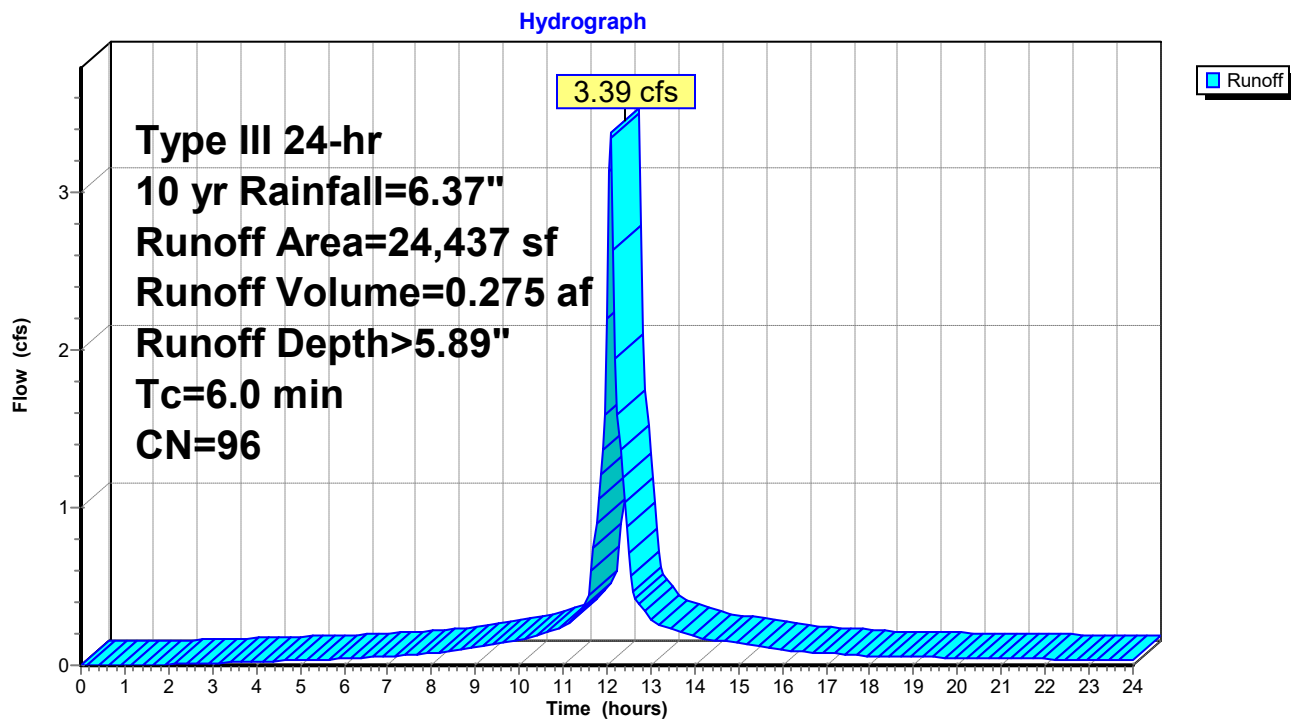
Summary for Subcatchment PR-2: to UGS

Runoff = 3.39 cfs @ 12.09 hrs, Volume= 0.275 af, Depth> 5.89"
Routed to Pond UGS-1 : UGS

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs
Type III 24-hr 10 yr Rainfall=6.37"

Area (sf)	CN	Description
2,323	74	>75% Grass cover, Good, HSG C
22,114	98	Paved parking, HSG A
24,437	96	Weighted Average
2,323		9.51% Pervious Area
22,114		90.49% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, Direct

Subcatchment PR-2: to UGS

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Type III 24-hr 10 yr Rainfall=6.37"

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Summary for Subcatchment PR-3: to Wetlands

Runoff = 3.73 cfs @ 12.12 hrs, Volume= 0.288 af, Depth> 3.29"
 Routed to Reach DPP-1 : Offsite

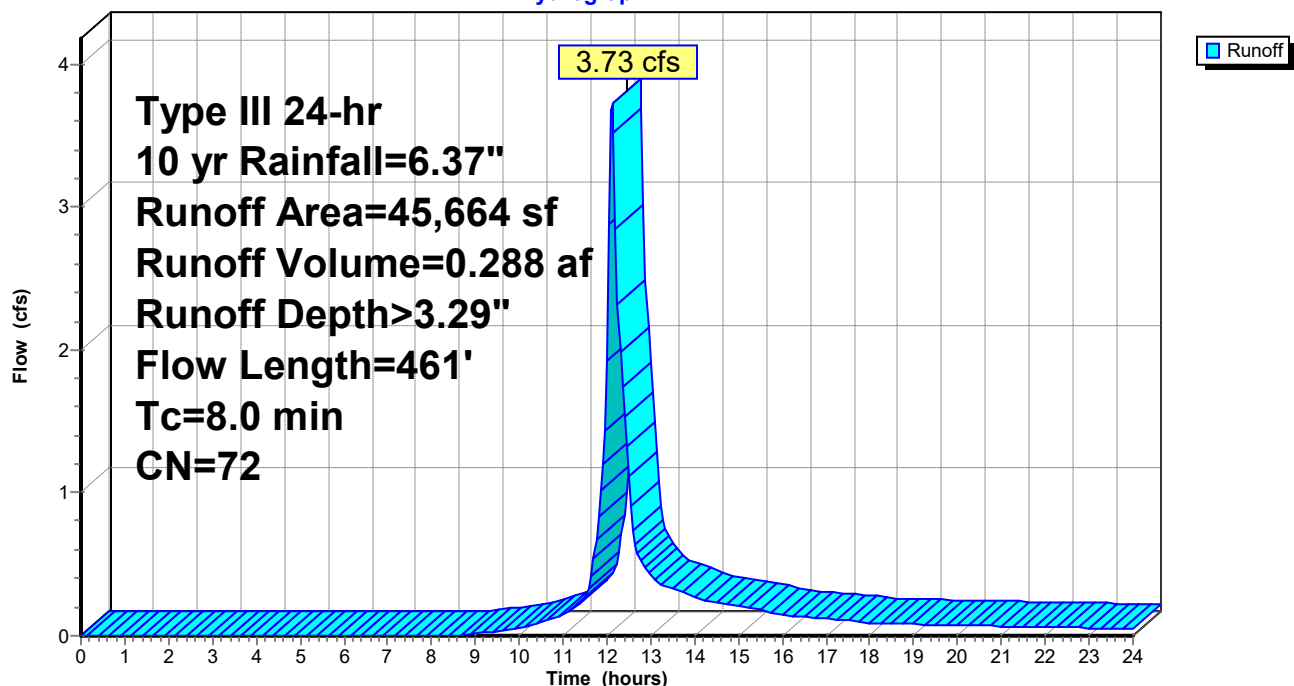
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs
 Type III 24-hr 10 yr Rainfall=6.37"

Area (sf)	CN	Description
22,467	74	>75% Grass cover, Good, HSG C
23,197	70	Woods, Good, HSG C
45,664	72	Weighted Average
45,664		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
0.9	12	0.2700	0.22		Sheet Flow, A to B Grass: Dense n= 0.240 P2= 3.28"
3.5	38	0.0920	0.18		Sheet Flow, B to C Grass: Dense n= 0.240 P2= 3.28"
3.4	388	0.0720	1.88		Shallow Concentrated Flow, C to D Short Grass Pasture Kv= 7.0 fps
0.2	23	0.1000	1.58		Shallow Concentrated Flow, C to D Woodland Kv= 5.0 fps
8.0	461	Total			

Subcatchment PR-3: to Wetlands

Hydrograph



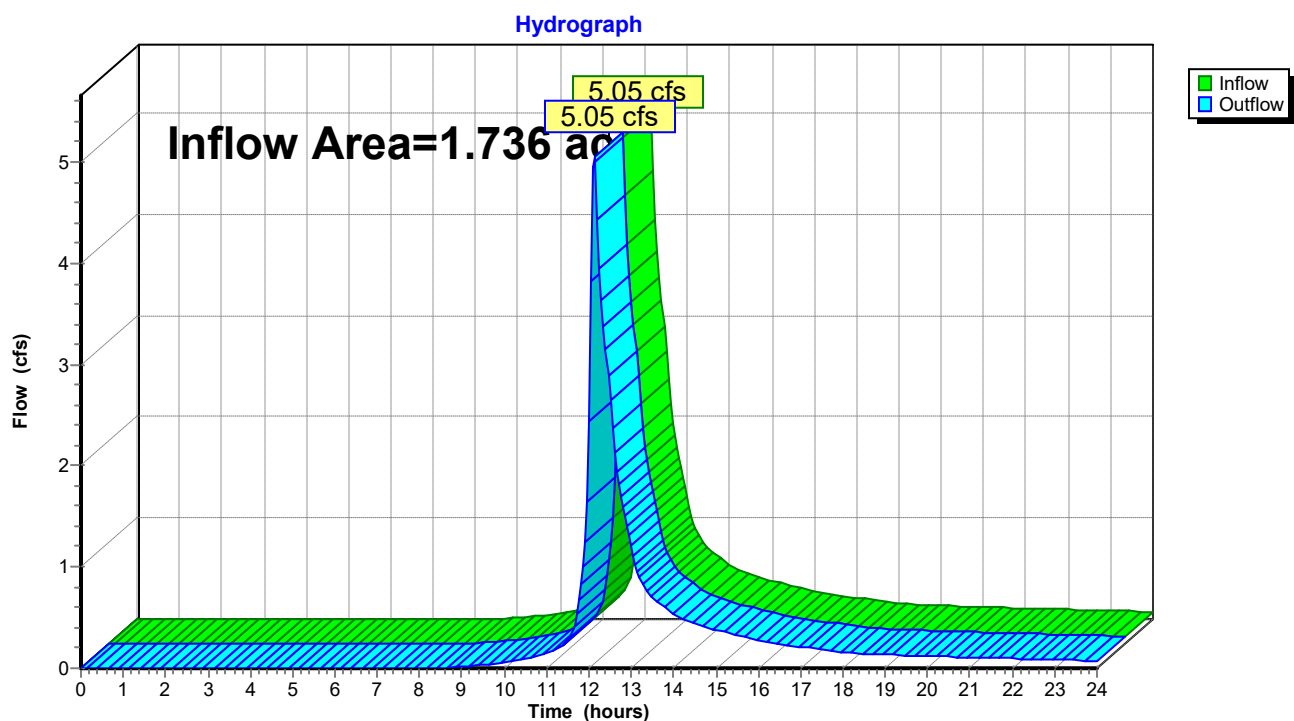
Summary for Reach DPP-1: Offsite

[40] Hint: Not Described (Outflow=Inflow)

Inflow Area = 1.736 ac, 36.53% Impervious, Inflow Depth > 3.44" for 10 yr event
 Inflow = 5.05 cfs @ 12.13 hrs, Volume= 0.498 af
 Outflow = 5.05 cfs @ 12.13 hrs, Volume= 0.498 af, Atten= 0%, Lag= 0.0 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs

Reach DPP-1: Offsite



Summary for Pond UGS-1: UGS

Inflow Area = 0.687 ac, 92.24% Impervious, Inflow Depth > 5.94" for 10 yr event
 Inflow = 4.16 cfs @ 12.09 hrs, Volume= 0.340 af
 Outflow = 1.66 cfs @ 12.31 hrs, Volume= 0.240 af, Atten= 60%, Lag= 13.3 min
 Discarded = 0.02 cfs @ 4.50 hrs, Volume= 0.030 af
 Primary = 1.64 cfs @ 12.31 hrs, Volume= 0.210 af
 Routed to Reach DPP-1 : Offsite

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs
 Peak Elev= 204.34' @ 12.31 hrs Surf.Area= 2,622 sf Storage= 6,797 cf

Plug-Flow detention time= 176.1 min calculated for 0.240 af (71% of inflow)
 Center-of-Mass det. time= 83.1 min (837.2 - 754.1)

Volume	Invert	Avail.Storage	Storage Description
#1A	200.75'	4,300 cf	28.50'W x 91.99'L x 6.75'H Field A 17,697 cf Overall - 6,946 cf Embedded = 10,751 cf x 40.0% Voids
#2A	201.50'	6,946 cf	ADS_StormTech MC-4500 b +Cap x 63 Inside #1 Effective Size= 90.4"W x 60.0"H => 26.46 sf x 4.03'L = 106.5 cf Overall Size= 100.0"W x 60.0"H x 4.33'L with 0.31' Overlap 63 Chambers in 3 Rows Cap Storage= 39.5 cf x 2 x 3 rows = 237.0 cf
		11,246 cf	Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Discarded	200.75'	0.270 in/hr Exfiltration over Surface area
#2	Primary	203.05'	8.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads
#3	Primary	207.25'	5.0' long x 1.0' breadth Broad-Crested Rectangular Weir Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 3.00 Coef. (English) 2.69 2.72 2.75 2.85 2.98 3.08 3.20 3.28 3.31 3.30 3.31 3.32

Discarded OutFlow Max=0.02 cfs @ 4.50 hrs HW=200.82' (Free Discharge)

↑ **1=Exfiltration** (Exfiltration Controls 0.02 cfs)

Primary OutFlow Max=1.64 cfs @ 12.31 hrs HW=204.33' TW=0.00' (Dynamic Tailwater)

↑ **2=Orifice/Grate** (Orifice Controls 1.64 cfs @ 4.70 fps)

↑ **3=Broad-Crested Rectangular Weir** (Controls 0.00 cfs)

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Type III 24-hr 10 yr Rainfall=6.37"

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Pond UGS-1: UGS - Chamber Wizard Field A

Chamber Model = ADS_StormTech MC-4500 b +Cap (ADS StormTech® MC-4500 with cap volume)

Effective Size= 90.4"W x 60.0"H => 26.46 sf x 4.03'L = 106.5 cf

Overall Size= 100.0"W x 60.0"H x 4.33'L with 0.31' Overlap

Cap Storage= 39.5 cf x 2 x 3 rows = 237.0 cf

100.0" Wide + 9.0" Spacing = 109.0" C-C Row Spacing

21 Chambers/Row x 4.02' Long +2.73' Cap Length x 2 = 89.99' Row Length +12.0" End Stone x 2 = 91.99' Base Length

3 Rows x 100.0" Wide + 9.0" Spacing x 2 + 12.0" Side Stone x 2 = 28.50' Base Width

9.0" Stone Base + 60.0" Chamber Height + 12.0" Stone Cover = 6.75' Field Height

63 Chambers x 106.5 cf + 39.5 cf Cap Volume x 2 x 3 Rows = 6,945.9 cf Chamber Storage

17,696.9 cf Field - 6,945.9 cf Chambers = 10,751.0 cf Stone x 40.0% Voids = 4,300.4 cf Stone Storage

Chamber Storage + Stone Storage = 11,246.3 cf = 0.258 af

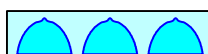
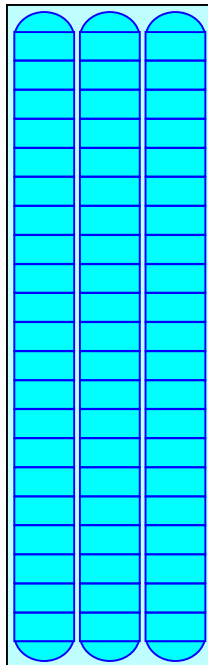
Overall Storage Efficiency = 63.5%

Overall System Size = 91.99' x 28.50' x 6.75'

63 Chambers

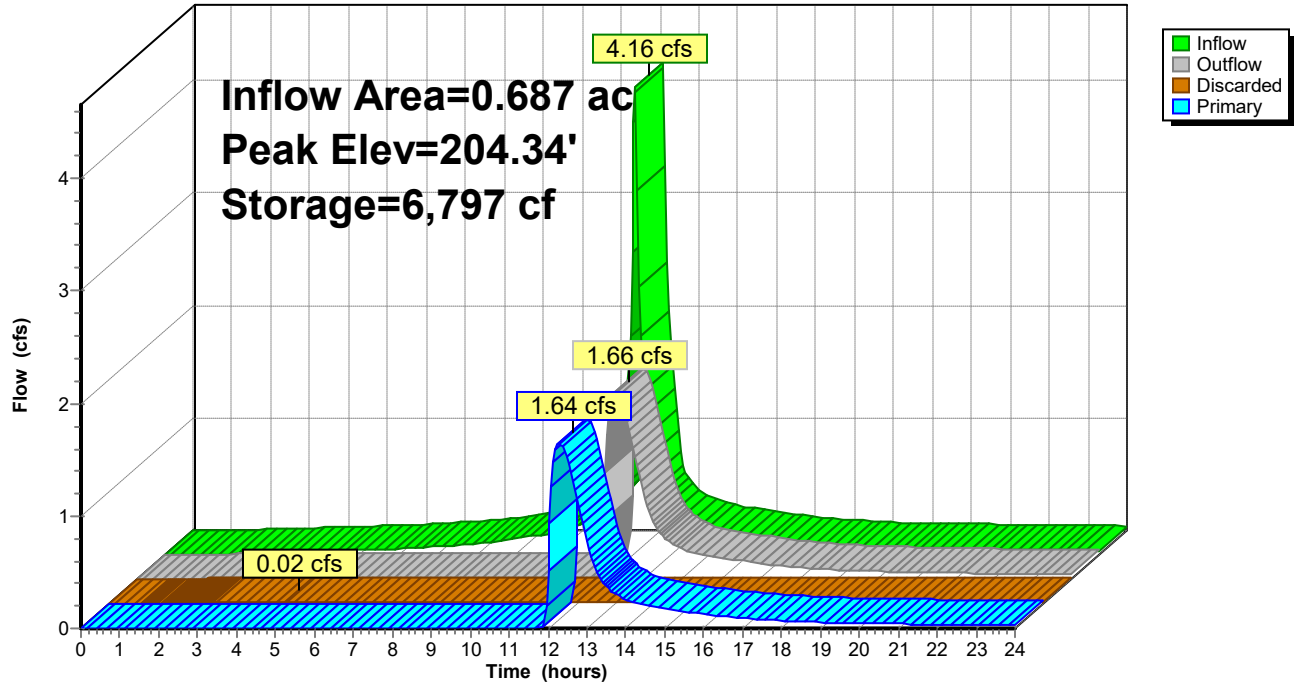
655.4 cy Field

398.2 cy Stone



Pond UGS-1: UGS

Hydrograph



MAA250027 Model*Type III 24-hr 25 yr Rainfall=8.25"*

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Time span=0.00-24.00 hrs, dt=0.05 hrs, 481 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

Subcatchment PR-1: Roof

Runoff Area=5,500 sf 100.00% Impervious Runoff Depth>8.01"
Tc=6.0 min CN=98 Runoff=1.00 cfs 0.084 af

Subcatchment PR-2: to UGS

Runoff Area=24,437 sf 90.49% Impervious Runoff Depth>7.77"
Tc=6.0 min CN=96 Runoff=4.41 cfs 0.363 af

Subcatchment PR-3: to Wetlands

Runoff Area=45,664 sf 0.00% Impervious Runoff Depth>4.91"
Flow Length=461' Tc=8.0 min CN=72 Runoff=5.56 cfs 0.429 af

Reach DPP-1: Offsite

Inflow=7.48 cfs 0.744 af
Outflow=7.48 cfs 0.744 af

Pond UGS-1: UGS

Peak Elev=205.09' Storage=8,188 cf Inflow=5.41 cfs 0.447 af
Discarded=0.02 cfs 0.031 af Primary=2.20 cfs 0.316 af Outflow=2.21 cfs 0.346 af

Total Runoff Area = 1.736 ac Runoff Volume = 0.876 af Average Runoff Depth = 6.06"
63.47% Pervious = 1.102 ac 36.53% Impervious = 0.634 ac

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Type III 24-hr 25 yr Rainfall=8.25"

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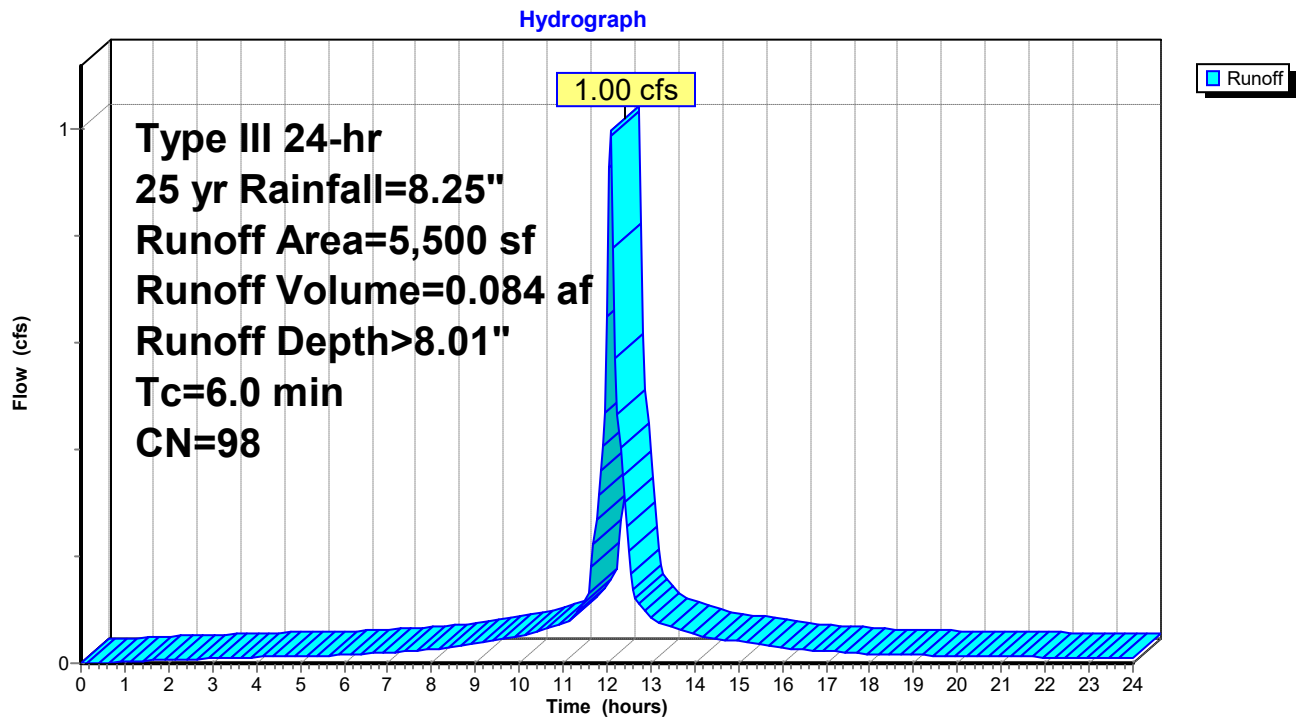
Summary for Subcatchment PR-1: Roof

Runoff = 1.00 cfs @ 12.09 hrs, Volume= 0.084 af, Depth> 8.01"
Routed to Pond UGS-1 : UGS

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs
Type III 24-hr 25 yr Rainfall=8.25"

Area (sf)	CN	Description
5,500	98	Unconnected roofs, HSG A
5,500		100.00% Impervious Area
5,500		100.00% Unconnected

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, Direct

Subcatchment PR-1: Roof

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Type III 24-hr 25 yr Rainfall=8.25"

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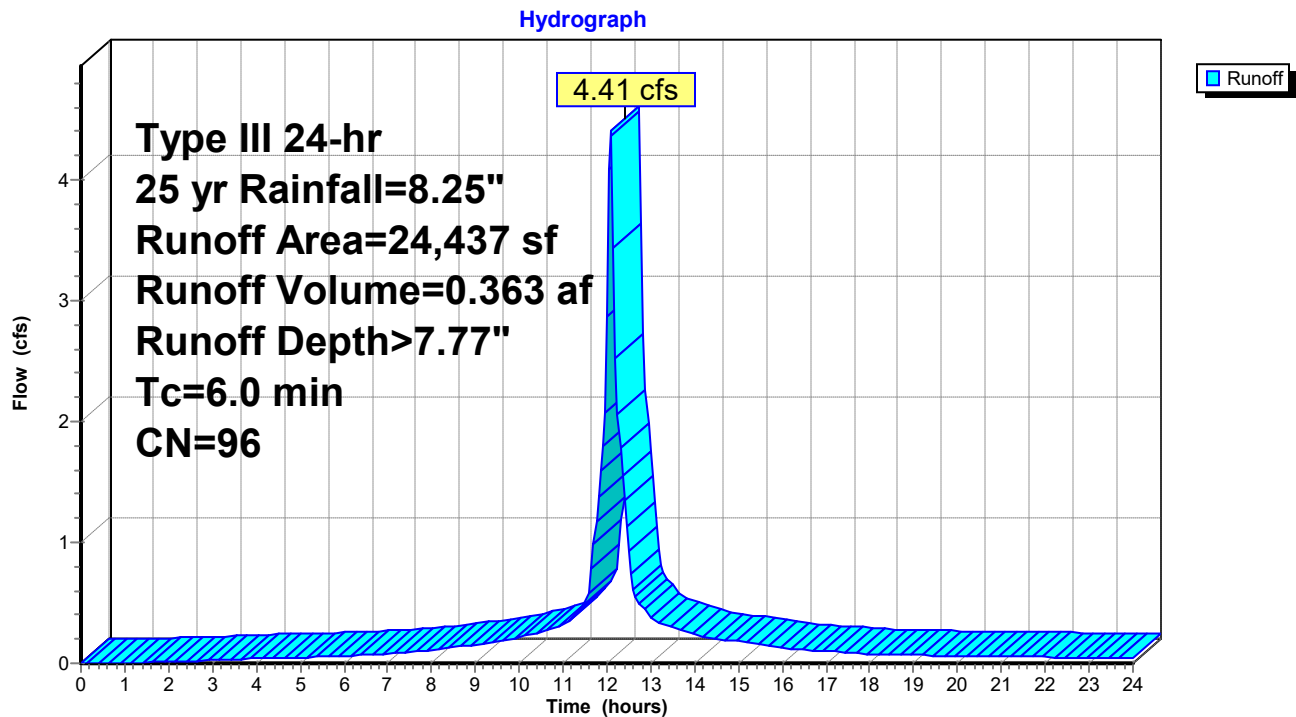
Summary for Subcatchment PR-2: to UGS

Runoff = 4.41 cfs @ 12.09 hrs, Volume= 0.363 af, Depth> 7.77"
Routed to Pond UGS-1 : UGS

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs
Type III 24-hr 25 yr Rainfall=8.25"

Area (sf)	CN	Description
2,323	74	>75% Grass cover, Good, HSG C
22,114	98	Paved parking, HSG A
24,437	96	Weighted Average
2,323		9.51% Pervious Area
22,114		90.49% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, Direct

Subcatchment PR-2: to UGS

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Type III 24-hr 25 yr Rainfall=8.25"

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Summary for Subcatchment PR-3: to Wetlands

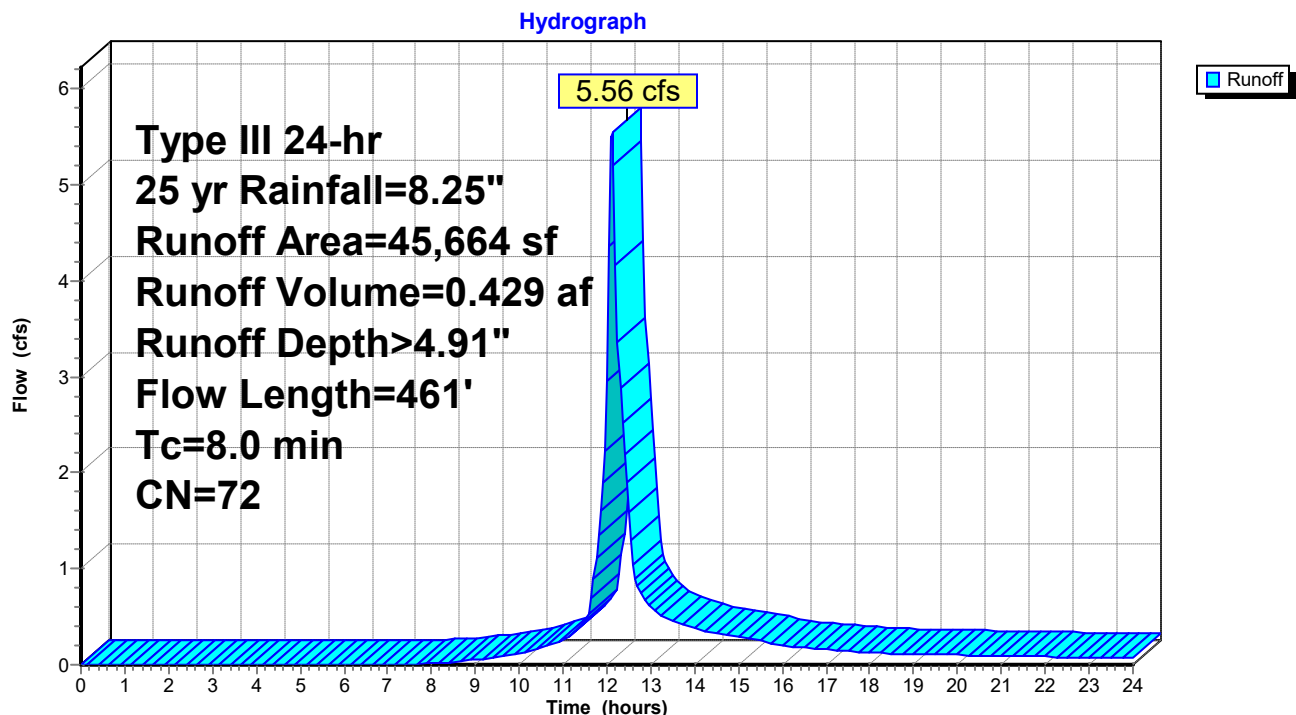
Runoff = 5.56 cfs @ 12.11 hrs, Volume= 0.429 af, Depth> 4.91"
 Routed to Reach DPP-1 : Offsite

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs
 Type III 24-hr 25 yr Rainfall=8.25"

Area (sf)	CN	Description
22,467	74	>75% Grass cover, Good, HSG C
23,197	70	Woods, Good, HSG C
45,664	72	Weighted Average
45,664		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
0.9	12	0.2700	0.22		Sheet Flow, A to B Grass: Dense n= 0.240 P2= 3.28"
3.5	38	0.0920	0.18		Sheet Flow, B to C Grass: Dense n= 0.240 P2= 3.28"
3.4	388	0.0720	1.88		Shallow Concentrated Flow, C to D Short Grass Pasture Kv= 7.0 fps
0.2	23	0.1000	1.58		Shallow Concentrated Flow, C to D Woodland Kv= 5.0 fps
8.0	461	Total			

Subcatchment PR-3: to Wetlands

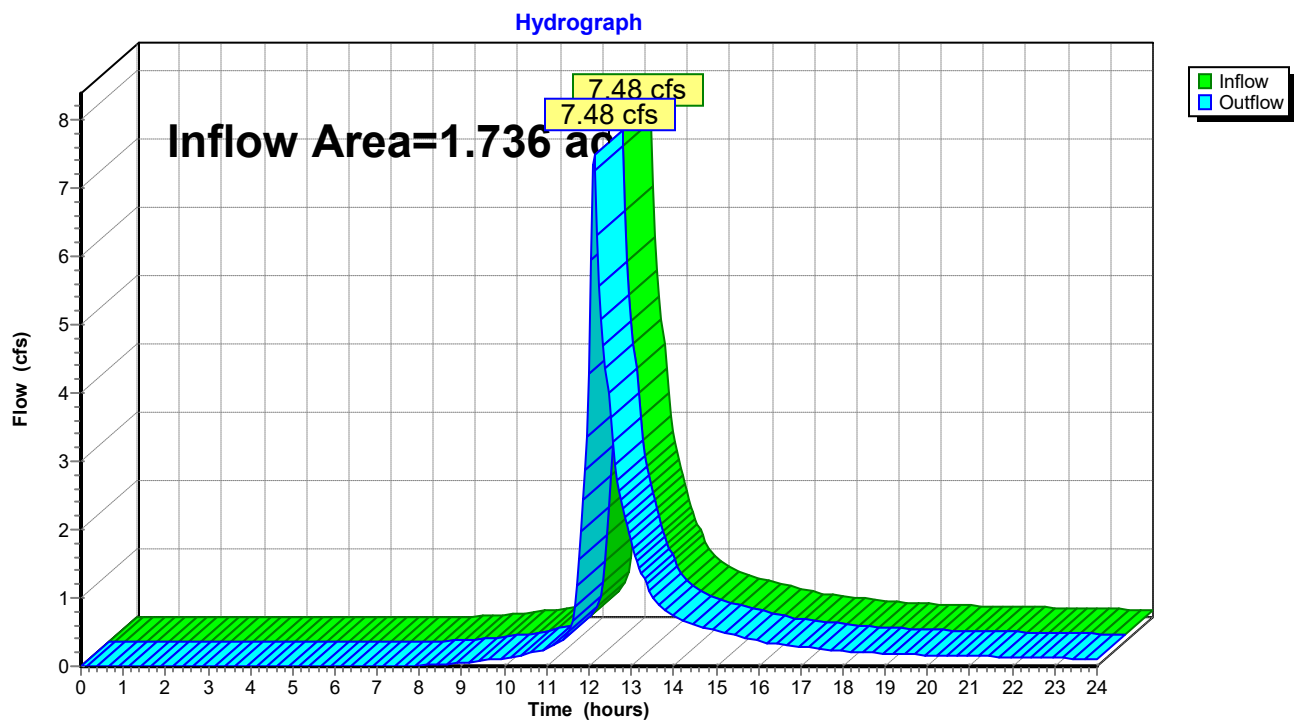


Summary for Reach DPP-1: Offsite

[40] Hint: Not Described (Outflow=Inflow)

Inflow Area = 1.736 ac, 36.53% Impervious, Inflow Depth > 5.15" for 25 yr event
Inflow = 7.48 cfs @ 12.12 hrs, Volume= 0.744 af
Outflow = 7.48 cfs @ 12.12 hrs, Volume= 0.744 af, Atten= 0%, Lag= 0.0 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs

Reach DPP-1: Offsite

MAA250027 Model

Prepared by Bohler Engineering, PC

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Type III 24-hr 25 yr Rainfall=8.25"

Printed 12/12/2025

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Summary for Pond UGS-1: UGS

Inflow Area = 0.687 ac, 92.24% Impervious, Inflow Depth > 7.81" for 25 yr event
 Inflow = 5.41 cfs @ 12.09 hrs, Volume= 0.447 af
 Outflow = 2.21 cfs @ 12.30 hrs, Volume= 0.346 af, Atten= 59%, Lag= 12.7 min
 Discarded = 0.02 cfs @ 3.45 hrs, Volume= 0.031 af
 Primary = 2.20 cfs @ 12.30 hrs, Volume= 0.316 af
 Routed to Reach DPP-1 : Offsite

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs
 Peak Elev= 205.09' @ 12.30 hrs Surf.Area= 2,622 sf Storage= 8,188 cf

Plug-Flow detention time= 161.3 min calculated for 0.346 af (77% of inflow)
 Center-of-Mass det. time= 79.4 min (828.7 - 749.3)

Volume	Invert	Avail.Storage	Storage Description
#1A	200.75'	4,300 cf	28.50'W x 91.99'L x 6.75'H Field A 17,697 cf Overall - 6,946 cf Embedded = 10,751 cf x 40.0% Voids
#2A	201.50'	6,946 cf	ADS_StormTech MC-4500 b +Cap x 63 Inside #1 Effective Size= 90.4"W x 60.0"H => 26.46 sf x 4.03'L = 106.5 cf Overall Size= 100.0"W x 60.0"H x 4.33'L with 0.31' Overlap 63 Chambers in 3 Rows Cap Storage= 39.5 cf x 2 x 3 rows = 237.0 cf
		11,246 cf	Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Discarded	200.75'	0.270 in/hr Exfiltration over Surface area
#2	Primary	203.05'	8.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads
#3	Primary	207.25'	5.0' long x 1.0' breadth Broad-Crested Rectangular Weir Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 3.00 Coef. (English) 2.69 2.72 2.75 2.85 2.98 3.08 3.20 3.28 3.31 3.30 3.31 3.32

Discarded OutFlow Max=0.02 cfs @ 3.45 hrs HW=200.82' (Free Discharge)↑ **1=Exfiltration** (Exfiltration Controls 0.02 cfs)**Primary OutFlow** Max=2.20 cfs @ 12.30 hrs HW=205.09' TW=0.00' (Dynamic Tailwater)↑ **2=Orifice/Grate** (Orifice Controls 2.20 cfs @ 6.29 fps)↑ **3=Broad-Crested Rectangular Weir** (Controls 0.00 cfs)

Pond UGS-1: UGS - Chamber Wizard Field A**Chamber Model = ADS_StormTech MC-4500 b +Cap (ADS StormTech® MC-4500 with cap volume)**

Effective Size= 90.4"W x 60.0"H => 26.46 sf x 4.03'L = 106.5 cf

Overall Size= 100.0"W x 60.0"H x 4.33'L with 0.31' Overlap

Cap Storage= 39.5 cf x 2 x 3 rows = 237.0 cf

100.0" Wide + 9.0" Spacing = 109.0" C-C Row Spacing

21 Chambers/Row x 4.02' Long +2.73' Cap Length x 2 = 89.99' Row Length +12.0" End Stone x 2 = 91.99' Base Length

3 Rows x 100.0" Wide + 9.0" Spacing x 2 + 12.0" Side Stone x 2 = 28.50' Base Width

9.0" Stone Base + 60.0" Chamber Height + 12.0" Stone Cover = 6.75' Field Height

63 Chambers x 106.5 cf + 39.5 cf Cap Volume x 2 x 3 Rows = 6,945.9 cf Chamber Storage

17,696.9 cf Field - 6,945.9 cf Chambers = 10,751.0 cf Stone x 40.0% Voids = 4,300.4 cf Stone Storage

Chamber Storage + Stone Storage = 11,246.3 cf = 0.258 af

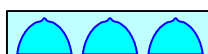
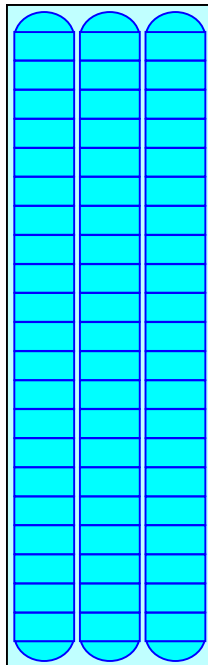
Overall Storage Efficiency = 63.5%

Overall System Size = 91.99' x 28.50' x 6.75'

63 Chambers

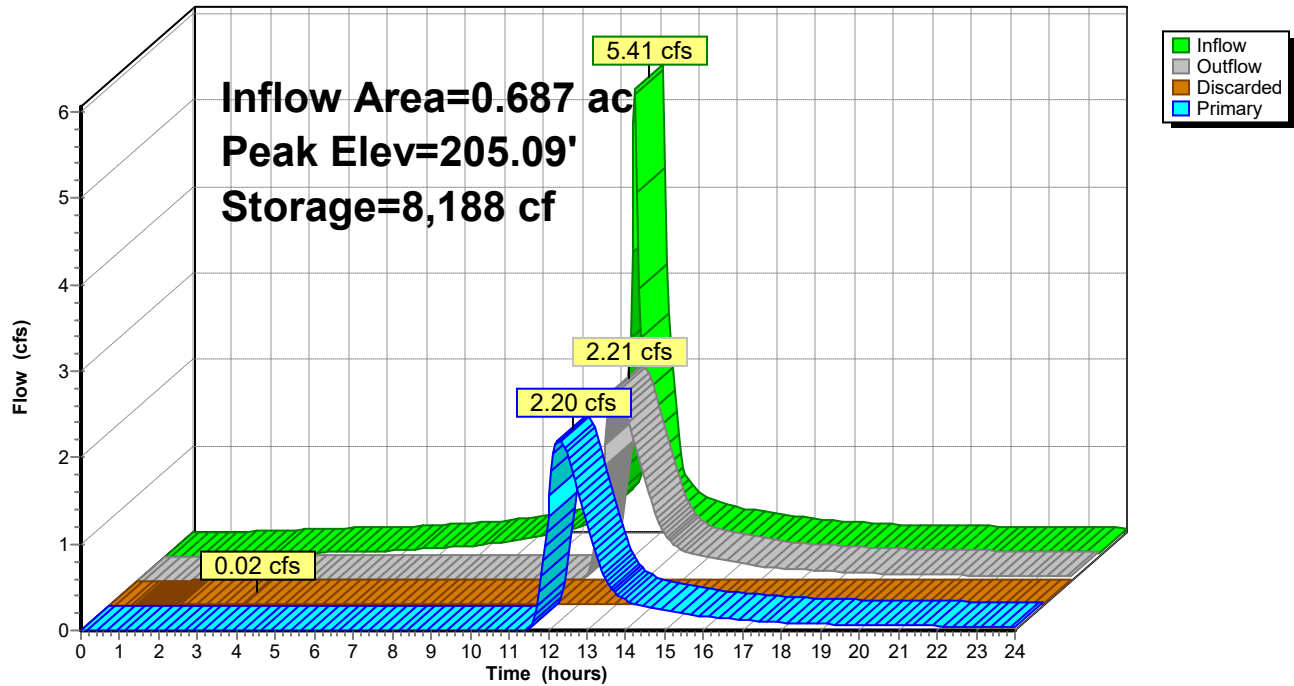
655.4 cy Field

398.2 cy Stone



Pond UGS-1: UGS

Hydrograph



MAA250027 Model

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Type III 24-hr 100 yr Rainfall=11.40"

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Time span=0.00-24.00 hrs, dt=0.05 hrs, 481 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

Subcatchment PR-1: Roof

Runoff Area=5,500 sf 100.00% Impervious Runoff Depth>11.15"
Tc=6.0 min CN=98 Runoff=1.38 cfs 0.117 af

Subcatchment PR-2: to UGS

Runoff Area=24,437 sf 90.49% Impervious Runoff Depth>10.91"
Tc=6.0 min CN=96 Runoff=6.12 cfs 0.510 af

Subcatchment PR-3: to Wetlands

Runoff Area=45,664 sf 0.00% Impervious Runoff Depth>7.77"
Flow Length=461' Tc=8.0 min CN=72 Runoff=8.71 cfs 0.678 af

Reach DPP-1: Offsite

Inflow=11.19 cfs 1.172 af
Outflow=11.19 cfs 1.172 af

Pond UGS-1: UGS

Peak Elev=206.42' Storage=10,115 cf Inflow=7.50 cfs 0.627 af
Discarded=0.02 cfs 0.031 af Primary=2.93 cfs 0.494 af Outflow=2.95 cfs 0.525 af

Total Runoff Area = 1.736 ac Runoff Volume = 1.306 af Average Runoff Depth = 9.03"
63.47% Pervious = 1.102 ac 36.53% Impervious = 0.634 ac

Summary for Subcatchment PR-1: Roof

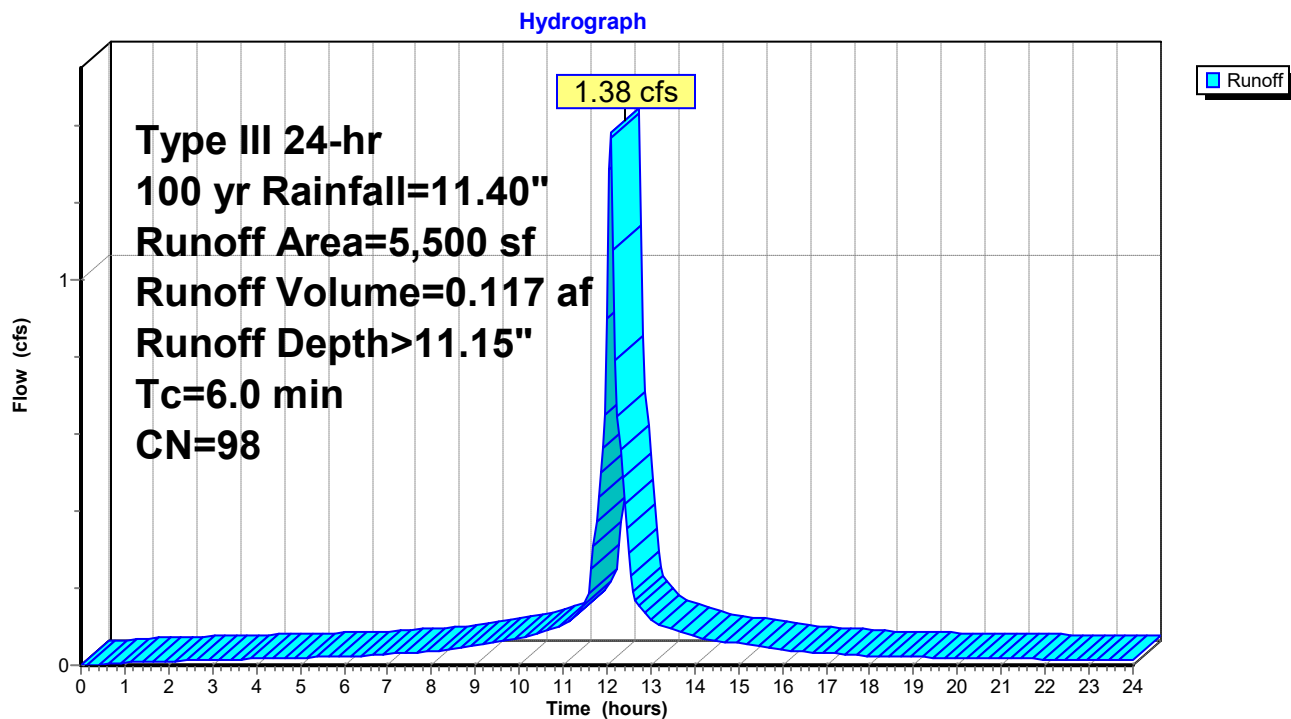
Runoff = 1.38 cfs @ 12.09 hrs, Volume= 0.117 af, Depth>11.15"
Routed to Pond UGS-1 : UGS

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs
Type III 24-hr 100 yr Rainfall=11.40"

Area (sf)	CN	Description
5,500	98	Unconnected roofs, HSG A
5,500		100.00% Impervious Area
5,500		100.00% Unconnected

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, Direct

Subcatchment PR-1: Roof



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Type III 24-hr 100 yr Rainfall=11.40"

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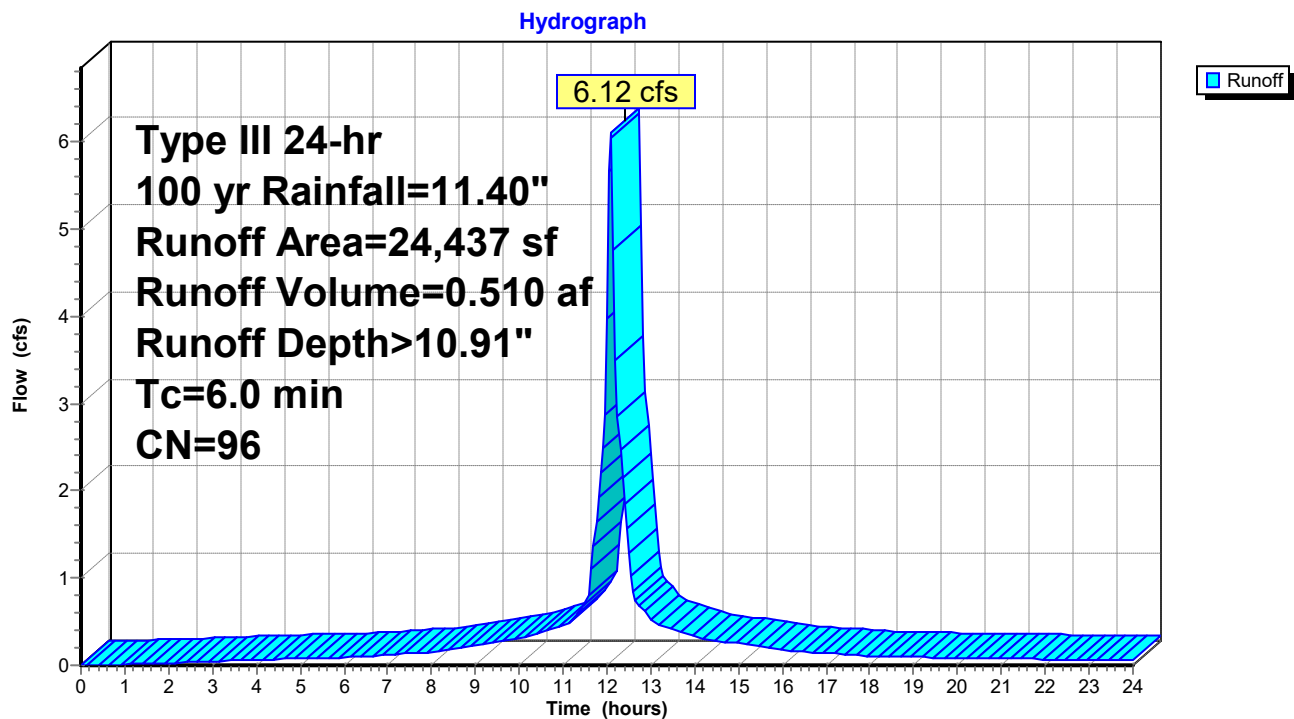
Summary for Subcatchment PR-2: to UGS

Runoff = 6.12 cfs @ 12.09 hrs, Volume= 0.510 af, Depth>10.91"
Routed to Pond UGS-1 : UGS

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs
Type III 24-hr 100 yr Rainfall=11.40"

Area (sf)	CN	Description
2,323	74	>75% Grass cover, Good, HSG C
22,114	98	Paved parking, HSG A
24,437	96	Weighted Average
2,323		9.51% Pervious Area
22,114		90.49% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, Direct

Subcatchment PR-2: to UGS

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Type III 24-hr 100 yr Rainfall=11.40"

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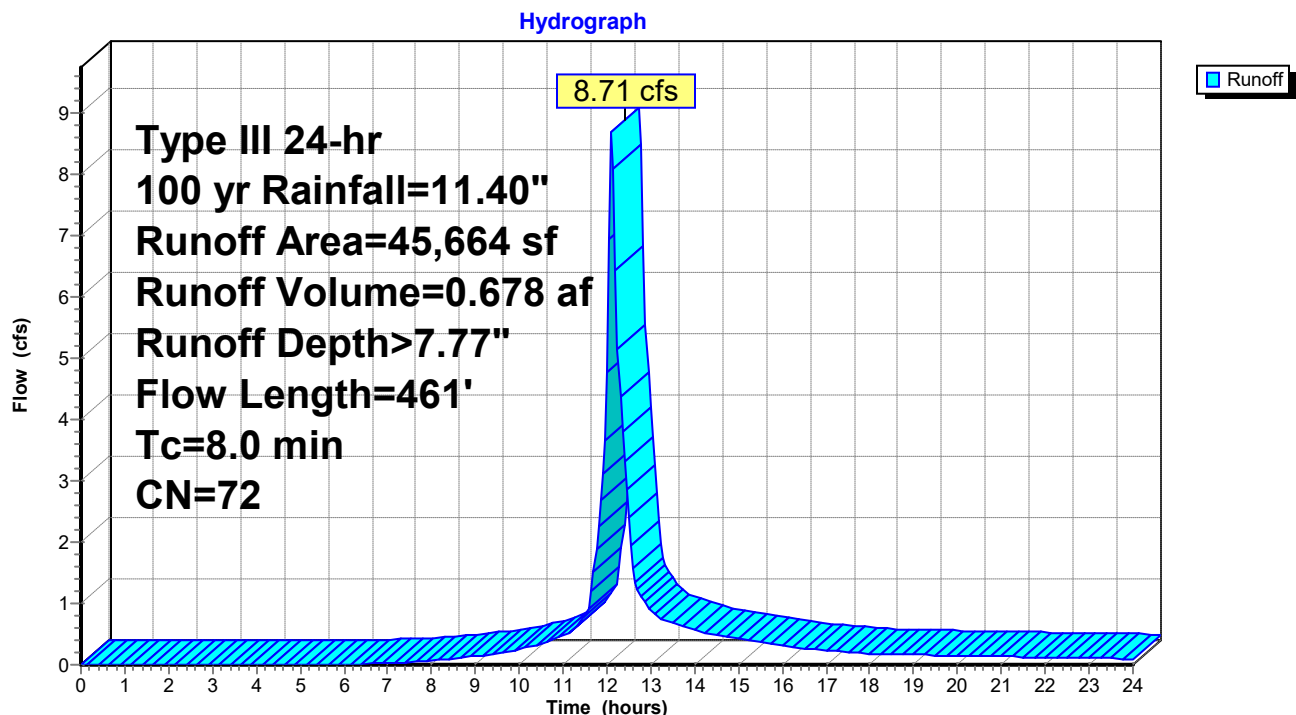
Summary for Subcatchment PR-3: to Wetlands

Runoff = 8.71 cfs @ 12.11 hrs, Volume= 0.678 af, Depth> 7.77"
 Routed to Reach DPP-1 : Offsite

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs
 Type III 24-hr 100 yr Rainfall=11.40"

Area (sf)	CN	Description
22,467	74	>75% Grass cover, Good, HSG C
23,197	70	Woods, Good, HSG C
45,664	72	Weighted Average
45,664		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
0.9	12	0.2700	0.22		Sheet Flow, A to B Grass: Dense n= 0.240 P2= 3.28"
3.5	38	0.0920	0.18		Sheet Flow, B to C Grass: Dense n= 0.240 P2= 3.28"
3.4	388	0.0720	1.88		Shallow Concentrated Flow, C to D Short Grass Pasture Kv= 7.0 fps
0.2	23	0.1000	1.58		Shallow Concentrated Flow, C to D Woodland Kv= 5.0 fps
8.0	461	Total			

Subcatchment PR-3: to Wetlands

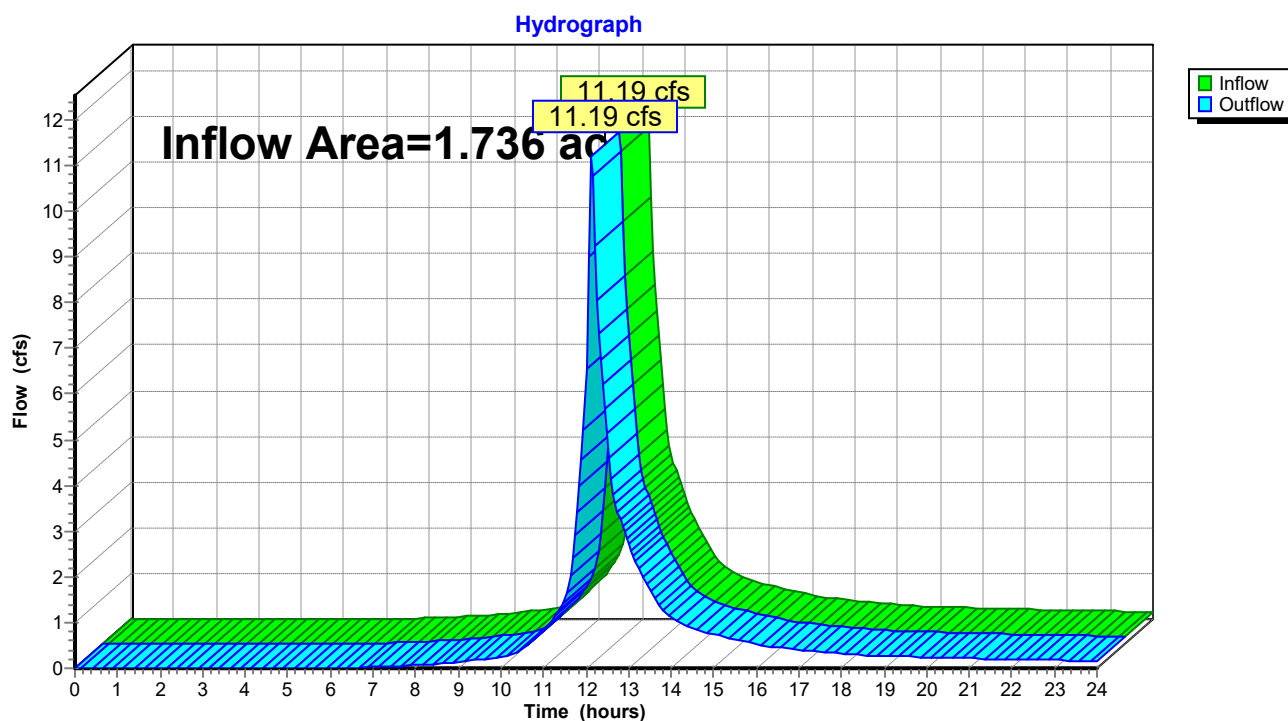
Summary for Reach DPP-1: Offsite

[40] Hint: Not Described (Outflow=Inflow)

Inflow Area = 1.736 ac, 36.53% Impervious, Inflow Depth > 8.11" for 100 yr event
 Inflow = 11.19 cfs @ 12.12 hrs, Volume= 1.172 af
 Outflow = 11.19 cfs @ 12.12 hrs, Volume= 1.172 af, Atten= 0%, Lag= 0.0 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs

Reach DPP-1: Offsite



Summary for Pond UGS-1: UGS

Inflow Area = 0.687 ac, 92.24% Impervious, Inflow Depth > 10.95" for 100 yr event
 Inflow = 7.50 cfs @ 12.09 hrs, Volume= 0.627 af
 Outflow = 2.95 cfs @ 12.31 hrs, Volume= 0.525 af, Atten= 61%, Lag= 13.5 min
 Discarded = 0.02 cfs @ 2.40 hrs, Volume= 0.031 af
 Primary = 2.93 cfs @ 12.31 hrs, Volume= 0.494 af
 Routed to Reach DPP-1 : Offsite

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs
 Peak Elev= 206.42' @ 12.31 hrs Surf.Area= 2,622 sf Storage= 10,115 cf

Plug-Flow detention time= 142.8 min calculated for 0.524 af (84% of inflow)
 Center-of-Mass det. time= 75.1 min (819.3 - 744.1)

Volume	Invert	Avail.Storage	Storage Description
#1A	200.75'	4,300 cf	28.50'W x 91.99'L x 6.75'H Field A 17,697 cf Overall - 6,946 cf Embedded = 10,751 cf x 40.0% Voids
#2A	201.50'	6,946 cf	ADS_StormTech MC-4500 b +Cap x 63 Inside #1 Effective Size= 90.4"W x 60.0"H => 26.46 sf x 4.03'L = 106.5 cf Overall Size= 100.0"W x 60.0"H x 4.33'L with 0.31' Overlap 63 Chambers in 3 Rows Cap Storage= 39.5 cf x 2 x 3 rows = 237.0 cf
		11,246 cf	Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Discarded	200.75'	0.270 in/hr Exfiltration over Surface area
#2	Primary	203.05'	8.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads
#3	Primary	207.25'	5.0' long x 1.0' breadth Broad-Crested Rectangular Weir Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 3.00 Coef. (English) 2.69 2.72 2.75 2.85 2.98 3.08 3.20 3.28 3.31 3.30 3.31 3.32

Discarded OutFlow Max=0.02 cfs @ 2.40 hrs HW=200.82' (Free Discharge)

↑ **1=Exfiltration** (Exfiltration Controls 0.02 cfs)

Primary OutFlow Max=2.93 cfs @ 12.31 hrs HW=206.42' TW=0.00' (Dynamic Tailwater)

↑ **2=Orifice/Grate** (Orifice Controls 2.93 cfs @ 8.38 fps)

↑ **3=Broad-Crested Rectangular Weir** (Controls 0.00 cfs)

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Type III 24-hr 100 yr Rainfall=11.40"

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Pond UGS-1: UGS - Chamber Wizard Field A

Chamber Model = ADS_StormTech MC-4500 b +Cap (ADS StormTech® MC-4500 with cap volume)

Effective Size= 90.4"W x 60.0"H => 26.46 sf x 4.03'L = 106.5 cf

Overall Size= 100.0"W x 60.0"H x 4.33'L with 0.31' Overlap

Cap Storage= 39.5 cf x 2 x 3 rows = 237.0 cf

100.0" Wide + 9.0" Spacing = 109.0" C-C Row Spacing

21 Chambers/Row x 4.02' Long +2.73' Cap Length x 2 = 89.99' Row Length +12.0" End Stone x 2 = 91.99' Base Length

3 Rows x 100.0" Wide + 9.0" Spacing x 2 + 12.0" Side Stone x 2 = 28.50' Base Width

9.0" Stone Base + 60.0" Chamber Height + 12.0" Stone Cover = 6.75' Field Height

63 Chambers x 106.5 cf + 39.5 cf Cap Volume x 2 x 3 Rows = 6,945.9 cf Chamber Storage

17,696.9 cf Field - 6,945.9 cf Chambers = 10,751.0 cf Stone x 40.0% Voids = 4,300.4 cf Stone Storage

Chamber Storage + Stone Storage = 11,246.3 cf = 0.258 af

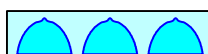
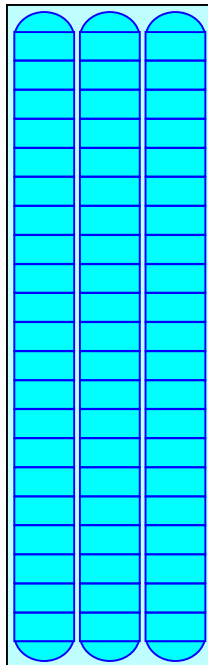
Overall Storage Efficiency = 63.5%

Overall System Size = 91.99' x 28.50' x 6.75'

63 Chambers

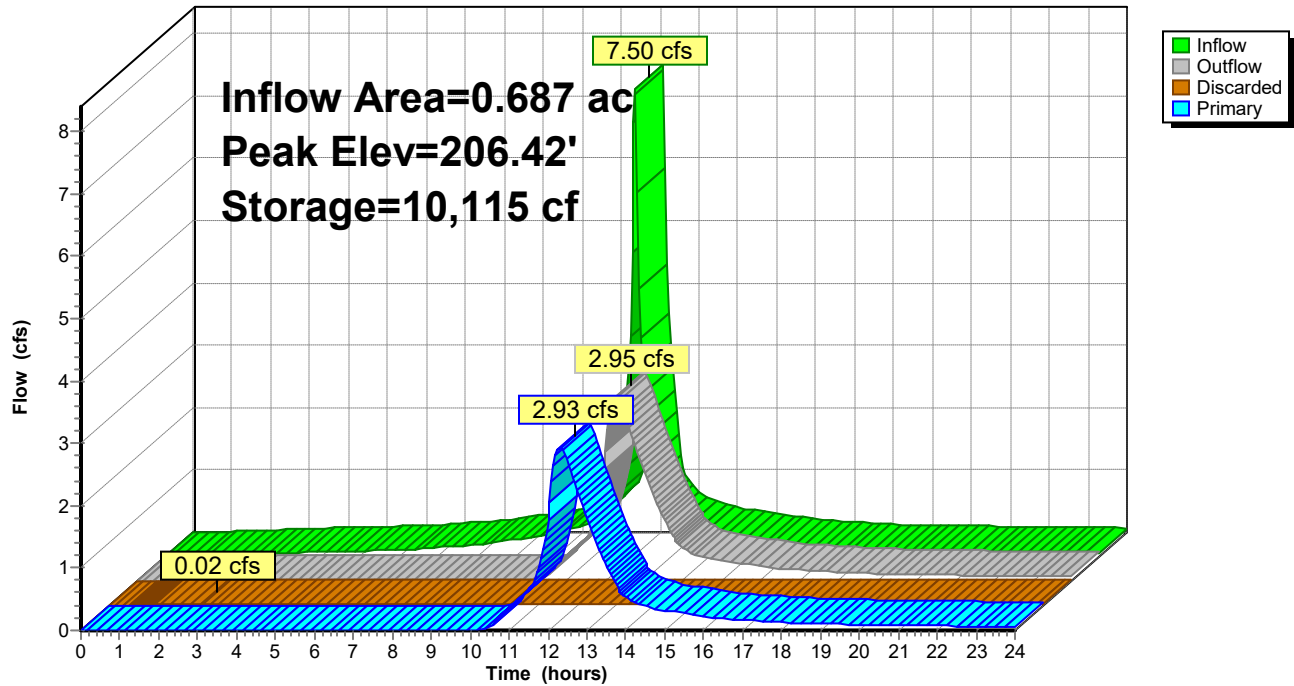
655.4 cy Field

398.2 cy Stone



Pond UGS-1: UGS

Hydrograph



APPENDIX F: STORMWATER CALCULATIONS

- MA STANDARD #3 – RECHARGE AND DRAWDOWN TIME
- MA STANDARD #4 – WATER QUALITY AND TSS REMOVAL
- NOAA RAINFALL DATA
- PIPE AND INLET SIZING
- PHOSPHORUS REMOVAL CALCULATIONS

TLE - Burlington
Cambridge Street
Burlington, MA
Bohler Job Number: MAA250027
December 12, 2025

MA DEP Standard 3: Recharge Volume Calculations

Required Recharge Volume - A Soils (0.60 in.)

Existing Site Impervious Area (ac)	0.000
Proposed Site Impervious Area (ac)	0.000
Proposed Increase in Site Impervious Area (ac)	0.000
Recharge Volume Required (cf)	0

Required Recharge Volume - B Soils (0.35 in.)

Existing Site Impervious Area (ac)	0.000
Proposed Site Impervious Area (ac)	0.000
Proposed Increase in Site Impervious Area (ac)	0.000
Recharge Volume Required (cf)	0

Required Recharge Volume - C Soils (0.25 in.)

Existing Site Impervious Area (ac)	0.000
Proposed Site Impervious Area (ac)	0.627
Proposed Increase in Site Impervious Area (ac)	0.627
Recharge Volume Required (cf)	569

Required Recharge Volume - D Soils (0.10 in.)

Existing Site Impervious Area (ac)	0.000
Proposed Site Impervious Area (ac)	0.000
Proposed Increase in Site Impervious Area (ac)	0.000
Recharge Volume Required (cf)	0

Total Recharge Volume Required (cf)	569
--	------------

Recharge Volume Adjustment Factor

Impervious Area Directed to Infiltration BMP (ac)	0.000
%Impervious Directed to Infiltration BMP	
Adjustment Factor	
Adjusted Total Recharge Volume Required (cf)	

Provided Recharge Volume*

BMP #1 Name from HydroCAD	4,177
Total Recharge Volume Provided (cf)	4,177

Input Required

*Volume provided below lowest outlet in cubic feet (cf)

TLE - Burlington
Cambridge Street
Burlington, MA
Bohler Job Number: MAA250027
December 12, 2025

MA DEP Standard 3: Drawdown Time Calculations

Drawdown Time - BMP #1 Name from HydroCAD	
Volume below outlet pipe (Rv) (cf)	4,177
Soil Type	Silt Loam - C
Infiltration rate (K)*	0.27
Bottom Area (sf)	2,622
Drawdown time (Hours)*	70.8

*Infiltration Rates taken from Rawls Table

**Drawdown time = $R_v / (K \times \text{bottom area})$

TLE - Burlington
Cambridge Street
Burlington, MA
Bohler Job Number: MAA250027
December 12, 2025

MA DEP Standard 4: Water Quality Volume Calculations

Water Quality Volume Required

Water Quality Volume runoff (in.)*	1.0
Total Post Development Impervious Area (sf)	27,311
Required Water Quality Volume (cf)	2,276

*Water Quality volume runoff is equal to 0.5 or 1.0 inches of runoff times the total impervious area of the post development project site.

Water Quality Volume Provided*

BMP #1 Name from HydroCAD	4,177
0	0
0	0
0	0
0	0
Total Provided Water Quality Volume (cf)	4,177

Provided greater than or Equal to Required

*Volume provided below lowest outlet pipe in cubic feet (cf)

TSS Removal Calculation Worksheet

Location: PR-2 to UGS

A BMP ¹	B TSS Removal Rate ¹	C Starting TSS Load*	D Amount Removed (B*C)	E Remaining Load (C-D)
Water Quality Unit	0.80	1.00	0.80	0.20
Isolator Row	0.80	0.20	0.16	0.04

Total TSS Removal =

96%

Project: TLE - Burlington
 Prepared By: Bohler Engineering
 Date: 12/12/2025

*Equals remaining load from previous BMP (E)
 which enters the BMP

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Type III 24-hr 100 yr Rainfall=11.40"

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Stage-Area-Storage for Pond UGS-1: UGS

Elevation (feet)	Surface (sq-ft)	Storage (cubic-feet)	Elevation (feet)	Surface (sq-ft)	Storage (cubic-feet)
200.75	2,622	0	205.95	2,622	9,556
200.85	2,622	105	206.05	2,622	9,687
200.95	2,622	210	206.15	2,622	9,808
201.05	2,622	315	206.25	2,622	9,923
201.15	2,622	419	206.35	2,622	10,035
201.25	2,622	524	206.45	2,622	10,145
201.35	2,622	629	206.55	2,622	10,250
201.45	2,622	734	206.65	2,622	10,355
201.55	2,622	898	206.75	2,622	10,460
201.65	2,622	1,121	206.85	2,622	10,565
201.75	2,622	1,344	206.95	2,622	10,670
201.85	2,622	1,566	207.05	2,622	10,774
201.95	2,622	1,788	207.15	2,622	10,879
202.05	2,622	2,009	207.25	2,622	10,984
202.15	2,622	2,229	207.35	2,622	11,089
202.25	2,622	2,449	207.45	2,622	11,194
202.35	2,622	2,668			
202.45	2,622	2,886			
202.55	2,622	3,104			
202.65	2,622	3,320			
202.75	2,622	3,536			
202.85	2,622	3,751			
202.95	2,622	3,965			
203.05	2,622	4,177			
203.15	2,622	4,389			
203.25	2,622	4,600			
203.35	2,622	4,809			
203.45	2,622	5,017			
203.55	2,622	5,224			
203.65	2,622	5,429			
203.75	2,622	5,633			
203.85	2,622	5,836			
203.95	2,622	6,037			
204.05	2,622	6,236			
204.15	2,622	6,434			
204.25	2,622	6,629			
204.35	2,622	6,823			
204.45	2,622	7,015			
204.55	2,622	7,205			
204.65	2,622	7,393			
204.75	2,622	7,578			
204.85	2,622	7,761			
204.95	2,622	7,941			
205.05	2,622	8,119			
205.15	2,622	8,294			
205.25	2,622	8,465			
205.35	2,622	8,634			
205.45	2,622	8,799			
205.55	2,622	8,960			
205.65	2,622	9,117			
205.75	2,622	9,269			
205.85	2,622	9,416			

VOLUME BELOW LOWEST
OUTLET = 4,177 CF
(8" ORIFICE, INV=203.05")



NOAA Atlas 14, Volume 10, Version 3
Location name: Burlington, Massachusetts, USA*
Latitude: 42.4979°, Longitude: -71.1947°
Elevation: m/ft**
 * source: ESRI Maps
 ** source: USGS



POINT PRECIPITATION FREQUENCY ESTIMATES

Sanja Perica, Sandra Pavlovic, Michael St. Laurent, Carl Trypaluk, Dale Unruh, Orlan Wilhite

NOAA, National Weather Service, Silver Spring, Maryland

[PF_tabular](#) | [PF_graphical](#) | [Maps & aerals](#)

PF tabular

PDS-based point precipitation frequency estimates with 90% confidence intervals (in inches) ¹										
Duration	Average recurrence interval (years)									
	1	2	5	10	25	50	100	200	500	1000
5-min	0.310 (0.241-0.391)	0.376 (0.291-0.474)	0.483 (0.373-0.611)	0.572 (0.439-0.727)	0.694 (0.517-0.924)	0.785 (0.574-1.07)	0.882 (0.629-1.25)	0.996 (0.669-1.43)	1.16 (0.754-1.74)	1.31 (0.827-1.98)
10-min	0.440 (0.341-0.554)	0.532 (0.412-0.671)	0.683 (0.528-0.864)	0.809 (0.622-1.03)	0.983 (0.733-1.31)	1.11 (0.814-1.52)	1.25 (0.891-1.77)	1.41 (0.949-2.03)	1.65 (1.07-2.46)	1.85 (1.17-2.81)
15-min	0.517 (0.401-0.651)	0.626 (0.485-0.790)	0.804 (0.620-1.02)	0.952 (0.731-1.21)	1.16 (0.862-1.54)	1.31 (0.957-1.78)	1.47 (1.05-2.08)	1.66 (1.12-2.39)	1.94 (1.26-2.89)	2.18 (1.38-3.31)
30-min	0.709 (0.550-0.893)	0.859 (0.666-1.08)	1.10 (0.853-1.40)	1.31 (1.01-1.67)	1.59 (1.19-2.12)	1.80 (1.32-2.45)	2.02 (1.44-2.87)	2.29 (1.54-3.29)	2.68 (1.74-4.00)	3.01 (1.91-4.58)
60-min	0.900 (0.698-1.13)	1.09 (0.846-1.38)	1.41 (1.09-1.78)	1.67 (1.28-2.12)	2.02 (1.51-2.70)	2.29 (1.68-3.12)	2.58 (1.84-3.66)	2.92 (1.96-4.20)	3.42 (2.21-5.10)	3.85 (2.44-5.84)
2-hr	1.16 (0.910-1.46)	1.42 (1.11-1.78)	1.84 (1.43-2.31)	2.19 (1.69-2.76)	2.67 (2.00-3.54)	3.02 (2.23-4.10)	3.40 (2.46-4.83)	3.88 (2.61-5.54)	4.60 (2.99-6.81)	5.24 (3.32-7.89)
3-hr	1.35 (1.06-1.68)	1.65 (1.30-2.06)	2.14 (1.68-2.68)	2.55 (1.98-3.21)	3.12 (2.35-4.12)	3.53 (2.62-4.78)	3.98 (2.89-5.64)	4.54 (3.07-6.47)	5.42 (3.53-7.99)	6.19 (3.94-9.29)
6-hr	1.74 (1.38-2.15)	2.13 (1.68-2.64)	2.77 (2.18-3.44)	3.30 (2.58-4.12)	4.02 (3.06-5.29)	4.56 (3.40-6.14)	5.14 (3.75-7.24)	5.88 (3.99-8.31)	7.02 (4.58-10.3)	8.02 (5.12-12.0)
12-hr	2.21 (1.76-2.71)	2.70 (2.15-3.33)	3.52 (2.79-4.34)	4.19 (3.30-5.20)	5.12 (3.91-6.67)	5.80 (4.35-7.74)	6.54 (4.79-9.12)	7.47 (5.09-10.5)	8.91 (5.84-12.9)	10.2 (6.50-15.0)
24-hr	2.64 (2.12-3.22)	3.28 (2.63-4.00)	4.32 (3.45-5.29)	5.18 (4.11-6.37)	6.36 (4.90-8.25)	7.23 (5.47-9.61)	8.19 (6.04-11.4)	9.39 (6.43-13.1)	11.3 (7.42-16.3)	12.9 (8.31-19.0)
2-day	3.00 (2.42-3.63)	3.79 (3.06-4.59)	5.09 (4.09-6.18)	6.16 (4.93-7.53)	7.64 (5.94-9.87)	8.72 (6.65-11.6)	9.92 (7.40-13.8)	11.5 (7.89-15.9)	14.0 (9.25-20.1)	16.3 (10.5-23.7)
3-day	3.28 (2.67-3.96)	4.14 (3.36-4.99)	5.53 (4.47-6.70)	6.68 (5.37-8.14)	8.28 (6.46-10.7)	9.43 (7.23-12.5)	10.7 (8.04-14.9)	12.4 (8.56-17.1)	15.2 (10.0-21.7)	17.7 (11.4-25.7)
4-day	3.56 (2.90-4.27)	4.44 (3.61-5.34)	5.87 (4.76-7.09)	7.06 (5.69-8.58)	8.71 (6.81-11.2)	9.90 (7.61-13.0)	11.2 (8.44-15.5)	13.0 (8.97-17.9)	15.9 (10.5-22.6)	18.4 (11.9-26.7)
7-day	4.32 (3.54-5.16)	5.23 (4.28-6.26)	6.72 (5.48-8.07)	7.96 (6.45-9.60)	9.66 (7.60-12.3)	10.9 (8.41-14.2)	12.3 (9.25-16.8)	14.1 (9.77-19.2)	17.0 (11.3-24.0)	19.7 (12.7-28.2)
10-day	5.02 (4.13-5.97)	5.95 (4.89-7.09)	7.48 (6.13-8.94)	8.75 (7.12-10.5)	10.5 (8.28-13.3)	11.8 (9.10-15.3)	13.2 (9.92-17.9)	15.0 (10.4-20.4)	17.9 (11.9-25.1)	20.4 (13.3-29.3)
20-day	6.99 (5.80-8.26)	8.02 (6.65-9.49)	9.70 (8.01-11.5)	11.1 (9.09-13.2)	13.0 (10.3-16.2)	14.4 (11.1-18.3)	16.0 (11.9-21.0)	17.7 (12.4-23.8)	20.3 (13.6-28.2)	22.4 (14.6-31.9)
30-day	8.64 (7.20-10.2)	9.74 (8.11-11.5)	11.5 (9.56-13.6)	13.0 (10.7-15.5)	15.1 (11.9-18.6)	16.6 (12.8-20.9)	18.2 (13.5-23.7)	19.9 (14.0-26.7)	22.3 (15.0-30.8)	24.1 (15.7-34.1)
45-day	10.7 (8.99-12.6)	11.9 (9.96-14.0)	13.8 (11.5-16.3)	15.4 (12.8-18.2)	17.6 (14.0-21.6)	19.3 (14.9-24.1)	21.0 (15.5-26.9)	22.7 (16.0-30.1)	24.8 (16.7-34.1)	26.4 (17.2-37.1)
60-day	12.5 (10.5-14.6)	13.7 (11.5-16.1)	15.8 (13.2-18.5)	17.4 (14.5-20.6)	19.7 (15.7-24.0)	21.5 (16.7-26.7)	23.3 (17.2-29.7)	24.9 (17.6-33.0)	27.0 (18.2-37.0)	28.4 (18.6-39.8)

¹ Precipitation frequency (PF) estimates in this table are based on frequency analysis of partial duration series (PDS).

Numbers in parenthesis are PF estimates at lower and upper bounds of the 90% confidence interval. The probability that precipitation frequency estimates (for a given duration and average recurrence interval) will be greater than the upper bound (or less than the lower bound) is 5%. Estimates at upper bounds are not checked against probable maximum precipitation (PMP) estimates and may be higher than currently valid PMP values.

Please refer to NOAA Atlas 14 document for more information.

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PF graphical

TLE - Burlington
Cambridge Street
Burlington, MA
Bohler Job Number: MAA250027
December 12, 2025

Rational Pipe and Grate Sizing Calculations

Design Period Storm:		25	Year	Design Period Intensity*			8.25	in/hr									
LOCATION		IMPERVIOUS			OTHER			SUM CA	Tc (min)	I (in/hr)	Q (cfs)	D (in)	S (ft/ft)	Matieral	n	Q Full (cfs)	V Full (fps)
FROM	TO	A	C	CA	A	C	CA										
A-61	A-60	0.30	0.95	0.29	0.05	0.30	0.02	0.30	6	8.25	2.48	12	0.038	HDPE	0.012	7.52	9.58
A-62	A-60	0.09	0.95	0.09	0.00	0.30	0.00	0.09	6	8.25	0.72	12	0.015	HDPE	0.012	4.73	6.02
A-60	A-50	0.39	0.95	0.37	0.05	0.30	0.02	0.39	6	8.25	3.19	12	0.019	HDPE	0.012	5.32	6.77
A-50	A-40	0.39	0.95	0.37	0.05	0.30	0.02	0.39	6	8.25	3.19	18	0.008	RCP	0.013	9.40	5.32
A-41	A-40	0.13	0.95	0.12	0.00	0.30	0.00	0.12	6	8.25	1.02	8	0.052	HDPE	0.012	2.99	8.55
A-40	A-30								6		2.20	15	0.011	HDPE	0.012	7.34	5.98
A-30	A-20								6		2.20	15	0.011	HDPE	0.012	7.34	5.98
A-20	A-10								6		2.20	15	0.011	HDPE	0.012	7.34	5.98

*Rainfall intesity provided by NOAA Atlas 14, Volume 10, Version 2 on 10/27/2025

Inlet	Flow To CB (cfs)**	Grate Type	Grate Inlet Area (36) 2"x2"	Head (ft)***	Head (in)***	Single Grate Capacity (cfs)	Double Grate Capacity (cfs)	Grate Size
A-61	2.48	Standard	1.00	0.5	6	2.38	4.77	Double
A-62	0.72	Standard	1.00	0.5	6	2.38	4.77	Single

Orifice Equation used to calculate Grate Inlet Capacity

Q=C x A x (2gh)^(1/2)x f

where: Q = Grate Capacity in cfs

C = Orifice Coefficient = 0.6

A = Free open area of grate in sf

h = Head over grate in ft

g = Gravity = 32.2 ft/s2

f = Clogging Factor = 0.7 (30% clogged)

**Flow to inlet structure from Rational pipe sizing calculations
***Available head is the largest depth that an inlet can be submerged before runoff overtops and flows to another inlet or catchment area

Proposed Child Care Facility
Cambridge Street, Burlington, MA
December 15, 2025

Post-Construction Phosphorus Reduction Calculation

Objective: Determine the reduction in total phosphorus (TP) loading for a given construction land use following the installation of the stormwater Best Management Practices (BMPs). Percent reduction shall be greater than or equal to 60% for new development, as required by the City of Burlington.

Methodology: Output from the U.S. EPA "BMP Accounting and Tracking Tool (BATT) version 2.2"

Treatment Train #1: Subcatchments PR-#1 & PR-#2 to Underground Infiltration System

Add Structural BMP

BMP Land Use Information | BMP Information | Property Information

Project Type

☒ New Development* ☐ Retrofit BMP ☐ Other

* If the associated project will alter land uses, enter a Land Use Change project separately.

Select Land Area Treated by the BMP

Land Use Type: **COMMERCIAL (P)**

Land Use Area (acre):

Hydrologic Soil Group:

Note: Land use types are followed by letter to represent pervious or impervious. P denotes pervious land use, and I denotes impervious land use. **Add ->**

View/Edit Land Loading Rates

BMP Drainage Area *

Note: Click the Refresh button after changing the land use info in BMP drainage area.

COMMERCIAL (I), 0.13, N/A, 1.78, 1, 15.08, 1, 377.39, 1

Delete Selected Drainage Area

* BMP Drainage Area Note

The format of land use information stored in BMP drainage area: Land Use Type, Area, HSG, Phosphorus Land Loading Rate, Phosphorus Adjustment Factor, Nitrogen Land Loading Rate, Nitrogen Adjustment Factor, Sediment Land Loading Rate, Sediment Adjustment Factor.

Refresh **Calculate Credit** **Save** **Close** **Next ->**

Subcatchment PR-1: Phosphorus Loading

Commercial (Impervious Area) = 0.13 acres x 1.78 lb/year/acre = 0.231 lb/year

BMP Land Use Information | **BMP Information** | **Property Information**

Project Type
☒ New Development* ☐ Retrofit BMP ☐ Other
* If the associated project will alter land uses, enter a Land Use Change project separately.

Select Land Area Treated by the BMP
 Land Use Type: **COMMERCIAL (I)**
 Land Use Area (acre): **0.51**
 Hydrologic Soil Group: **N/A**
Note: Land use types are followed by letter to represent pervious or impervious. P denotes pervious land use, and I denotes impervious land use.

*** BMP Drainage Area Note**
 The format of land use information stored in BMP drainage area: Land Use Type, Area, HSG, Phosphorus Land Loading Rate, Phosphorus Adjustment Factor, Nitrogen Land Loading Rate, Nitrogen Adjustment Factor, Sediment Land Loading Rate, Sediment Adjustment Factor.

Unique Project Identifier:
 Receiving Water: **N/A**

BMP Drainage Area *
Note: Click the Refresh button after changing the land use info in BMP drainage area.
COMMERCIAL (P), 0.05, C, 0.21, 1, 2.41, 1, 59.78, 1
COMMERCIAL (I), 0.51, N/A, 1.78, 1, 15.08, 1, 377.39, 1

Subcatchment PR-2: Phosphorus Loading

Commercial (Impervious Area) = 0.51 acres x 1.78 lb/year/acre = 0.908 lb/year

Commercial (Pervious Area) = 0.05 acres x 0.21 lb/year/acre = 0.011 lb/year

Add Structural BMP

BMP Land Use Information | **BMP Information** | **Property Information**

Project Type
☒ New Development* ☐ Retrofit BMP ☐ Other
* If the associated project will alter land uses, enter a Land Use Change project separately.

Select Land Area Treated by the BMP
 Land Use Type: **COMMERCIAL (P)**
 Land Use Area (acre): **1.05**
 Hydrologic Soil Group: **C**
Note: Land use types are followed by letter to represent pervious or impervious. P denotes pervious land use, and I denotes impervious land use.

*** BMP Drainage Area Note**
 The format of land use information stored in BMP drainage area: Land Use Type, Area, HSG, Phosphorus Land Loading Rate, Phosphorus Adjustment Factor, Nitrogen Land Loading Rate, Nitrogen Adjustment Factor, Sediment Land Loading Rate, Sediment Adjustment Factor.

Unique Project Identifier:
 Receiving Water: **N/A**

BMP Drainage Area *
Note: Click the Refresh button after changing the land use info in BMP drainage area.
COMMERCIAL (P), 1.05, C, 0.21, 1, 2.41, 1, 59.78, 1

Subcatchment PR-3: Phosphorus Loading

Commercial (Pervious Area) = 1.05 acres x 0.21 lb/year/acre = 0.221 lb/year

Total (PR-1 + PR-2 + PR-3 + PR-4) = 1.371 lb/year

Add Structural BMP

BMP Land Use Information | **BMP Information** | Property Information

Select BMP Type: **INFILTRATION BASIN** View/Edit BMP Efficiencies

Note: Click the Refresh button after changing the BMP Type

BMP Specifications

Infiltration Rate (in/hr): **0.27**

Storage Volume (ft³): **4177** Calculate Storage Volume

BMP Location (Optional)

BMP Latitude (decimal degree):

BMP Longitude (decimal degree):

BMP Credit

Removed Phosphorus Load (lb/yr): **1.134**

Removed Nitrogen Load (lb/yr): **9.732**

Removed Sediment Load (lb/yr): **244.519** Close

Date of BMP Completion:

Date of Last Inspection:

< Back Refresh Calculate Credit Save Close Next ->

Infiltration System: Storage Volume

Basin Volume below outlet elevation (per HydroCAD) = 4,177 CF

Total = 4,177 CF

Removal Credit = 1.134 lb/year (see above)

Total Phosphorus Weighted Removal

Phosphorus Loading = 1.371 lb/year

Removed Phosphorus Load = 1.134 lb/year (Infiltration System)

Post-Construction TP Reduction = 1.134 / 1.371 = 82.7%

APPENDIX G: OPERATION AND MAINTENANCE

- STORMWATER OPERATION AND MAINTENANCE PLAN
- INSPECTION REPORT
- INSPECTION AND MAINTENANCE LOG FORM
- LONG-TERM POLLUTION PREVENTION PLAN
- ILLICIT DISCHARGE STATEMENT
- SPILL PREVENTION
- PROPOSED BMP MAP
- ISOLATOR ROW O&M

STORMWATER OPERATION AND MAINTENANCE PLAN

*Foxborough Learning, LLC
Cambridge Street
Burlington, MA*

RESPONSIBLE PARTY DURING CONSTRUCTION:

*Foxborough Learning, LLC
Cambridge Street
Burlington, MA*

RESPONSIBLE PARTY POST CONSTRUCTION:

*Foxborough Learning, LLC
Cambridge Street
Burlington, MA*

Construction Phase

During the construction phase, all erosion control devices and measures shall be maintained in accordance with the final record plans, local/state approvals and conditions, the EPA Construction General Permit and the Stormwater Pollution Prevention Plan (SWPPP) if applicable. Additionally, the maintenance of all erosion / siltation control measures during construction shall be the responsibility of the general contractor. Contact information of the OWNER and CONTRACTOR shall be listed in the SWPPP for this site. The SWPPP also includes information regarding construction period allowable and illicit discharges, housekeeping and emergency response procedures. Upon proper notice to the property owner, the Town/City or its authorized designee shall be allowed to enter the property at a reasonable time and in a reasonable manner for the purposes of inspection.

Post Development Controls

Once construction is completed, the post development stormwater controls are to be operated and maintained in compliance with the following permanent procedures (note that the continued implementation of these procedures shall be the responsibility of the Owner or its assignee):

1. Parking lots and on-site driveways: Sweep at least four (4) times per year and on a more frequent basis depending on sanding operations. All resulting sweepings shall be collected and properly disposed of off site in accordance with MADEP and other applicable requirements.

Approximate Maintenance Budget: \$1,000/year

2. Catch basins, yard drains, trench drains, manholes and piping: Inspect four (4) times per year and at the end of foliage and snow-removal seasons. These features shall be cleaned four (4) times per year or whenever the depth of deposits is greater than or equal to one half the depth from the bottom of the invert of the lowest pipe in the catch basin or underground system. Accumulated sediment and hydrocarbons present must be removed

and properly disposed of off site in accordance with MADEP and other applicable requirements.

Approximate Maintenance Budget: \$500/year per structure.

3. Water Quality Unit (Proprietary Separator): Follow manufacturer's recommendations (attached).

Approximate Maintenance Budget: \$1,000/year per unit.

4. Underground Infiltration Basins: Preventative maintenance after every major storm event during the first three (3) months of operation and at least twice per year thereafter. Inspect structure and pretreatment BMP to ensure proper operation after every major storm event (generally equal or greater to 3.0 inches in 24 hours) for the first three months. The outlet of the basin, if any, shall be inspected for erosion and sedimentation, and rip-rap shall be promptly repaired in the case of erosion. Sediment collecting in the bottom of the basin shall be inspected twice annually, and removal shall commence any time the sediment reaches a depth of six inches anywhere in the basin. Any sediment removed shall be disposed of in accordance with MADEP and other applicable requirements.

Approximate Maintenance Budget: Cleaning - \$1,000/year, Inspection - \$200/year

All components of the stormwater system will be accessible by the owner or their assignee.

STORMWATER MANAGEMENT SYSTEM
POST-CONSTRUCTION INSPECTION REPORT

LOCATION:

*Foxborough Learning, LLC
Cambridge Street
Burlington, MA*

RESPONSIBLE PARTY:

*Foxborough Learning, LLC
Cambridge Street
Burlington, MA*

NAME OF INSPECTOR:	INSPECTION DATE:
Note Condition of the Following (sediment depth, debris, standing water, damage, etc.):	
Catch Basins:	
Discharge Points/ Flared End Sections / Rip Rap:	
Infiltration Basin:	
Water Quality Units:	
Other:	

Note Recommended Actions to be taken on the Following (sediment and/or debris removal, repairs, etc.):

Catch Basins:

Discharge Points / Flared End Sections / Rip Rap:

Infiltration Basin:

Water Quality Units:

Other:

Other:

Comments:

STORMWATER INSPECTION AND MAINTENANCE LOG FORM

Foxborough Learning, LLC

Foxborough Learning, LLC – Burlington, MA

[illegible]

LONG-TERM POLLUTION PREVENTION PLAN

*Foxborough Learning, LLC
Cambridge Street
Burlington, MA*

RESPONSIBLE PARTY DURING CONSTRUCTION:

*Foxborough Learning, LLC
Cambridge Street
Burlington, MA*

RESPONSIBLE PARTY POST CONSTRUCTION:

*Foxborough Learning, LLC
Cambridge Street
Burlington, MA*

For this site, the Long-Term Pollution Prevention Plan will consist of the following:

- The property owner shall be responsible for “good housekeeping” including proper periodic maintenance of building and pavement areas, curbing, landscaping, etc.
- Proper storage and removal of solid waste (dumpsters).
- Sweeping of driveways a minimum of twice per year with a commercial cleaning unit. Any sediment removed shall be disposed of in accordance with applicable local and state requirements.
- Regular inspections and maintenance of Stormwater Management System as noted in the “O&M Plan”.
- Snow removal shall be the responsibility of the property owner. Snow shall not be plowed, dumped and/or placed in forebays, infiltration basins or similar stormwater controls. Salting and/or sanding of pavement / walkway areas during winter conditions shall only be done in accordance with all state/local requirements and approvals.

OPERATON AND MAINTENANCE TRAINING PROGRAM

The Owner will coordinate an annual in-house training session to discuss the Operations and Maintenance Plan, the Long-Term Pollution Prevention Plan, and the Spill Prevention Plan and response procedures. Annual training will include the following:

Discuss the Operations and Maintenance Plan

- Explain the general operations of the stormwater management system and its BMPs
- Identify potential sources of stormwater pollution and measures / methods of reducing or eliminating that pollution
- Emphasize good housekeeping measures

Discuss the Spill Prevention and Response Procedures

- Explain the process in the event of a spill
- Identify potential sources of spills and procedures for cleanup and /or reporting and notification
- Complete a yearly inventory or Materials Safety Data sheets of all tenants and confirm that no potentially harmful chemicals are in use.

ILLICIT DISCHARGE STATEMENT

Certain types of non-stormwater discharges are allowed under the U.S. Environmental Protection Agency Construction General Permit. These types of discharges will be allowed under the conditions that no pollutants will be allowed to come in contact with the water prior to or after its discharge. The control measures which have been outlined previously in this LTPPP will be strictly followed to ensure that no contamination of these non-storm water discharges takes place. Any existing illicit discharges, if discovered during the course of the work, will be reported to MassDEP and the local DPW, as applicable, to be addressed in accordance with their respective policies. No illicit discharges will be allowed in conjunction with the proposed improvements.

Duly Acknowledged:

Name & Title

SPILL PREVENTION AND RESPONSE PROCEDURES

(POST CONSTRUCTION)

In order to prevent or minimize the potential for a spill of Hazardous Substances or Oil or come into contact with stormwater, the following steps will be implemented:

1. All Hazardous Substances or Oil (such as pesticides, petroleum products, fertilizers, detergents, acids, paints, paint solvents, cleaning solvents, etc.) will be stored in a secure location, with their lids on, preferably under cover, when not in use.
2. The minimum practical quantity of all such materials will be kept on site.
3. A spill control and containment kit (containing, for example, absorbent materials, acid neutralizing powder, brooms, dust pans, mops, rags, gloves, goggles, plastic and metal trash containers, etc.) will be provided on site.
4. Manufacturer's recommended methods for spill cleanup will be clearly posted and site personnel will be trained regarding these procedures and the location of the information and cleanup supplies.
5. It is the OWNER's responsibility to ensure that all Hazardous Waste on site is disposed of properly by a licensed hazardous material disposal company. The OWNER is responsible for not exceeding Hazardous Waste storage requirements mandated by the EPA or state and local authorities.

In the event of a spill of Hazardous Substances or Oil, the following procedures should be followed:

1. All measures should be taken to contain and abate the spill and to prevent the discharge of the Hazardous Substance or Oil to stormwater or off-site. (The spill area should be kept well ventilated and personnel should wear appropriate protective clothing to prevent injury from contact with the Hazardous Substances.)
2. For spills of less than five (5) gallons of material, proceed with source control and containment, clean-up with absorbent materials or other applicable means unless an imminent hazard or other circumstances dictate that the spill should be treated by a professional emergency response contractor.
3. For spills greater than five (5) gallons of material immediately contact the MADEP at the toll-free 24-hour statewide emergency number: **1-888-304-1133**, the local fire department (**9-1-1**) and an approved emergency response contractor. Provide information on the type of material spilled, the location of the spill, the quantity spilled, and the time of the spill to the emergency response contractor or coordinator, and proceed with prevention, containment and/or clean-up if so desired. (Use the form provided, or similar).
4. If there is a Reportable Quantity (RQ) release, then the National Response Center should be notified immediately at (800) 424-8802; within 14 days a report should be submitted to the EPA regional office describing the release, the date and circumstances of the release and the steps taken to prevent another release. This Pollution Prevention Plan should be updated to reflect any such steps or actions taken and measures to prevent the same from reoccurring.

Foxborough Learning, LLC
Cambridge Street
Burlington, MA

1. Immediately notify The Burlington Fire Department (at **9-1-1**)
2. All measures must be taken to contain and abate the spill and to prevent the discharge of the pollutant(s) to off-site locations, receiving waters, wetlands and/or resource areas.
3. Notify the Burlington Health Department at (781) 270-1955 and the Burlington Conservation Commission at (781) 270-1655.
4. Provide documentation from licensed contractor showing disposal and cleanup procedures were completed as well as details on chemicals that were spilled to the Town of Burlington Health Department and Conservation Commission.

Weather Conditions: _____

[illegible]

Cause of Spill: _____

Measures Taken to Clean up Spill: _____

Type of equipment: _____ Make: _____ Size: _____

License or S/N: _____

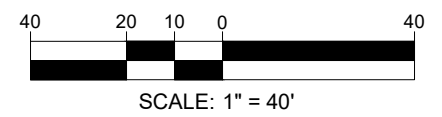
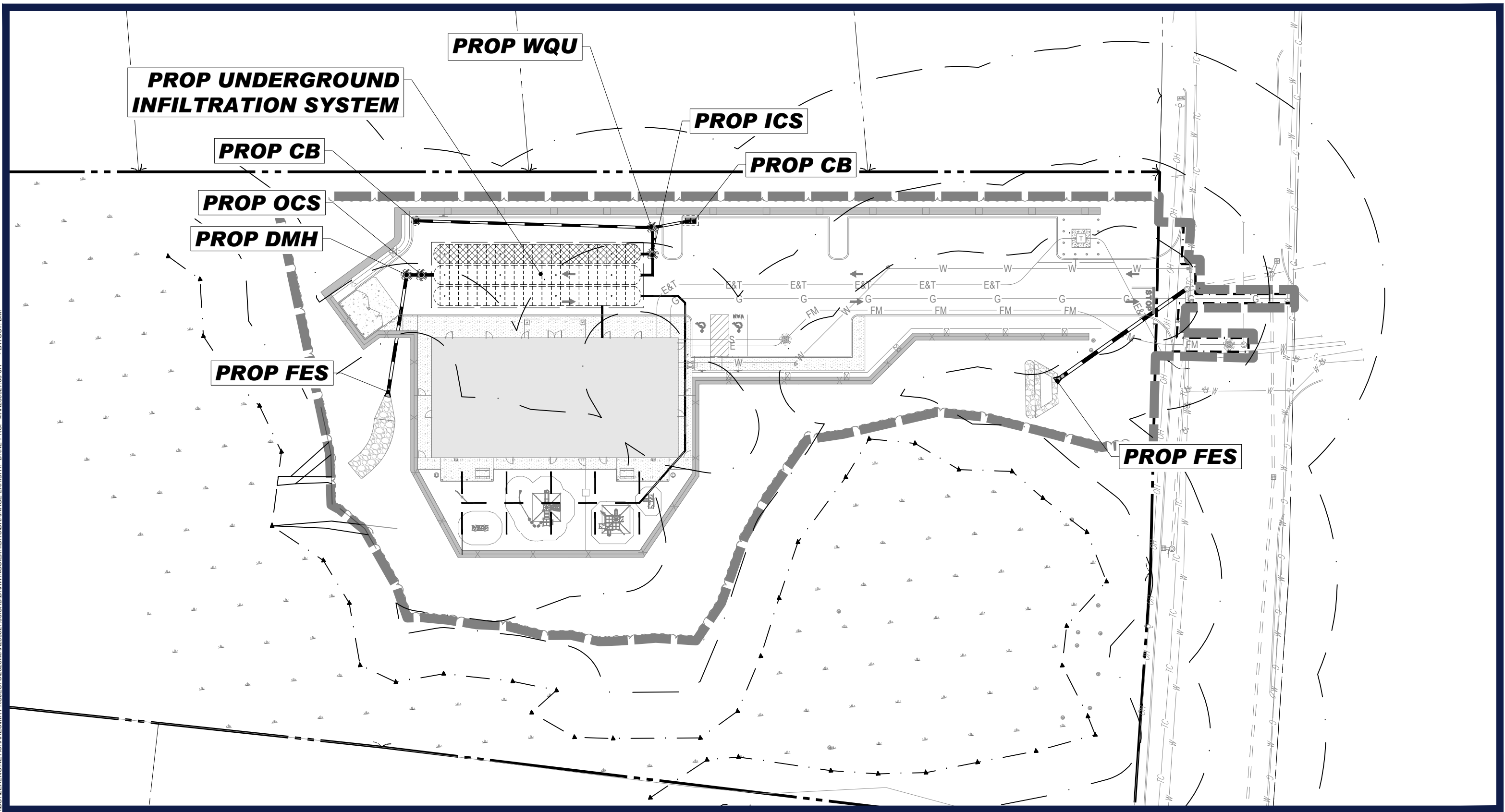
Location and Method of Disposal _____

Procedures, method, and precautions instituted to prevent a similar occurrence from recurring: _____

Additional Contact Numbers:

- DEPARTMENT OF ENVIRONMENTAL PROTECTION (DEP) EMERGENCY PHONE: 1-888-304-1133
- NATIONAL RESPONSE CENTER PHONE: (800) 424-8802
- U.S. ENVIRONMENTAL PROTECTION AGENCY PHONE: (888) 372-7341

\\BOHLER\NET\SHARES\MA-PROJECTS\2025\MAA250027.00\CAD\DRAWINGS\EXHIBITS\DRANAGE EXHIBIT\PROP-MAA250027.00-0A----->LAYOUT- BMP



**Save Valuable Land and
Protect Water Resources**



Isolator[®] Row O&M Manual
StormTech[®] Chamber System for Stormwater Management

1.0 The Isolator[®] Row

1.1 INTRODUCTION

An important component of any Stormwater Pollution Prevention Plan is inspection and maintenance. The StormTech Isolator Row is a patented technique to inexpensively enhance Total Suspended Solids (TSS) removal and provide easy access for inspection and maintenance.



Looking down the Isolator Row from the manhole opening, woven geotextile is shown between the chamber and stone base.

1.2 THE ISOLATOR ROW

The Isolator Row is a row of StormTech chambers, either SC-310, SC-310-3, SC-740, DC-780, MC-3500 or MC-4500 models, that is surrounded with filter fabric and connected to a closely located manhole for easy access. The fabric-wrapped chambers provide for settling and filtration of sediment as storm water rises in the Isolator Row and ultimately passes through the filter fabric. The open bottom chambers and perforated sidewalls (SC-310, SC-310-3 and SC-740 models) allow storm water to flow both vertically and horizontally out of the chambers. Sediments are captured in the Isolator Row protecting the storage areas of the adjacent stone and chambers from sediment accumulation.

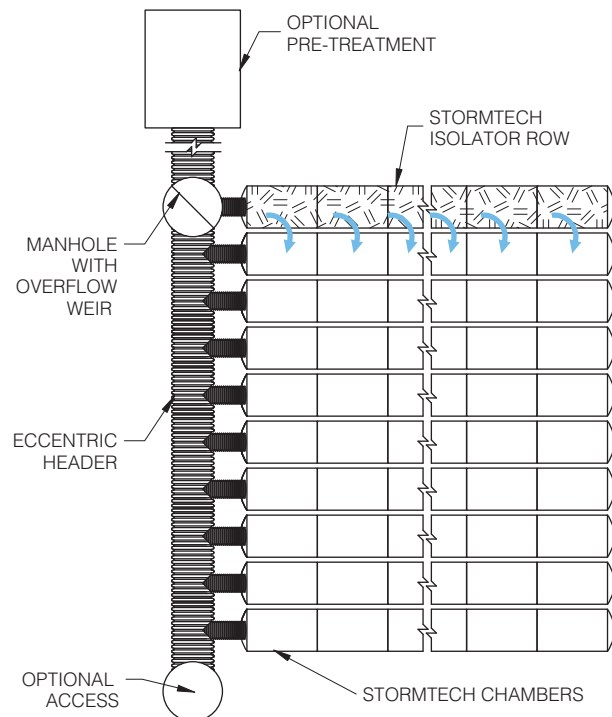
Two different fabrics are used for the Isolator Row. A woven geotextile fabric is placed between the stone and the Isolator Row chambers. The tough geotextile provides a media for storm water filtration and provides a durable surface for maintenance operations. It is also designed to prevent scour of the underlying stone and remain intact during high pressure jetting. A non-woven fabric is placed over the chambers to provide a filter media for flows passing through the perforations in the sidewall of the chamber. The non-woven fabric is not required over the DC-780, MC-3500 or MC-4500 models as these chambers do not have perforated side walls.

The Isolator Row is typically designed to capture the “first flush” and offers the versatility to be sized on a volume basis or flow rate basis. An upstream manhole not only provides access to the Isolator Row but typically includes a high flow weir such that storm water flowrates or volumes that exceed the capacity of the Isolator Row overtop the over flow weir and discharge through a manifold to the other chambers.

The Isolator Row may also be part of a treatment train. By treating storm water prior to entry into the chamber system, the service life can be extended and pollutants such as hydrocarbons can be captured. Pre-treatment best management practices can be as simple as deep sump catch basins, oil-water separators or can be innovative storm water treatment devices. The design of the treatment train and selection of pretreatment devices by the design engineer is often driven by regulatory requirements. Whether pretreatment is used or not, the Isolator Row is recommended by StormTech as an effective means to minimize maintenance requirements and maintenance costs.

Note: See the StormTech Design Manual for detailed information on designing inlets for a StormTech system, including the Isolator Row.

StormTech Isolator Row with Overflow Spillway (not to scale)



2.0 Isolator Row Inspection/Maintenance



2.1 INSPECTION

The frequency of Inspection and Maintenance varies by location. A routine inspection schedule needs to be established for each individual location based upon site specific variables. The type of land use (i.e. industrial, commercial, residential), anticipated pollutant load, percent imperviousness, climate, etc. all play a critical role in determining the actual frequency of inspection and maintenance practices.

At a minimum, StormTech recommends annual inspections. Initially, the Isolator Row should be inspected every 6 months for the first year of operation. For subsequent years, the inspection should be adjusted based upon previous observation of sediment deposition.

The Isolator Row incorporates a combination of standard manhole(s) and strategically located inspection ports (as needed). The inspection ports allow for easy access to the system from the surface, eliminating the need to perform a confined space entry for inspection purposes.

If upon visual inspection it is found that sediment has accumulated, a stadia rod should be inserted to determine the depth of sediment. When the average depth of sediment exceeds 3 inches throughout the length of the Isolator Row, clean-out should be performed.

2.2 MAINTENANCE

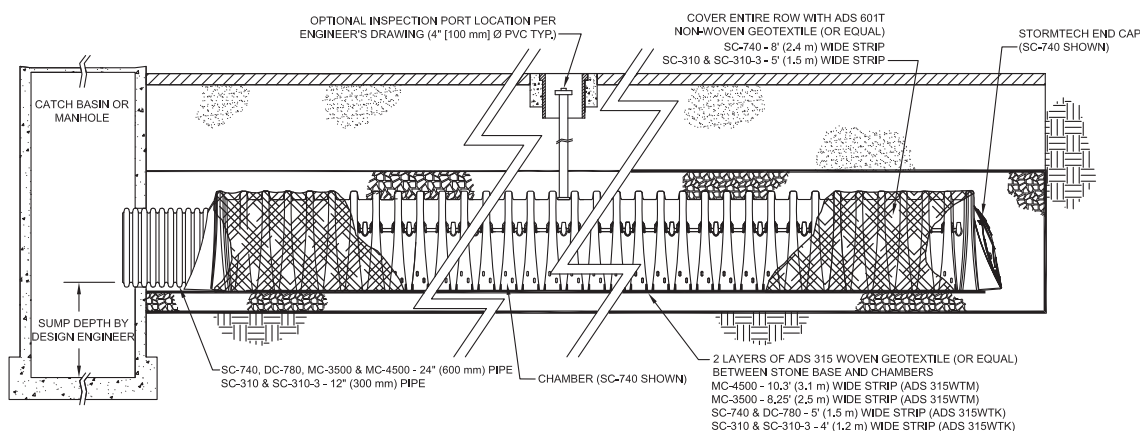
The Isolator Row was designed to reduce the cost of periodic maintenance. By “isolating” sediments to just one row, costs are dramatically reduced by eliminating the need to clean out each row of the entire storage bed. If inspection indicates the potential need for maintenance, access is provided via a manhole(s) located on the end(s) of the row for cleanout. If entry into the manhole is required, please follow local and OSHA rules for a confined space entries.



Examples of culvert cleaning nozzles appropriate for Isolator Row maintenance. (These are not StormTech products.)

Maintenance is accomplished with the JetVac process. The JetVac process utilizes a high pressure water nozzle to propel itself down the Isolator Row while scouring and suspending sediments. As the nozzle is retrieved, the captured pollutants are flushed back into the manhole for vacuuming. Most sewer and pipe maintenance companies have vacuum/JetVac combination vehicles. Selection of an appropriate JetVac nozzle will improve maintenance efficiency. Fixed nozzles designed for culverts or large diameter pipe cleaning are preferable. Rear facing jets with an effective spread of at least 45” are best. Most JetVac reels have 400 feet of hose allowing maintenance of an Isolator Row up to 50 chambers long. **The JetVac process shall only be performed on StormTech Isolator Rows that have AASHTO class 1 woven geotextile (as specified by StormTech) over their angular base stone.**

StormTech Isolator Row (not to scale)



NOTE: NON-WOVEN FABRIC IS ONLY REQUIRED OVER THE INLET PIPE CONNECTION INTO THE END CAP FOR DC-780, MC-3500 AND MC-4500 CHAMBER MODELS AND IS NOT REQUIRED OVER THE ENTIRE ISOLATOR ROW.

3.0 Isolator Row Step By Step Maintenance Procedures

Step 1) Inspect Isolator Row for sediment

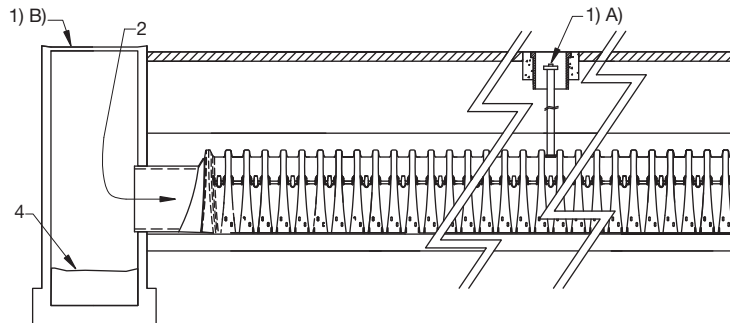
A) Inspection ports (if present)

- i. Remove lid from floor box frame
- ii. Remove cap from inspection riser
- iii. Using a flashlight and stadia rod, measure depth of sediment and record results on maintenance log.
- iv. If sediment is at, or above, 3 inch depth proceed to Step 2. If not proceed to step 3.

B) All Isolator Rows

- i. Remove cover from manhole at upstream end of Isolator Row
- ii. Using a flashlight, inspect down Isolator Row through outlet pipe
 1. Mirrors on poles or cameras may be used to avoid a confined space entry
 2. Follow OSHA regulations for confined space entry if entering manhole
- iii. If sediment is at or above the lower row of sidewall holes (approximately 3 inches) proceed to Step 2. If not proceed to Step 3.

StormTech Isolator Row (not to scale)



Step 2) Clean out Isolator Row using the JetVac process

- A) A fixed culvert cleaning nozzle with rear facing nozzle spread of 45 inches or more is preferable
- B) Apply multiple passes of JetVac until backflush water is clean
- C) Vacuum manhole sump as required

Step 3) Replace all caps, lids and covers, record observations and actions

Step 4) Inspect & clean catch basins and manholes upstream of the StormTech system

Sample Maintenance Log

Date	Stadia Rod Readings		Sediment Depth (1) - (2)	Observations/Actions	Inspector
	Fixed point to chamber bottom (1)	Fixed point to top of sediment (2)			
3/15/01	6.3 ft.	none		New installation. Fixed point is CI frame at grade	djm
9/24/01		6.2	0.1 ft.	Some grit felt	sm
6/20/03		5.8	0.5 ft.	Mucky feel, debris visible in manhole and in Isolator row, maintenance due	rv
7/7/03	6.3 ft.		0	System jetted and vacuumed	djm



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